

Generalized theory for the dynamic analysis of thin shells with application to circular cylindrical geometries

by

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Abstract

A generalized theory is formulated for the analysis of thin shells of general curvatures based on the variational form of the Hamiltonian functional in conjunction with tensor calculus. Simplifying approximations and subtle inconsistencies made at the early stages of common classical formulations are avoided herein, and hence, the present treatment leads to field equations and boundary conditions that are accurate and consistent. The theory is then specialized to circular cylindrical shells. The well-known field equations of Flugge and Donnell-Mushtari-Vlasov (DMV) theories are recovered as consistent approximations from the present theory. Closed form solutions are then developed for the present and past cylindrical shell theories by Flugge, Timoshenko, and DMV. A comparative study is conducted to assess and quantify the effects of approximations made in classical theories on the predicted displacements and stresses.

Keywords

Thin shell theory, shells with general curvature, cylindrical shells, Hamilton principle, steady state response, Tensor-based formulation

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30 1. Introduction

31 Thin shells are commonly encountered in structural applications, onshore and offshore
32 pipelines, and mechanical equipment, where they are commonly subjected to a variety of
33 static, quasi-static or dynamic loading. Under such conditions, shells may undergo a complex
34 deformational behaviour. The literature reveals the presence of several seemingly conflicting
35 stress-deformation theories for thin-shells. The selection for a proper shell theory in a given
36 application requires (a) an in-depth understanding of the underlying assumptions, and (b) a
37 critical assessment of various assumptions on the predictive ability of the theory. Within this
38 context, the present study contributes to both objectives by developing a theory for thin shells
39 of general geometries while keeping the assumptions and simplifications made to a minimum.
40 The formulation is then specialized to circular cylindrical shells. By applying various
41 approximations to the resulting formulation, other well-established cylindrical thin-walled
42 shell theories are recovered as special cases.

43 1.1 Literature Review

44 Classical linear shell theories involve the work of Timoshenko [1], Novozhilov [2], Koiter [3],
45 Flugge [4] (and [5]), Saada [6], DMV [7] and Niordson [7]. Non-linear shell theories include
46 the work of Leonard [8], Sanders [9], Koiter [10] , [11] and Budiansky [12]. The
47 comprehensive monograph by Leissa [13] provides a detailed report on the wide range of shell
48 theories available. Libai and Bert [14] developed mixed variational principles for the elastic
49 small-strains and large-rotation analysis of shells based on the Kirchhoff-Love hypothesis.
50 Muneeb et al. [15] developed a higher order theory for the dynamic response of isotropic
51 thermo-elastic analysis of cylindrical shells. Kolesnikov [16] formulated a refined theory for
52 the vibration of multilayer orthotropic cylindrical shells through a series expansion of the
53 radial displacement field in terms of the shell thickness. Through asymptotic expansion of the
54 general equilibrium equations for a general state of stress in three-dimensional bodies,
55 Niordson [17] formulated a two-dimensional shell theory for circular cylindrical shells.
56 Ugrimov [18] presented a layerwise generalized theory for the elasto-dynamic analysis of
57 multilayer plates by expanding the displacement components of each layer as power series of
58 the transverse coordinate. Ciarlet and Gratie [19] developed an approach to minimize the
59 quadratic problem arising in Koiter's linear shell theory. Using the Cosserat surface model,
60 Birsan [20] presented a theory for porous elastic shells by employing the Nunziato-Cowin

61 theory of elastic materials with voids, to characterize the porosity within the shell. Altenbach
62 et al. [21] developed a linear shell theory that accounts for transverse shear and surface
63 stresses. Weicker et al. ([22] and [23]) developed a closed form solution for the static analysis
64 of thin-walled pipes [22], and a finite element formulation [23]. Salahifar and Mohareb [24]
65 formulated a closed form solution for circular cylindrical shells under harmonic forces.
66 Amabili and Reddy [25] derived a consistent higher-order shear deformable theory for doubly
67 curved shells of general geometries based on non-linear strain-displacement expressions.
68 Their solution accounts for geometric imperfections. Paimushin [26] developed a large
69 displacement shell theory based on the classical Kirchhoff-Love assumption. The solution
70 accounts for deformations in the transverse direction by introducing additional displacement
71 fields. Salahifar and Mohareb [27] formulated a finite element for the analysis of circular
72 cylindrical thin shells under harmonic forces based on shape functions that satisfy the
73 governing field equations. Favata and Podio-Guidugli [28] developed a theory of linearly
74 elastic orthotropic shells with potential application to the continuous modeling of carbon
75 nanotubes. Sansour et al. [29] developed a computationally efficiently strain gradient
76 formulation that captures the scale effects of shell-like structures when one of the dimensions
77 is very small relative to the other two dimensions. Xuea et al. [30] extended the Karman-
78 Donnell theory for shallow cylindrical shells to account for large deformations. Based on an
79 expansion of the axisymmetric equations of elasticity, Zozulya [31] developed a high-order
80 theory for functionally graded axisymmetric cylindrical shells using Legendre polynomial
81 series. Carrera et al. [32] (and [33]) developed a unified hierarchical formulation for
82 multilayered composite structures, which enables the implementation of multiple plate/shell
83 theories and finite elements based on a few fundamental nuclei. Cattabiani et al. [34]
84 developed a variational shell theory that approximates the solution for the vibration problem
85 as a sum of shape functions that identically satisfy the equilibrium equations while satisfying
86 the weak form of the boundary conditions. Chowdhurya et al. [35] developed a state-based
87 peridynamic formulation for the linear elastic analysis of shells that captures discontinuities
88 by expressing the equations of motion in integro-differential form as opposed to partial
89 differential equations. Zveryayev [36] developed an iterative shell solution, in which the
90 three-dimensional equations of the elasticity in curvilinear coordinates were reduced using the
91 Saint-Venant semi-inverse method. Awrejcewicz et al. [37] presented a mathematical model
92 for nonlinear analysis of micro-shells that accounts for temperature-deformation coupling.
93 Using Hamilton's principle, Wangab et al. [38] developed a first-order shear deformable shell

94 theory for the free and transient vibration analysis of composite laminated cylindrical shells
95 by partitioning the radial displacement field into bending and shear components. Okhovat and
96 Bostrom [39] derived a hierarchy of shell equations in the form of power series in terms of the
97 shell thickness for the dynamic analysis of orthotropic cylindrical shells. Qingshan et al. [40]
98 developed a first-order shear deformable shell theory for the free and transient vibration
99 analysis of laminated open cylindrical shells with general boundary conditions.

100 1.2 Common Assumptions

101 In general, the theory of thin shells involves two aspects: a) developing the governing field
102 equations and the boundary conditions and b) providing solutions to the field equations for
103 problems with specific geometries, boundary conditions, and loading. Most thin-shell
104 formulations are based on the following assumptions:

- 105 1. Conservation of normals: All points lying on a normal to the middle surface before
106 deformation remain on the normal to the deformed middle surface. This implies a linear
107 distribution of the in-plane displacements across the thickness of the shell and zero shear
108 strains in the planes normal to the mid-surface.
- 109 2. A surface at a distance z from the middle surface before deformation will remain at the
110 same distance from the middle surface after deformation, and
- 111 3. Displacements are small compared to the radii of curvature of the middle surface. This
112 signifies that the curvatures of the shell mid-surface after deformation are considered
113 nearly equal to the curvatures before deformation.

114 1.3 Differences among Shell Theories

115 A survey of the literature reveals the presence of multiple cylindrical thin-shell theories. The
116 differences between these shell theories are attributed to the following aspects:

- 117 1. The adoption of different strain-displacement relationships: In some theories (e.g.,
118 Timoshenko[1] and Flugge [3]) the strain-displacement relationships are obtained based on
119 geometric inspection of an infinitesimal portion of the shell undergoing deformation. In
120 other theories (e.g., Novozhilov [2], Saada [6] and Niordson [7]), a vector or tensor analysis
121 approach is adopted.

122 2. The use of different methodologies in formulating the governing equations: In most theories
123 (e.g., Timoshenko [1], Flugge [4], Saada [6] and Niordson [7]), equilibrium equations are
124 obtained based on the force equilibrium of an infinitesimal shell element, while others (e.g.,
125 Novozhilov [2]) adopt a variational approach.

126 3. The enforcement of additional simplifications: Most researchers have applied various
127 simplifications at different stages of the formulation, which had implications on the final
128 form of the governing field equations and the boundary conditions. For example,
129 Novozhilov [2] has neglected some of the terms in the expression of total potential energy
130 compared to the other terms.

131 4. Inconsistencies in enforcing simplifications: Most theories (Timoshenko [1], Saada [6],
132 Niordson [7], etc.) involved slight inconsistencies in their simplifications. For example, the
133 effect of the shell thickness in the strain expression is neglected in some terms, but kept in
134 other terms within the same formulation. These types of inconsistencies, in most cases,
135 arose from separating the membrane strains from the bending strains.

136 Given the above discrepancies, it is difficult to judge which theories provide superior results
137 and which are most suited for a given engineering problem. While, in some cases, there could
138 be some qualitative notion on which theory could be judged to provide reliable predictions of
139 the response, a systematic quantitative comparative assessment is missing. For a given
140 problem, it becomes difficult to judge how much error is attributed to treating a given problem
141 as a thin shell and how much is attributed to the approximations and treatments specific to the
142 adopted theory. In order to elucidate the problem, the present treatment focuses on developing
143 a generalized thin shell theory in which the strain-displacement relationships are thoroughly
144 formulated based on tensor calculus in conjunction with assumptions that are well accepted
145 within the framework of all thin-shell theories, while avoiding inconsistencies.

146 2. **Generalized Thin Shell Theory Formulation**

147 2.1 **Geometric preliminaries**

148 In general, a thin shell can be considered as a group of parallel curved surfaces in the
149 immediate vicinity of the middle surface. Point P_0 on a surface can be defined by two
150 curvilinear coordinates of a two dimensional subspace (\mathbf{R}^α) of the general three-dimensional

151 space (\mathbf{R}^i). As a notation convention, all Latin indices take the values 1, 2, 3, while Greek
 152 indices take the values 1 and 2. This implies that, in a curvilinear coordinate system x^i , the
 153 geometry of any of those surfaces can be defined by two curvilinear coordinates x^α within
 154 the middle surface and an additional coordinate x^3 normal to the middle surface (Figure 1).

155 In a Cartesian coordinate system χ^i with \mathbf{I}_i as a covariant unit vector, position vector
 156 $\mathbf{R}(\chi_{P_1}^i(t)) = \chi_{P_1}^i(t)\mathbf{I}_i$ at time t for a point P_1 within the shell (Figure 1), can be defined as
 157 the vector sum of position vector $\mathbf{r}(\chi_{P_0}^i(t)) = \chi_{P_0}^i(t)\mathbf{I}_i$ of its projection P_0 on the middle
 158 surface and vector $\vec{P_0P_1}$, i.e.,

$$159 \quad \mathbf{R}(\chi_{P_1}^i(t)) = \mathbf{r}(\chi_{P_0}^i(t)) + \vec{P_0P_1}(\chi_{P_0}^i(t), \chi_{P_1}^i(t)) \quad (1)$$

160 In Eq. 1, expressing the Cartesian coordinates χ^i for points P_0 and P_1 (i. e. $\chi_{P_0}^i(t)$ and $\chi_{P_1}^i(t)$)
 161) in terms of curvilinear coordinates x^i enables expressing the position vectors for points P_0
 162 and P_1 as $\mathbf{r}(\chi_{P_0}^i(t)) = \chi^i(x_{r_0}^j, t)\mathbf{I}_i$ and $\mathbf{R}(\chi_{P_1}^i(t)) = \chi^i(x_{r_1}^j, t)\mathbf{I}_i$, respectively. Thus,
 163 Cartesian coordinates χ^i appearing in Eq. 1 can be expressed in terms of curvilinear
 164 coordinates x^i yielding

$$165 \quad \mathbf{R}(\chi^i(x_{r_1}^j, t)) = \mathbf{r}(\chi^i(x_{r_0}^j, t)) + \vec{P_0P_1}(\chi^i(x_{r_0}^j, t), \chi^i(x_{r_1}^j, t)) \quad (2)$$

166 or

$$167 \quad \mathbf{R}(\chi^i(x_{r_1}^j, t)) = \mathbf{r}(\chi^i(x_{r_0}^\alpha, t)) + x^3\mathbf{a}_3 \quad (3)$$

168 in which unit vector $\mathbf{a}^3 = (\mathbf{a}_1 \otimes \mathbf{a}_2) / |\mathbf{a}_1 \otimes \mathbf{a}_2|$ is normal to the plane defined by base vectors
 169 $\mathbf{a}_\alpha = \partial\mathbf{r} / \partial x^\alpha$. Equation 3 provides the transformation of a shell surface from curvilinear
 170 coordinate system x^i into Cartesian coordinate system χ^i . It can be re-written as

$$171 \quad \mathbf{R}(x^1, x^2, x^3, t) = \mathbf{r}(x^1, x^2, t) + x^3\mathbf{a}_3 \quad (4)$$

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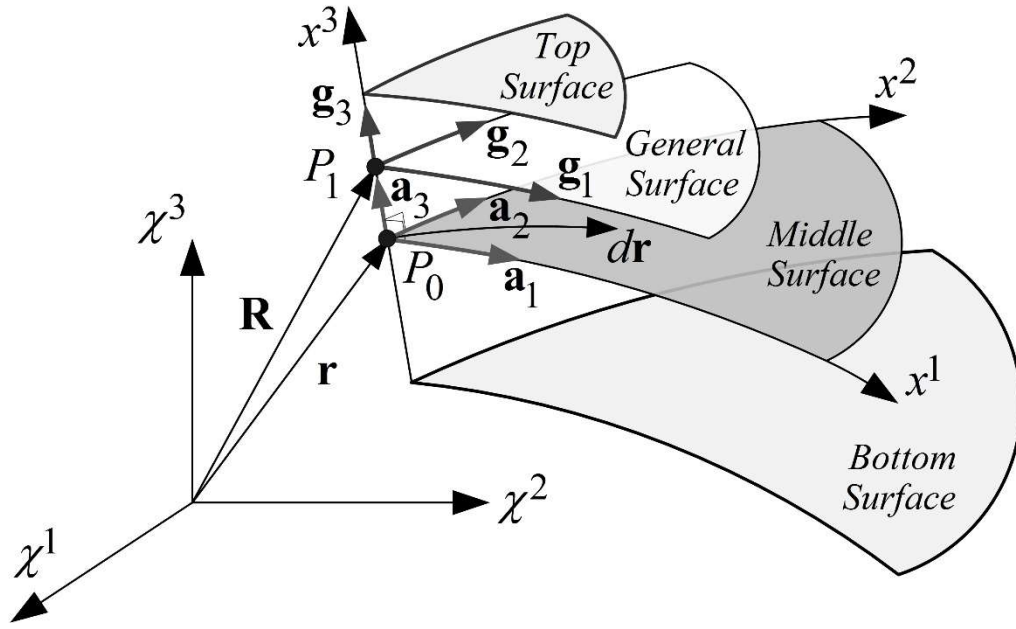


Figure 1 - Shell Geometry

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175 In a similar manner, bases $\mathbf{g}_i = \partial \mathbf{R} / \partial x^i$ pertain to a general surface within the shell volume.
 176 It can be shown that base vectors \mathbf{g}_α on a surface parallel to the mid-surface are related to the
 177 base vectors of the middle surface \mathbf{a}_γ through $\mathbf{g}_\alpha = \mu_\alpha^\gamma \mathbf{a}_\gamma$. Here, $\mu_\alpha^\gamma = \delta_\alpha^\gamma - x^3 \bar{b}_\alpha^\gamma$ is the shifter
 178 tensor, δ_α^γ is the Kronecker delta tensor and \bar{b}_α^γ is the mixed curvature tensor for the middle
 179 surface, which is related to the Christoffel symbol through $\bar{b}_\alpha^\gamma = -\bar{\Gamma}_{3\alpha}^\gamma$. Christoffel symbols
 180 of the second kind are defined as $\Gamma_{ij}^k = g^{kl} (g_{li,j} + g_{lj,i} - g_{ij,l}) / 2$ and, as a matter of
 181 convention, all symbols with a bar denote quantities pertaining to the middle surface, unless
 182 mentioned otherwise.

183 In general, the contravariant counterparts of the covariant base vectors are $\mathbf{g}^\alpha = \lambda_\gamma^\alpha \mathbf{a}^\gamma$ and
 184 $\mathbf{g}^3 = \mathbf{a}^3$, in which $\lambda_\gamma^\alpha \mu_\beta^\gamma = \delta_\beta^\alpha$ and the covariant and contravariant base vectors are related
 185 through $\mathbf{g}_j \cdot \mathbf{g}^i = \delta_j^i$. The covariant and contravariant metric tensors for surfaces parallel to the
 186 middle surface are respectively expressed as

187
$$\mathbf{g}_{\alpha\beta} = \mathbf{g}_\alpha \cdot \mathbf{g}_\beta = \mu_\alpha^\gamma \mu_\beta^\delta a_{\gamma\delta} \quad , \quad \mathbf{g}^{\alpha\beta} = \mathbf{g}^\alpha \cdot \mathbf{g}^\beta = \lambda_\gamma^\alpha \lambda_\delta^\beta a^{\gamma\delta} \quad (5a-b)$$

188 where $a_{\gamma\delta}$ and $a^{\gamma\delta}$ are the covariant and contravariant metric tensors of the middle surface,
 189 respectively, and defined as

$$190 \quad a_{\gamma\delta} = \mathbf{a}_\gamma \cdot \mathbf{a}_\delta \quad , \quad a^{\gamma\delta} = \mathbf{a}^\gamma \cdot \mathbf{a}^\delta \quad (6a-b)$$

191 The second fundamental tensor of the surface (curvature tensor) is defined as $\bar{b}_{\alpha\beta} = \mathbf{a}_{3,\alpha} \cdot \mathbf{a}_\beta$.

192 The Jacobian g at a generic point within the shell, given by $g = \det(g_{ij})$, represents a
 193 differential element of volume.

194 2.2 Strains in Thin Shells

195 Using the Lagrangian approach, the strain tensor is defined in terms of the metric tensor
 196 through $\eta_{ij} = (\hat{g}_{ij} - g_{ij})/2$. One can show [41] that the strain-displacement relationship is
 197 defined by $\eta_{ij} = \frac{1}{2}(U_i|_j + U_j|_i + U^k|_i U_k|_j)$ in which, the notation $|_i$ denotes the covariant
 198 derivative of the argument vector with respect to $i=1,2,3$. Under the small deformation
 199 assumption, the nonlinear terms $U^k|_i U_k|_j$ are negligible compared to the linear terms. Also,
 200 for thin shells, the strains can be resolved into in-plane components $\eta_{\alpha\beta}$ and out-of-plane
 201 components $\eta_{\alpha 3} = \eta_{3\alpha}$ and η_{33} , and the displacement strain relations can be approximated by:

$$202 \quad \eta_{\alpha\beta} = \frac{1}{2}(U_\alpha|_\beta + U_\beta|_\alpha) \quad , \quad \eta_{\alpha 3} = \eta_{3\alpha} = \frac{1}{2}(U_\alpha|_3 + U_3|_\alpha) \quad , \quad \eta_{33} = \frac{1}{2}(U_3|_3 + U_3|_3) \quad (7a-c)$$

203 According to Assumption 2 in Section 1.2, the strain normal to the mid-surface vanishes, i.e.
 204 $\eta_{33} = \frac{1}{2}(U_3|_3 + U_3|_3) \approx 0$. Also, according to Assumption 1 in Section 1.2, the transverse
 205 shear strains acting on the planes perpendicular to the mid-surface are negligible for thin shells
 206 leading to the simplification $\eta_{\alpha 3} = \eta_{3\alpha} \approx 0$.

207 2.3 Displacements of a Point within the Shell

208 In general, displacement vector \mathbf{U} of a point A offset from the middle surface can be defined
 209 as

$$210 \quad \mathbf{U} = U_i(x^1, x^2, x^3, t) \mathbf{g}^i = U^i(x^1, x^2, x^3, t) \mathbf{g}_i \quad (8)$$

211 where U_i and U^i are, respectively, the covariant and contravariant components of the
 212 displacement vector in the x^i coordinate system. Displacement vector \mathbf{U} is related to position
 213 vector \mathbf{R} in the undeformed configuration (Figure 2) and position vector $\hat{\mathbf{R}}$ in the deformed
 214 configuration through $\mathbf{U} = \hat{\mathbf{R}} - \mathbf{R}$. Vectors \mathbf{R} and $\hat{\mathbf{R}}$ at point A are related to vectors \mathbf{r} and
 215 $\hat{\mathbf{r}}$ at the corresponding point B on the mid-surface and unit vectors normal to the mid-surface
 216 \mathbf{a}_3 and $\hat{\mathbf{a}}_3$ as

$$217 \quad \mathbf{U} = \hat{\mathbf{R}} - \mathbf{R} = (\hat{\mathbf{r}} + x^3 \hat{\mathbf{a}}_3) - (\mathbf{r} + x^3 \mathbf{a}_3) = (\hat{\mathbf{r}} - \mathbf{r}) + x^3 (\hat{\mathbf{a}}_3 - \mathbf{a}_3) \quad (9)$$

218 Noting that the difference $\mathbf{u} = \hat{\mathbf{r}} - \mathbf{r}$ is the displacement vector at point B and \mathbf{a}_3 , $\hat{\mathbf{a}}_3$ are the
 219 unit vectors normal to the mid-surface at point B before and after deformation, the difference
 220 $\boldsymbol{\theta} = \hat{\mathbf{a}}_3 - \mathbf{a}_3$ characterizes the angle of rotation of the middle surface and thus Eq. 9 can be re-
 221 written as

$$222 \quad \mathbf{U} = \mathbf{u}(x^1, x^2, t) + x^3 \boldsymbol{\theta}(x^1, x^2, t) \quad (10)$$

223 Assumption 1 in Section 1.2, regarding the normality to the mid-surface, implies zero shear
 224 strains on the planes perpendicular to the mid-surface, i.e. $\eta_{3\alpha} = \eta_{\alpha 3} = (U_\alpha|_3 + U_3|_\alpha)/2 = 0$.
 225 It can be used [41] to relate the displacement \mathbf{U} of arbitrary point A , located at a distance
 226 $x^3 \neq 0$ from the shell mid-surface within the shell, to those of its projection on the middle
 227 surface, point B ($\mathbf{u} = u_i \mathbf{a}^i$), through

$$228 \quad \mathbf{U} = U_\alpha \mathbf{g}^\alpha + U_3 \mathbf{g}^3 \quad ; \quad U_\alpha = (\mu_\delta^\gamma u_\gamma - x^3 u_{3,\delta}) \mu_\alpha^\delta \quad , \quad U_3 = u_3 \quad (11)$$

229 Figure 2 shows points A and B , and their corresponding displacements \mathbf{U} and \mathbf{u} ,
 230 respectively. The geometric interpretations for $U_\alpha \mathbf{g}^\alpha$ and $U_3 \mathbf{g}^3$ are schematically shown in
 231 Figure 3, in which point A undergoes displacements U_α parallel to the shell mid-surface and
 232 a displacement U_3 normal to the mid-surface. Similarly, the contravariant components of the
 233 displacement are related to those of point B on the middle surface through

$$234 \quad \mathbf{U} = U^\alpha \mathbf{g}_\alpha + U^3 \mathbf{g}_3 \quad , \quad U^\alpha = U_\beta g^{\alpha\beta} = (\mu_\delta^\gamma u_\gamma - x^3 u_{3,\delta}) \mu_\beta^\delta g^{\alpha\beta} \quad , \quad U^3 = u^3 = u_3 \quad (12a-c)$$

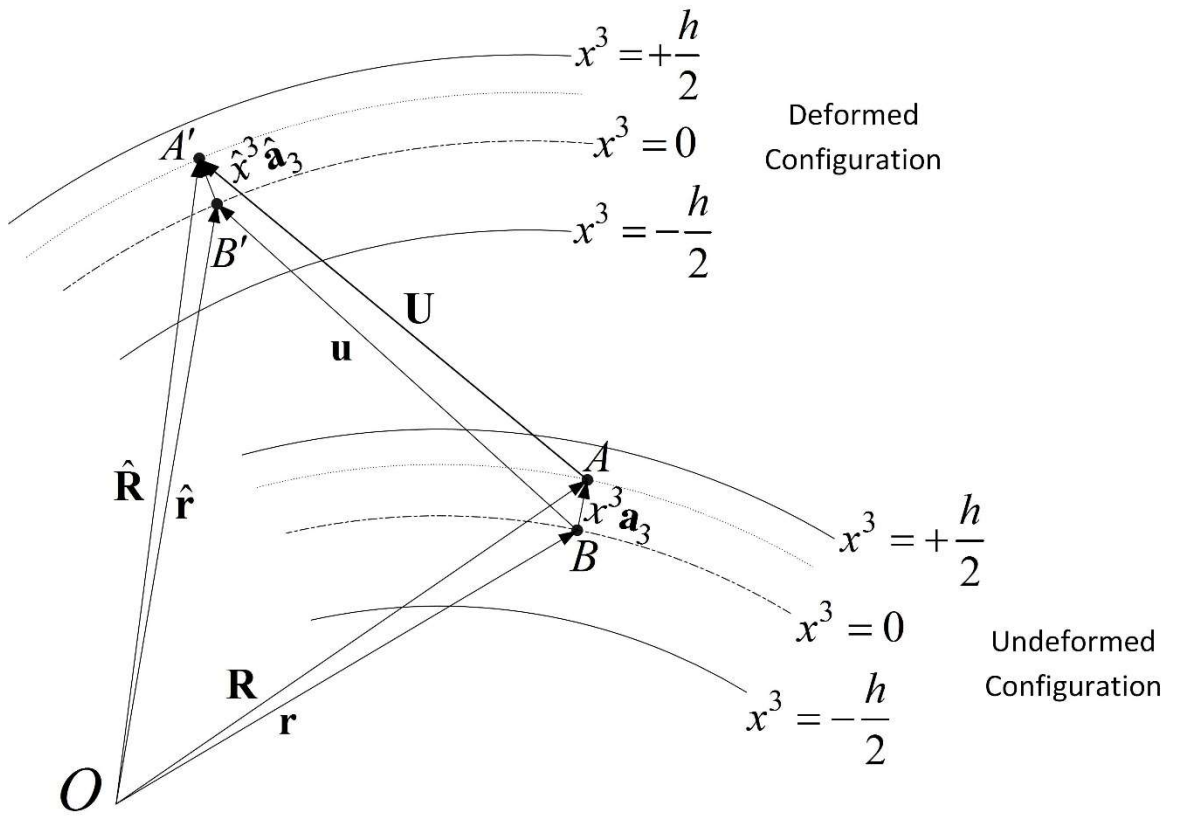


Figure 2- Deformed and Un-deformed Configurations of a Thin Shell

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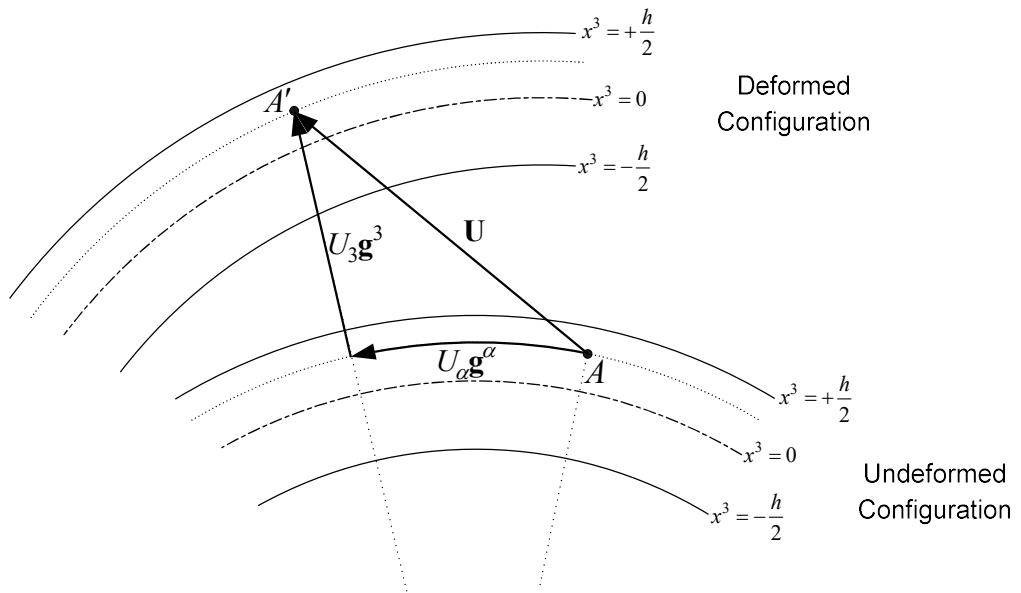


Figure 3 - Displacement of a generic point within the shell

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 240

241 2.4 Stress-Strain Relationship

242 For elastic materials, stress tensor σ^{ij} ($i, j = 1, 2, 3$) for a point at a distance x^3 from the middle
 243 surface ($-h/2 \leq x^3 \leq h/2$) is related to the corresponding strain tensor η_{lm} ($l, m = 1, 2, 3$)
 244 through $\sigma^{ij} = E^{ijlm}\eta_{lm}$ (e.g., Flugge [5]), in which E^{ijlm} is the fourth order constitutive elastic
 245 tensor. For thin shells, where the stresses perpendicular to the middle surface are negligible,
 246 the number of non-zero stress components reduces from nine to four and the stress-strain
 247 relationships reduce to

$$248 \quad \sigma^{\alpha\beta} = E^{\alpha\beta\lambda\delta}\eta_{\lambda\delta} \quad , \quad (\alpha, \beta, \lambda, \delta = 1, 2) \quad (13)$$

249 Under linear elastic isotropy, the constitutive tensor can be shown [41] to take the form

$$250 \quad E^{\alpha\beta\lambda\delta} = \frac{E}{2(1+\nu)} \left[g^{\alpha\lambda} g^{\beta\delta} + g^{\alpha\delta} g^{\beta\lambda} + \frac{2\nu}{1-\nu} g^{\alpha\beta} g^{\lambda\delta} \right] \quad (14)$$

251 in which E is the Modulus of elasticity and ν is Poisson's ratio.

252 2.5 Hamilton Principle for Thin Shells of General Geometries

253 According to Hamilton's principle, the variation $\delta\Pi$ of the mechanical energy is given by

$$254 \quad \int_{t_1}^{t_2} \delta\Pi dt = \int_{t_1}^{t_2} \delta(T^* - V^*) dt + \int_{t_1}^{t_2} \delta W^* dt = 0 \quad (15)$$

255 in which all time integrations are performed over an arbitrary period starting from time t_1 to
 256 time t_2 and T^* , V^* and W^* are the kinetic energy, internal strain energy and external work
 257 in the system, respectively, and are given by

$$258 \quad \delta T^* = \int_V \rho \dot{U}^i \delta \dot{U}_i dV \quad , \quad \delta V^* = \int_V E^{ijkl} \eta_{kl} \delta \eta_{ij} dV \quad , \quad \delta W^* = \int_V F^i \delta U_i dV \quad (16a-c)$$

259 in which integrations are over the volume V and symbols F^i denote the contravariant
 260 components of the applied forces. From Eq. 16a-c, by substituting into Eq. 15, integrating by
 261 parts the kinetic energy terms with respect to time, and using the relation
 262 $U_{\alpha|\beta} = U_{\alpha||\beta} - U_3 b_{\alpha\beta}$ (in which, symbol $||$ denotes the covariant derivative components of the

263 preceding argument with respect to x^α , i.e. $U_{\alpha\|\beta} = U_{\alpha,\beta} - U_{\gamma}\Gamma_{\alpha\beta}^{\gamma}$) in the internal strain energy
 264 integral term and integrating by parts with respect to x^β yields [41]

$$\begin{aligned}
 \int_{t_1}^{t_2} \delta \Pi dt = & \left[\int_V \dot{U}^i \rho \delta U_i dV \right]_{t_1}^{t_2} + \int_{t_1}^{t_2} \left\{ - \int_V \ddot{U}^i \rho \delta U_i dV dt \right. \\
 & - \int_A \left[\frac{1}{2} E^{\alpha\beta\lambda\delta} (U_{\lambda\|\delta} + U_{\delta\|\lambda} - 2U_3 b_{\lambda\delta}) \delta U_{\alpha} \right]_0^{x^\beta} dA \\
 265 & + \int_V \left[\frac{1}{2} E^{\alpha\beta\lambda\delta} (U_{\lambda\|\delta\beta} + U_{\delta\|\lambda\beta} - 2U_{3,\beta} b_{\lambda\delta} - 2U_3 b_{\lambda\delta\|\beta}) \right. \\
 & \quad \left. + \frac{1}{2} (E^{\alpha\beta\lambda\delta\|\beta}) (U_{\lambda\|\delta} + U_{\delta\|\lambda} - 2U_3 b_{\lambda\delta}) \right] \delta U_{\alpha} dV \\
 & + \int_V \frac{1}{2} E^{\alpha\beta\lambda\delta} (U_{\lambda\|\delta} + U_{\delta\|\lambda} - 2U_3 b_{\lambda\delta}) (b_{\alpha\beta} \delta U_3) dV \\
 & \left. + \int_V F^i \delta U_i dV \right\} dt = 0
 \end{aligned} \tag{17}$$

266 Equation 17 is the variational form of the mechanical energy for a thin shell with general
 267 curvature. The theory can be specialized to shells with specific geometries. In this context, the
 268 present paper aims at specializing Eq. 17 to circular cylindrical shells. Readers interested in
 269 the application of the theory to other shell geometries are referred to [41], where it has been
 270 specialized to toroidal shells.

271 3. Special Case: Circular Cylindrical Thin Shells (CCTS)

272 3.1 Geometric properties

273 Position vector \mathbf{r} for a general point lying on the middle surface of a circular cylinder is
 274 expressed in terms of unit vectors $\bar{\mathbf{I}}_i$ in the Cartesian coordinate system χ^i (Figure 4). In terms
 275 of the curvilinear coordinates system x^i , it takes the form

$$276 \quad \mathbf{r}(x^1, x^2, t) = x^1 \bar{\mathbf{I}}_1 + R \sin(x^2/R) \bar{\mathbf{I}}_2 + R \cos(x^2/R) \bar{\mathbf{I}}_3 \tag{18}$$

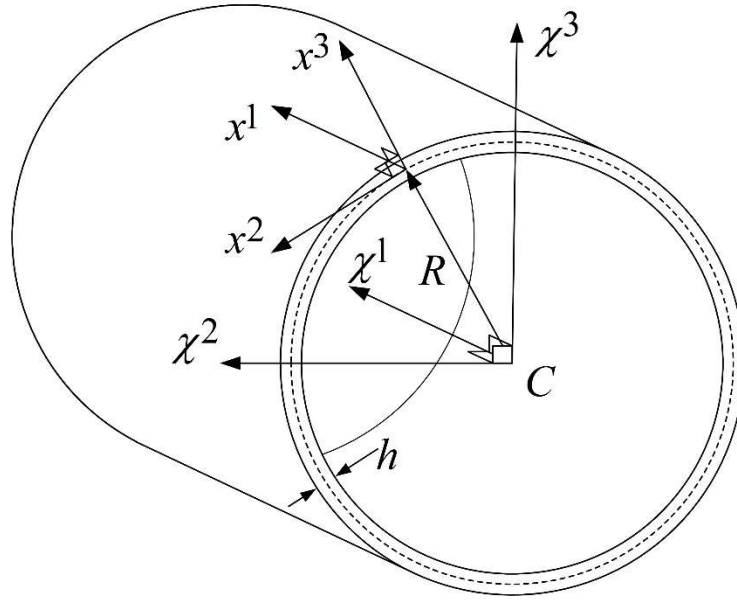


Figure 4 - Cartesian and Curvilinear Coordinates

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278

279 From this point, symbols (x, s, z) will be used instead of (x^1, x^2, x^3) for clarity. The non-zero
 280 components of the covariant and contravariant base vectors for a surface parallel to the mid-
 281 surface lying within the shell can be shown [41] to be

$$\begin{aligned}
 \mathbf{g}_1 &= \bar{\mathbf{j}}_1, & \mathbf{g}_2 &= (1+z/R)\bar{\mathbf{j}}_2, & \mathbf{g}_3 &= \bar{\mathbf{j}}_3 \\
 \mathbf{g}^1 &= \bar{\mathbf{j}}^1, & \mathbf{g}^2 &= (1+z/R)^{-1}\bar{\mathbf{j}}^2, & \mathbf{g}^3 &= \bar{\mathbf{j}}^3
 \end{aligned}
 \tag{19}$$

283 where $\bar{\mathbf{j}}_i$ and $\bar{\mathbf{j}}^i$ are the covariant and contravariant unit vectors associated with base vectors
 284 \mathbf{g}_i and \mathbf{g}^i , respectively. The non-zero covariant and contravariant components of the metric
 285 tensor are

$$g_{11} = g_{33} = g^{11} = g^{33} = 1, \quad g_{22} = (1+z/R)^2, \quad g^{22} = (1+z/R)^{-2}
 \tag{20}$$

287 Also, the non-zero covariant and mixed components of the curvature tensor for a general
 288 surface within the shell that is parallel to the middle surface are $b_{22} = -(1+z/R)/R$ and
 289 $b_2^2 = -1/R(1+z/R)$, respectively. Finally, the volume of an infinitesimal volumetric element
 290 is $dV = (1+z/R)dzdsdx$.

291 3.2 Displacements in terms of mid-surface displacements

292 Considering the geometric properties obtained in Section 3.1 , the covariant and contravariant
293 displacement components are expressed as

$$294 \begin{aligned} U_1 &= u - zw_{,1} \quad , \quad U_2 = (1 + z/R)^2 v - z(1 + z/R)w_{,2} \quad , \quad U_3 = w \\ U^1 &= u - zw_{,1} \quad , \quad U^2 = v - z(1 + z/R)^{-1} w_{,2} \quad , \quad U^3 = w \end{aligned} \quad (21a-f)$$

295 where symbols (u, v, w) have been used instead of (u_1, u_2, u_3) for clarity. It is noted that the
296 covariant and contravariant components of the displacements become identical at the middle
297 surface $z = 0$, i.e., $U_i(z = 0) = U^i(z = 0)$.

298 3.3 Strain-displacement relations

299 By specializing the strain displacement relations in Eq. 7a, to cylindrical shells (e.g., [41]) one
300 obtains

$$301 \quad \eta_{\alpha\beta} = \frac{1}{2} \left(\mu_{\alpha}^{\delta} \mu_{\delta}^{\gamma} u_{\gamma,\beta} - z \mu_{\alpha}^{\delta} u_{3,\delta\beta} - \mu_{\alpha}^{\gamma} u_3 \bar{b}_{\gamma\beta} + \mu_{\beta}^{\delta} \mu_{\delta}^{\gamma} u_{\gamma,\alpha} - z \mu_{\beta}^{\delta} u_{3,\delta\alpha} - \mu_{\beta}^{\gamma} u_3 \bar{b}_{\gamma\alpha} \right) \quad (22)$$

302 Prescribing the shifter tensor μ_{α}^{δ} and the curvature tensor of the middle surface $\bar{b}_{\gamma\beta}$, introduced
303 in Sec. 2.1 , for CCTSs and substituting into Eq. 22 yields the corresponding covariant strain
304 tensor. The physical components of the strain tensor, denoted subsequently by a tilde, can be
305 expressed as

$$306 \begin{aligned} \tilde{\eta}_{11} &= u_{,1} - zw_{,11} \\ \tilde{\eta}_{22} &= v_{,2} + \frac{w}{R(1+z/R)} - \frac{zw_{,22}}{(1+z/R)} \\ \tilde{\eta}_{12} = \tilde{\eta}_{21} &= \frac{1}{2} \left\{ \frac{u_{,2}}{(1+z/R)} + (1+z/R)v_{,1} - z \left[1 + \frac{1}{(1+z/R)} \right] w_{,12} \right\} \end{aligned} \quad (23a-c)$$

307 3.4 Components of Constitutive Tensor

308 Given the components of the metric tensors (Eqs.20), by substituting into the elastic
309 constitutive tensor in Eq.14, one recovers the following expressions for non-zero components
310 of the constitutive tensor

$$\begin{aligned}
E^{1111} &= \frac{E}{1-\nu^2}, & E^{2222} &= \frac{E}{(1-\nu^2)(1+z/R)^4}, & E^{1122} = E^{2211} &= \frac{E\nu}{(1-\nu^2)(1+z/R)^2}, \\
E^{1212} = E^{1221} = E^{2112} = E^{2121} &= \frac{E(1-\nu)}{2(1-\nu^2)(1+z/R)^2}
\end{aligned}
\tag{24a-d}$$

3.5 Hamilton Principle for CCTS

From Eqs. 21 and 24, by substituting into the variation of the Hamilton's principle in Eq. 17, integrating with respect to z , grouping like-terms and integrating by parts with respect to x and s yields [41]

$$\begin{aligned}
\int_{t_1}^{t_2} \delta \Pi dt &= \int_{t_1}^{t_2} \left\langle \int_s \frac{Eh}{1-\nu^2} \left[\left({}^1\bar{H}u_{,1} + {}^2\bar{H}v_{,2} + {}^3\bar{H}w + {}^4\bar{H}w_{,11} \right) \delta u \right. \right. \\
&\quad + \left({}^2\bar{H}u_{,2} + {}^2\bar{H}v_{,1} + {}^3\bar{H}w_{,12} \right) \delta v \\
&\quad + \left({}^3\bar{H}u_{,11} + {}^3\bar{H}u_{,22} + {}^3\bar{H}v_{,12} + {}^4\bar{H}w_{,111} + {}^5\bar{H}w_{,122} + {}^6\bar{H}\ddot{u} + {}^7\bar{H}\ddot{w}_{,1} + {}^8\bar{H}f^{xw} \right) \delta w \\
&\quad \left. + \left({}^4\bar{H}u_{,1} + {}^4\bar{H}v_{,2} + {}^4\bar{H}w_{,11} + {}^4\bar{H}w_{,22} \right) \delta w_{,1} \right]_0^L ds \\
&+ \int_A \frac{Eh}{1-\nu^2} \left[\left({}^1\bar{G}u_{,11} + {}^2\bar{G}u_{,22} + {}^3\bar{G}v_{,12} + {}^4\bar{G}w_{,1} + {}^5\bar{G}w_{,111} + {}^6\bar{G}w_{,122} \right. \right. \\
&\quad \left. \left. + {}^7\bar{G}\ddot{u} + {}^8\bar{G}\ddot{w}_{,1} + {}^9\bar{G}f^{xu} \right) \delta u \right. \\
&\quad + \left({}^2\bar{G}u_{,12} + {}^2\bar{G}v_{,11} + {}^3\bar{G}v_{,22} + {}^4\bar{G}w_{,2} + {}^5\bar{G}w_{,112} + {}^2\bar{G}w_{,222} + {}^6\bar{G}\ddot{v} + {}^7\bar{G}\ddot{w}_{,2} + {}^8\bar{G}f^{sv} \right) \delta v \\
&\quad + \left({}^3\bar{G}u_{,1} + {}^3\bar{G}v_{,2} + {}^3\bar{G}w + {}^a\bar{G}w_{,11} + {}^4\bar{G}w_{,22} + {}^5\bar{G}u_{,111} + {}^6\bar{G}u_{,122} + {}^7\bar{G}v_{,112} + {}^b\bar{G}w_{,222} \right. \\
&\quad \left. + {}^8\bar{G}w_{,1111} + {}^9\bar{G}w_{,1122} + {}^{10}\bar{G}w_{,2222} + {}^{11}\bar{G}\ddot{w} + {}^{12}\bar{G}\ddot{w}_{,11} + {}^{13}\bar{G}\ddot{w}_{,22} + {}^{14}\bar{G}\ddot{u}_{,1} + {}^{15}\bar{G}\ddot{v}_{,2} \right. \\
&\quad \left. + {}^{16}\bar{G}f^{xw} + {}^{17}\bar{G}f^{sw} + {}^{18}\bar{G}f^{zw} \right) \delta w \left. \right] ds dx \Bigg\rangle dt = 0
\end{aligned}
\tag{25}$$

where coefficients ${}^1\bar{G} - {}^3\bar{G}$ and ${}^1\bar{H} - {}^4\bar{H}$, depend on the parameter

$$\Phi = \Phi(h/R) = (R/h) \ln \left[(2+h/R)/(2-h/R) \right]
\tag{26}$$

and are defined in Appendix A. The terms $f^{xu}, f^{xw}, f^{sv}, f^{sw}, f^{zw}$ arising in Eq. 25 are related to the physical force components per unit volume p^x, p^s, p^z and are provided in Appendix B.

323 3.6 Discussion

324 The field equations and corresponding boundary conditions for a CCTS can be obtained from
325 Eq. 25 through integration by parts. The details and relevant equations are provided in
326 Appendix B, where the boundary conditions are given by Eq. B.2 and the field equations are
327 given by Eq. B.3. These are entirely consistent with the assumptions postulated in Section 1.2
328 . In order to recover simpler approximate forms of the field equations and cast the present
329 formulation in a format comparable to other CCTS theories, the logarithmic function (Eq. 26
330) appearing in Eq. 25 is replaced by a truncated Taylor series expansion

$$331 \quad \Phi(h/R) \approx 1 + (h/R)^2/12 + (h/R)^4/80 + \dots \quad (27)$$

332 By omitting all time dependent terms, retaining the traction terms f^{xu} , f^{sv} , f^{zw} , omitting
333 the terms f^{xw} , f^{sw} and using the series approximation in Eq. 27 up to the second term, one
334 recovers the field equations in Flugge's theory [4] as a special case of the present formulation.
335 A simpler version of the present theory can also be obtained by adopting the approximation
336 $1 + z/R \approx 1$, in the strain displacement expressions (Eqs. 23a-c), the base vector expressions,
337 and the Jacobian. The resulting field equations are found to coincide with those of the DMV
338 theory [7]. Finally, as $(1 + z/R) \rightarrow 1$, the present theory, the Flugge theory and the DMV are
339 found to converge to the same field equations.

340 In contrast to other thin-shell theories known to the authors, the present formulation treats the
341 applied loads as body forces, and thus, is able to capture the spatial variations of the loads
342 along all three dimensions. Mid-surface tractions, line loads, and point loads can all be treated
343 as body forces by using the Dirac delta and Heaviside functions.

344 3.7 Comparison

345 Table 1 compares the strain expressions adopted in the past theories to those of the present
346 study. All theories are in agreement regarding the expression $\tilde{\eta}_{11} = u_{,1} - zw_{,11}$ for the
347 longitudinal strain. The Jacobian determinant is taken as unity in all theories except the present
348 and Flugge's theories, in which the Jacobian is $(1 + z/R)^2$. Table 1 provides a comparison of
349 the circumferencial and shear strains $\tilde{\eta}_{22}$ and $\tilde{\eta}_{12}$ as given in various theories. Strain

350 expressions based on Flugge's theory are observed to become in exact agreement with those
351 of the present theory when the approximation $\Phi(h/R) \approx 1 + (h/R)^2/12$ is adopted. The
352 Novozhilov strain expressions can be obtained from the strains of the present theory by
353 applying the approximations $\Phi(h/R) \approx 1 + (h/R)^2/12$ and $1 + z/R \approx (1 - z/R)^{-1}$, while the
354 DMV strain expressions can be recovered from the strains of the present theory by applying
355 the approximations $\Phi(h/R) \approx 1 + (h/R)^2/12$ and $1 + z/R \approx 1$. In contrast, strain expressions
356 in the theories of Timoshenko [1], Saada [6], Morley-Koiter [7] and Niordson [7] include
357 additional terms that do not arise in the present tensor-based approach. Such terms stem from
358 additional approximations that have been introduced in these theories, but avoided in the
359 present tensorial treatment.

360 Table 1– Comparing strain-displacement expressions in various thin shell theories

Theory	Strain Expressions	
	$\tilde{\eta}_{22}$	$\tilde{\eta}_{12}$
Flugge's theory [4] & Present Theory with approximation $\Phi(h/R) \approx 1 + (h/R)^2/12$	$v_{,2} + \frac{1}{R+z} w - z \frac{R}{R+z} w_{,22}$	$\frac{1}{2} \left(\frac{R}{R+z} u_{,2} + \frac{R+z}{R} v_{,1} - z \frac{2R+z}{R+z} w_{,12} \right)$
Novozhilov [2] & Present Theory with approximations $\Phi(h/R) \approx 1 + (h/R)^2/12$ and $1 + z/R \approx (1 - z/R)^{-1}$	$v_{,2} + \frac{1}{R} \frac{R-z}{R} w - z w_{,22}$	$\frac{1}{2} \left(\frac{R-z}{R} u_{,2} + \frac{R+z}{R} v_{,1} - 2z w_{,12} \right)$
Saada [6]	$\frac{R+z}{R} v_{,2} + \frac{1}{R} w - z w_{,22}$	$\frac{1}{2} \left(u_{,2} + \frac{R+z}{R} v_{,1} - 2z w_{,12} \right)$
Timoshenko [1]	$\frac{R+z}{R} v_{,2} + \frac{1}{R} w - z w_{,22}$	$\frac{1}{2} \left(u_{,2} + \frac{R+2z}{R} v_{,1} - 2z w_{,12} \right)$
Morley-Koiter [7] & Niordson [7]	$\frac{R+2z}{R} v_{,2} + \frac{1}{R} \frac{R+z}{R} w - z w_{,22}$	$\frac{1}{2} \left(u_{,2} + \frac{R+2z}{R} v_{,1} - 2z w_{,12} \right)$
DMV [7] & Present Theory with approximations $\Phi(h/R) \approx 1$ and $1 + z/R \approx 1$	$v_{,2} + \frac{1}{R} w - z w_{,22}$	$\frac{1}{2} (u_{,2} + v_{,1} - 2z w_{,12})$

361 The presence of such differences at the strain expression stage, combined with other
362 simplifications and approximations, leads to differences in their corresponding field equations

363 and boundary conditions. In order to conduct a comparative study, irrespective of the CCTS
 364 theory adopted, the equilibrium conditions can be cast into the following generic form

$$365 \quad u_{,11} + \boxed{\alpha_{1.1}} \frac{1-\nu}{2} u_{,22} + \frac{1+\nu}{2} v_{,12} + \frac{\nu}{R} w_{,1} - \boxed{\alpha_{1.2}} \frac{h^2}{12R} w_{,111} + \boxed{\alpha_{1.3}} \frac{1-\nu}{2} w_{,122} \\ - \rho \frac{1-\nu^2}{E} \left(\ddot{u} - \boxed{\alpha_{1.4}} \frac{h^2}{12R} \ddot{w}_{,1} \right) + f^{xu} = 0$$

$$366 \quad \frac{1+\nu}{2} u_{,12} + \boxed{\alpha_{2.1}} \frac{1-\nu}{2} v_{,11} + \boxed{\alpha_{2.2}} v_{,22} + \frac{1}{R} w_{,2} - \boxed{\alpha_{2.3}} \frac{h^2}{12R} w_{,112} - \boxed{\alpha_{2.4}} \frac{h^2}{12R} w_{,222} \\ - \rho \frac{1-\nu^2}{E} \left(\boxed{\alpha_{2.5}} \ddot{v} - \boxed{\alpha_{2.6}} \frac{h^2}{6R} \ddot{w}_{,2} \right) + f^{sv} = 0$$

$$367 \quad \frac{\nu}{R} u_{,1} + \frac{1}{R} v_{,2} + \boxed{\alpha_{3.1}} \frac{1}{R^2} w + \boxed{\alpha_{3.2}} w_{,11} + \boxed{\alpha_{3.3}} w_{,22} - \boxed{\alpha_{3.4}} \frac{h^2}{12R} u_{,111} + \boxed{\alpha_{3.5}} \frac{1-\nu}{2} u_{,122} \\ - \boxed{\alpha_{3.6}} \frac{h^2}{12R} v_{,112} + \boxed{\alpha_{3.7}} w_{,222} + \frac{h^2}{12} w_{,1111} + \boxed{\alpha_{3.8}} w_{,1122} + \boxed{\alpha_{3.9}} w_{,2222} \\ - \rho (1-\nu^2) / E \left[\ddot{w} - \boxed{\alpha_{3.10}} \frac{h^2}{12} (\ddot{w}_{,11} + \ddot{w}_{,22}) + \boxed{\alpha_{3.11}} \frac{h^2}{12R} (\ddot{u}_{,1} + 2\ddot{v}_{,2}) \right] \\ + (1-\nu^2) / Eh (-f_{,1}^{xw} - f_{,2}^{sw} + f^{zw}) = 0$$

368 where coefficients $\alpha_{i,j}$ are theory-dependent and are defined in Table 2. As observed, the
 369 adoption of various assumptions/approximations in various theories, has led to the elimination
 370 of terms in some cases (e.g., $-\boxed{\alpha_{1.2}} (h^2/12R) w_{,111}$, $-\boxed{\alpha_{3.4}} (h^2/12R) u_{,111}$ and
 371 $\boxed{\alpha_{3.5}} [(1-\nu)/2] u_{,122}$ for Timoshenko, Novozhilov and Saada), and to the emergence of
 372 additional terms in other cases (e.g., $\boxed{\alpha_{2.4}} w_{,222}$, $\boxed{\alpha_{3.2}} w_{,11}$ and $\boxed{\alpha_{3.7}} w_{,222}$ for Timoshenko,
 373 Novozhilov and Saada), when compared to the present treatment. Such
 374 approximations/simplifications have only affected the magnitude of coefficients of other terms
 375 in some cases.

376

377 **Table 2 - Comparison of field equation coefficients $\alpha_{i,j}$ for various CCTS theories**

F.E.s	Present Theory	Flugge [4]	Timoshenko [1], Novozhilov [2], Saada [6],	Morley-Koiter [7], Niordson [7], DMV [7]
$\alpha_{1.1}$	Φ	$(1 + h^2/12R^2)$	1	1
$\alpha_{1.2}$	1	1	0	0

$\alpha_{1.3}$	$R(\Phi-1)$	$h^2/12R$	0	0
$\alpha_{1.4}$	1	-	0	0
$\alpha_{2.1}$	$1+h^2/4R^2$	$1+h^2/4R^2$	$1+h^2/3R^2$ [Take $(1+h^2/12R^2)$ for Timoshenko]	1
$\alpha_{2.2}$	1	1	$1+h^2/12R^2$	1
$\alpha_{2.3}$	$(3-\nu)/2$	$(3-\nu)/2$	1 [Take $(2-\nu)$ for Novozhilov]	0
$\alpha_{2.4}$	0	0	1	0
$\alpha_{2.5}$	$1+h^2/12R^2$	$1+h^2/12R^2$	-	1
$\alpha_{2.6}$	1	1	-	0
$\alpha_{3.1}$	Φ	$1+h^2/12R^2$	1	$1+h^2/12R^2$ [Take unity for DMV]
$\alpha_{3.2}$	0	0	0	$1+h^2/6R^2$ [Vanishes for DMV]
$\alpha_{3.3}$	$2(\Phi-1)$	$h^2/6R^2$	0	$h^2/6R^2$ [Vanishes for DMV]
$\alpha_{3.4}$	1	1	0	0
$\alpha_{3.5}$	$R(\Phi-1)$	$h^2/12R$	0	0
$\alpha_{3.6}$	$(3-\nu)/2$	$(3-\nu)/2$	$(2-\nu)$ [Take unity for Saada]	0
$\alpha_{3.7}$	0	0	$h^2/12R$	0
$\alpha_{3.8}$	$(3+\nu)h^2/24 + R^2(\Phi-1)(1-\nu)/2$	$h^2/6$	$h^2/6$	$h^2/6$
$\alpha_{3.9}$	$R^2(\Phi-1)$	$h^2/12$	$h^2/12$	$h^2/12$
$\alpha_{3.10}$	1	-	-	1
$\alpha_{3.11}$	1	-	-	0
$\alpha_{3.12}$	1	-	-	1

378 Similarly, the boundary terms have been cast into the following generic form.

379
$$\left(u_{,1} + \nu v_{,2} + \frac{\nu}{R} w - \boxed{\beta_{1.1}} \frac{h^2}{12R} w_{,11} \right) \delta u$$

380
$$\left(u_{,2} + \boxed{\beta_{2.1}} v_{,1} - \frac{h^2}{4R} w_{,12} \right) \delta v$$

$$381 \left(\boxed{\beta_{3.1}} \frac{h^2}{12R} u_{,11} - \boxed{\beta_{3.2}} \frac{1-\nu}{2} u_{,22} + \boxed{\beta_{3.3}} \frac{h^2}{12R} v_{,12} - \frac{h^2}{12} w_{,111} - \boxed{\beta_{3.4}} \frac{h^2}{12} w_{,122} \right) \delta w$$

$$382 \left(\boxed{\beta_{4.1}} \frac{h^2}{12R} u_{,1} + \boxed{\beta_{4.2}} \frac{h^2}{12R} v_{,2} - \frac{h^2}{12} w_{,11} - \frac{h^2}{12} v_{,22} \right) \delta w_{,1}$$

383 Table 3 provides coefficients $\beta_{i,j}$ for the present theory and those of Flugge, Timoshenko
384 and DMV.

385

386 **Table 3 - Comparison of field equation coefficients $\beta_{i,j}$ for various CCTS theories**

B.C.s	Present Theory	Flugge	Timoshenko	DMV
$\beta_{1.1}$	1	1	0	0
$\beta_{2.1}$	$(1 + h^2/4R^2)$	$(1 + h^2/4R^2)$	2/3	0
$\beta_{3.1}$	1	1	0	0
$\beta_{3.2}$	$R(\Phi - 1)$	$h^2/12R$	0	0
$\beta_{3.3}$	$(3 - \nu)/2$	$(3 - \nu)/2$	1	0
$\beta_{3.4}$	$[(3 - \nu)/2 + (\Phi - 1)(1 - \nu)6R^2/h^2]$	$(2 - \nu)$	1	$(2 - \nu)$
$\beta_{4.1}$	1	1	0	0
$\beta_{4.2}$	1	1	1	0

387

388 3.8 Steady State Analysis under Harmonic Forces

389 Consider a CCTS under general harmonic force per unit volume

$$390 [p^x(x, s, z, t), p^s(x, s, z, t), p^z(x, s, z, t)] = [\bar{p}^x(x, s, z), \bar{p}^s(x, s, z), \bar{p}^z(x, s, z)] \text{Re}(e^{i\bar{\omega}t}),$$

391 with an exciting frequency $\bar{\omega}$. It is possible to express the force functions using double Fourier

392 series with N circumferential modes and K longitudinal modes yielding:

$$\begin{Bmatrix} p^x \\ p^s \\ p^z \end{Bmatrix} = \sum_{n=-N}^N \sum_{k=-K}^K \left[\begin{Bmatrix} p_{nk}^x(z) \\ p_{nk}^s(z) \\ p_{nk}^z(z) \end{Bmatrix} e^{2\pi i k x/L} e^{i n s/R} \right] \text{Re}(e^{i \bar{\omega} t}) \quad ,$$

393

(28a-b)

$$\begin{Bmatrix} p_{nk}^x(z) \\ p_{nk}^s(z) \\ p_{nk}^z(z) \end{Bmatrix} = \int_{s=0}^{s=2\pi R} \int_{x=0}^{x=L} \begin{Bmatrix} \bar{p}^x(x, s, z) \\ \bar{p}^s(x, s, z) \\ \bar{p}^z(x, s, z) \end{Bmatrix} e^{-2\pi i k x/L} e^{-i n s/R} dx ds$$

394 in which, each applied force component is expressed in terms of its Fourier series expansion
 395 in x and s . From Eq. 28, by substituting into Equations B.1a-e and integrating over the
 396 thickness, one obtains

$$\begin{Bmatrix} f^{xu} \\ f^{sv} \\ f^{zw} \\ f^{xw} \\ f^{sw} \end{Bmatrix} = \left[\sum_{n=-N}^N \sum_{k=-K}^K \begin{Bmatrix} f_{nk}^{xu} \\ f_{nk}^{sv} \\ f_{nk}^{zw} \\ f_{nk}^{xw} \\ f_{nk}^{sw} \end{Bmatrix} e^{2\pi i k x/L} e^{i n s/R} \right] \text{Re}(e^{i \bar{\omega} t}) \quad ,$$

397

(29a-b)

$$\begin{Bmatrix} f_{nk}^{xu} \\ f_{nk}^{sv} \\ f_{nk}^{zw} \\ f_{nk}^{xw} \\ f_{nk}^{sw} \end{Bmatrix} = \int_{z=-\frac{h}{2}}^{z=+\frac{h}{2}} \begin{Bmatrix} p_{nk}^x(z)(1+z/R) \\ p_{nk}^s(z)(1+z/R)^2 \\ p_{nk}^z(z)(1+z/R) \\ p_{nk}^x(z)z(1+z/R) \\ p_{nk}^z(z)z(1+z/R) \end{Bmatrix} dz$$

398 For a given load, constants f^{xu} , f^{sv} , f^{zw} , f^{xw} and f^{sw} are determined and a procedure
 399 similar to that reported in [24] is then used to provide a closed form solution for each of the
 400 theories developed. The corresponding steady state displacements are expressed in the
 401 following form.

$$\begin{aligned}
402 \quad u(x, s, t) &= \bar{u}(x, s) e^{i\bar{\omega}t} = \sum_{n=-N}^N \left(\sum_{j=1}^8 A_{nj} \bar{A}_{nj} e^{m_{nj}x} + \sum_{k=-K}^K R_{nk} e^{2\pi i k x / L} \right) e^{ins/R} e^{i\bar{\omega}t} \\
v(x, s, t) &= \bar{v}(x, s) e^{i\bar{\omega}t} = \sum_{n=-N}^N \left(\sum_{j=1}^8 A_{nj} \bar{B}_{nj} e^{m_{nj}x} + \sum_{k=-K}^K S_{nk} e^{2\pi i k x / L} \right) e^{ins/R} e^{i\bar{\omega}t} \\
w(x, s, t) &= \bar{w}(x, s) e^{i\bar{\omega}t} = \sum_{n=-N}^N \left(\sum_{j=1}^8 A_{nj} \bar{C}_{nj} e^{m_{nj}x} + \sum_{k=-K}^K T_{nk} e^{2\pi i k x / L} \right) e^{ins/R} e^{i\bar{\omega}t}
\end{aligned} \tag{30a-c}$$

403 Symbols m_{nj} are the eigenvalues and each set of symbols \bar{A}_{nj} , \bar{B}_{nj} , \bar{C}_{nj} form the
404 corresponding eigenvectors of the characteristic equation of the system related to mode n .
405 Symbols R_{nk} , S_{nk} , T_{nk} and A_{nj} are unknown integration constants to be determined from
406 boundary conditions (e.g., [24]).

407 3.9 Examples

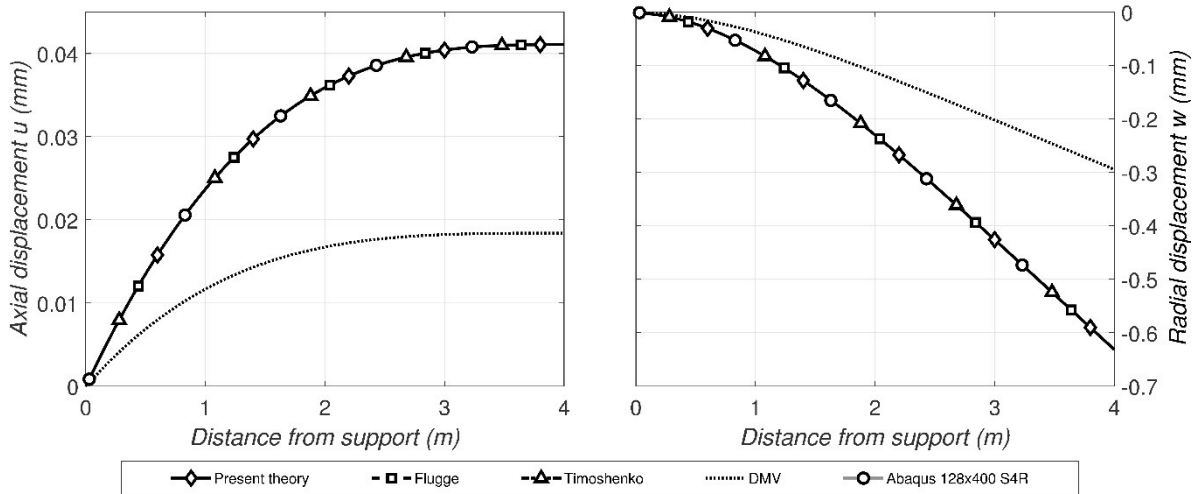
408 In the following examples, the above solution technique is adopted to express the applied
409 forces and displacement fields in the circumferential direction s and longitudinal direction x
410 . Examining the terms in Eqs. B.2 and B.3, as well as the entries in Table 2 and Table 3 reveals
411 that the theories of Timoshenko, Novozhilov and Saada have largely similar governing
412 equations and boundary conditions. Thus, the theory of Timoshenko has been chosen as a
413 representative of this group. Similarly, the DMV theory has been taken as a representative of
414 the Morley-Koiter, Niordson and DMV group of theories given their similarity. In all
415 examples, results based on Abaqus models are provided for comparison. Four types of
416 elements have been used in Abaqus models depending on the problem.

- 417 • S4R: A four-noded general-purpose (thin or thick) shell element, with reduced integration
418 and hourglass control.
- 419 • S8R: An eight-noded doubly curved thick shell element with reduced integration
- 420 • C3D8R: An eight-noded linearly interpolated brick element with reduced integration and
421 hourglass control, and
- 422 • C3D20R: A twenty-node quadratically interpolated brick element with reduced integration.

423 In all examples, steel is assumed to have a density of $\rho = 7850 \text{ kg/m}^3$, a modulus of elasticity
424 of 200 GPa and a Poisson's ratio of $\nu = 0.3$.

425 **3.9.1 Example 1 - Pipe under Self-Weight**

426 A 4 m long fixed-free horizontal pipe with a radius $R = 200\text{ mm}$ and a thickness $h = 6\text{ mm}$ is
 427 subjected to its self-weight. The response is obtained based on the present theory, the Flugge,
 428 Timoshenko, DMV theories, and the Abaqus shell model. In the Abaqus model, by refining
 429 the mesh from 64×200 to 128×400 S4R elements, the changes in the predicted
 430 displacements were within 0.9%, suggesting that convergence has been achieved. The static
 431 solution sought in the present problem is recovered by setting to zero the exciting frequency
 432 $\bar{\omega}$. Figure 5 provides a comparison of the longitudinal and radial displacements at the top
 433 generator of the pipe mid-surface. Nearly perfect agreement is observed among all solutions
 434 except for the DMV, which exhibits a significantly stiffer response as indicated by the lower
 435 displacements observed. A comparison of the predictions of the present and DMV theories
 436 shows a 53% difference (for a thickness $h = 6\text{ mm}$). When the pipe thickness is reduced to
 437 $h = 3\text{ mm}$, the difference between both solutions reduces to 22%, drops further to 6.5% for
 438 $h = 1.5\text{ mm}$, and becomes only 3% for $h = 1\text{ mm}$. The fact that the DMV theory adopts the
 439 approximation $(1 + z/R) \approx 1$, while the other theories do not enforce such an approximation,
 440 limits the usability of the DMV theory for very thin shells.



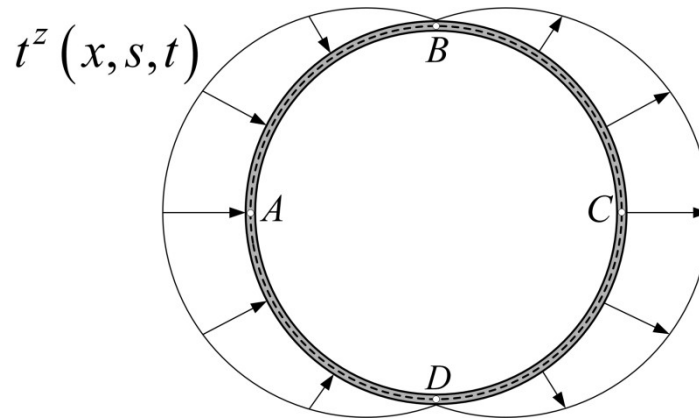
441
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Figure 5—Comparison of longitudinal and radial displacements at the top generator

443 **3.9.2 Example 2 - Fixed pipe under harmonic pressure**

444 Example 1 in [24] is revisited: A pipe with span length $L = 5\text{ m}$, radius $R = 25\text{ mm}$, thickness
 445 $h = 5\text{ mm}$ is fixed at both ends and is subjected to wind load traction
 446 $t^z(x, s, t) = 100 \cos(s/R) \cos(\bar{\omega}t)\text{ kN/m}^2$, where $\bar{\omega} = 200\text{ rad/s}$, simulating a vortex shedding

447 phenomenon. Casting the applied traction in form of body forces applicable to the present
 448 formulation yields $p^z = t^z(x, s, t) \text{Dirac}(z - h/2) \text{ N/m}^3$. In addition to the present theory,
 449 solutions are to be provided under the Flugge, Timoshenko, and DMV theories as well as
 450 Abaqus FEA for comparisons. In the Abaqus model, by refining the mesh from 64×200 to
 451 128×400 S4R elements, the change in the predicted displacements was less than 0.05%,
 452 suggesting that convergence has been achieved by 64×200 mesh.



453

454

Figure 6 - Cross-sectional distribution of the wind load.

455 The maximum longitudinal and radial displacements at midsurface ($z = 0$) take place at the
 456 generators passing through points $A (s = 0)$ and $C (s = \pi/4)$ and the maximum
 457 circumferential midsurface displacement occurs at the generators passing through points B (
 458 $s = \pi/8$) and $D (s = 3\pi/8)$, as shown in Figure 6. Table 4 gives the maximum displacements
 459 and their respective locations as provided by the present theory and percentage differences
 460 based on other solutions mentioned above. For the present problem, the predictions of the
 461 present theory are indistinguishable from those of the Flugge and Timoshenko theories, while
 462 the DMV solution slightly underpredicts the displacement by about 2.7%. The Abaqus
 463 solution, also, is found to slightly underpredicting the displacement by less than 1%.

464

465

Table 4 - Comparing the displacement components at the location of their maximum

	Present theory	Difference from the present theory (%)			
		Flugge	Timoshenko	DMV	Abaqus (64 × 200 S4R)
$\bar{u}_A = -\bar{u}_C$ at $x = 0.21L$ $-\bar{u}_A = \bar{u}_C$ at $x = 0.79L$	0.4410 mm	0.00	-0.01	2.73	0.91
$\bar{v}_B = -\bar{v}_D$ at $x = 0.5L$	3.753 mm	0.00	-0.01	2.72	0.92
$\bar{w}_A = -\bar{w}_C$ at $x = 0.5L$	-3.725 mm	0.00	-0.01	2.72	0.92

466

467 The distribution of longitudinal stresses σ_{xx} and transverse stresses σ_{ss} for the generator
468 passing through point A (Figure 6) are depicted in Figure 7 and Figure 8. Flugge's theory
469 predictions nearly coincide with those of the present solution, and the Timoshenko theory
470 predictions are in rather close agreement. The predictions of the DMV theory are 1-3% below
471 those based on the other theories. Although, in the present example, all four theories predict
472 stresses in rather close agreement, this will not always be the case. For instance, if all degrees
473 of freedom at end $x = L$ are released (turning the beam into a cantilever), the stresses predicted
474 by the DMV are found to depart from those predicted by the Timoshenko, Flugge and present
475 theories. Figure 9 depicts the longitudinal stress σ_{xx} along a generator passing through point
476 A (in Figure 6) under only 20% of the magnitude of $t^z(x, s, t)$ (to keep the maximum stresses
477 within the elastic range of the structural steel properties). The figure shows that the DMV
478 prediction significantly departs from those of the other theories.

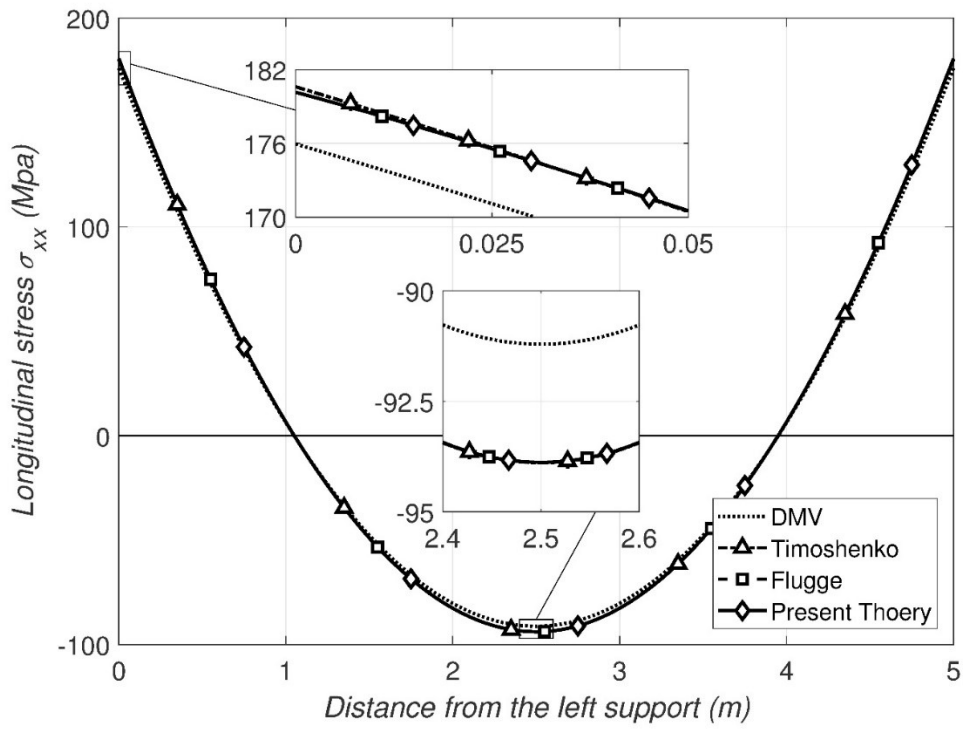


Figure 7 - Longitudinal stress (σ_{xx}) along a generator passing through point A

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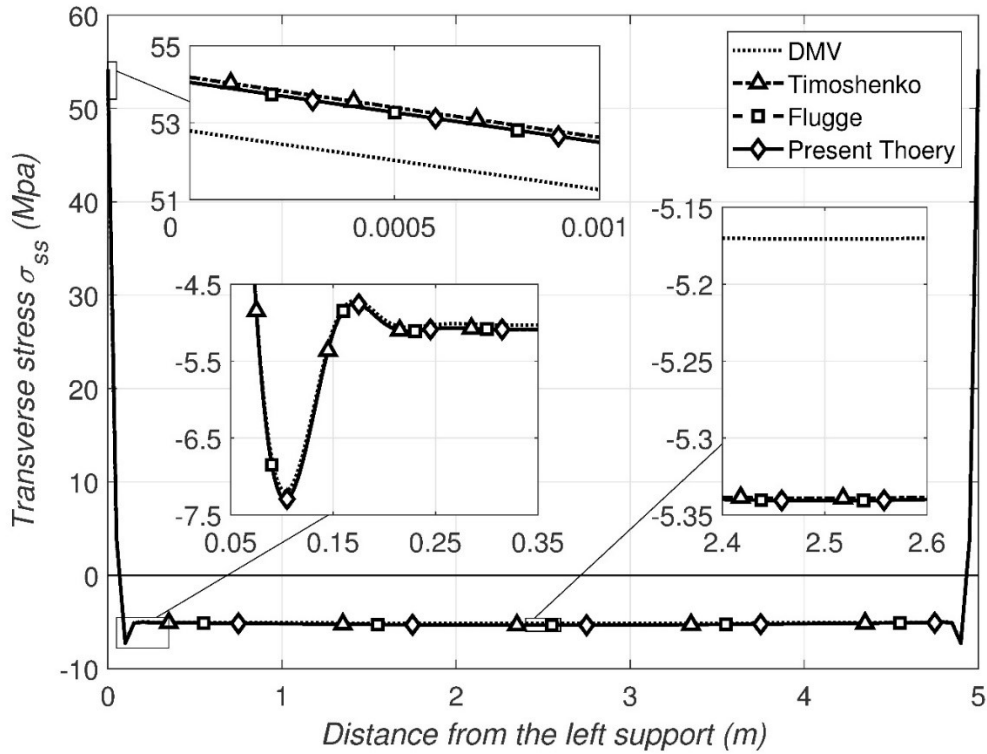
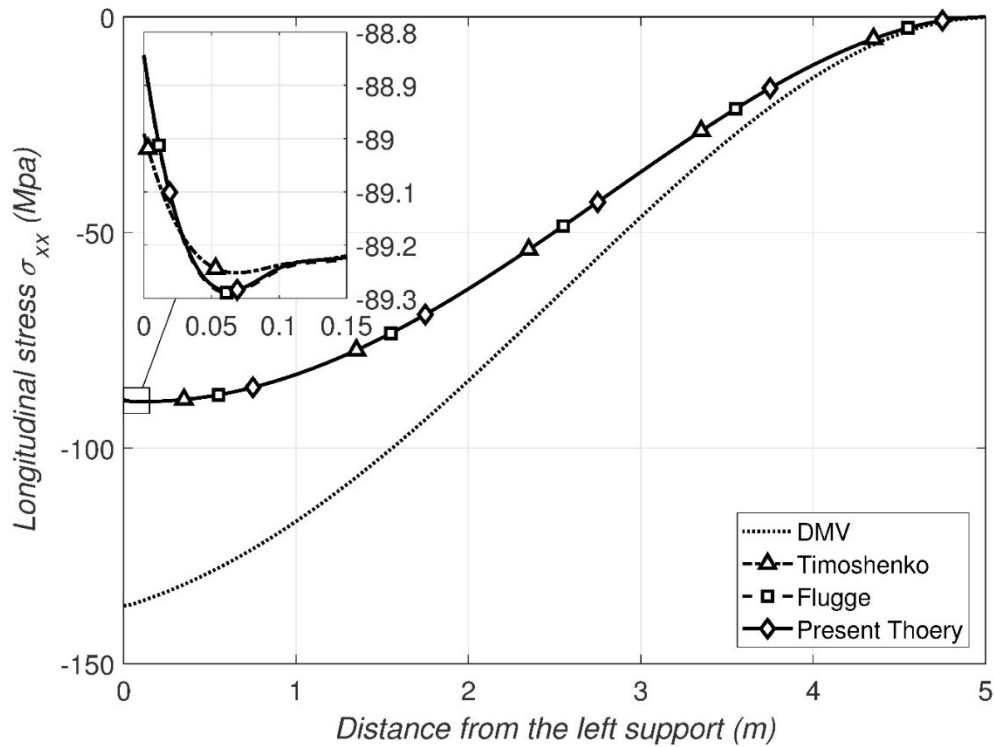


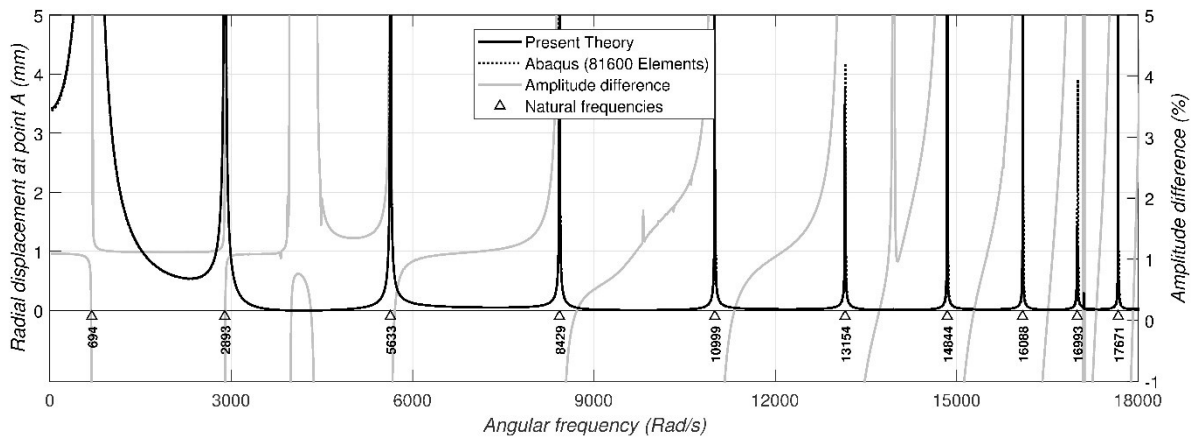
Figure 8 - Transverse stress (σ_{ss}) along a generator passing through point A

482
483



484
485 **Figure 9 - Longitudinal stress (σ_{xx}) along a generator passing through point A for cantilever**
486

487 The amplitudes of the radial displacements at Point A as predicted by the present solution are
488 compared to those predicted by the Abaqus shell model for wide range of angular frequencies
489 (Figure 10). In Abaqus, by refining the mesh from 80×255 to 160×510 S4R elements, the
490 change in the predicted displacements was less than 0.9% for frequencies ranging from 0 to 200
491 rad/s , suggesting that convergence has been achieved. The radial displacement amplitudes (as
492 identified on the scale of the left vertical axis) based on both solutions are nearly in perfect
493 agreement and are barely distinguishable within the scale of the figure. The loci of the peak
494 amplitudes, indicative of the natural frequencies, are also in excellent agreement with Abaqus
495 predictions. Also shown on the figure is the percentage difference of the radial displacement at
496 point A between both solutions, as identified on the scale of the right vertical axis. Except near
497 the natural frequencies where both solutions approach infinity, or where the amplitude
498 approaches zero, the amplitude difference is within 3%. The comparison indicates that the present
499 theory predicts lower natural frequencies for most of the frequency range and higher amplitudes
500 compared to Abaqus.



501

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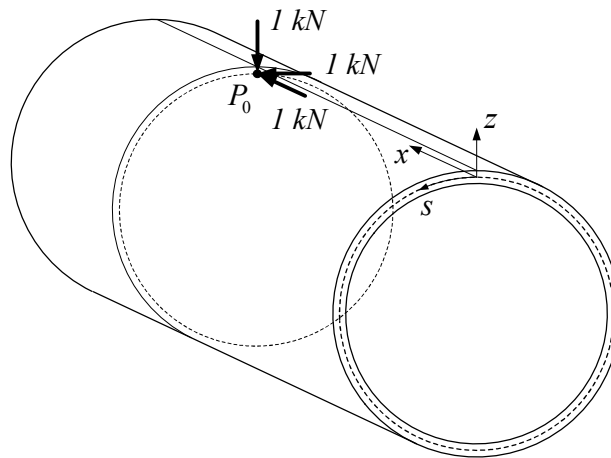
Figure 10 – Radial amplitude versus exciting frequency

504 **3.9.3 Example 3 - Pipe under a Point Load**

505 The fixed pipe in Example 2 is set free at end $x = L$ and is subject to an inclined point load

506 $(1,1,-1)kN$ (Figure 11) along the x , s and z directions, acting at point

507 $P_0(x, s, z) = (0.7L, 0, 0)$.



508

509

Figure 11 – Location, magnitude and direction of the point load components

510 The point loads can be expressed as body forces

511 $(p^x, p^s, p^z) = Dirac(x - 0.7L)Dirac(s)Dirac(z)(1,1,1)kN$ and expanded as double Fourier

512 series in the x and z directions. To investigate the convergence of the solution, the number of

513 Fourier modes taken is varied ($N = K = 7, 10, 15, 20, 25$). Five Abaqus S4R shell models,

514 with uniform meshes, were conducted for comparison ($16 \times 50, 32 \times 100, 64 \times 200, 128 \times 400$

515 and 320×640). The four coarse meshes are uniform while the most refined mesh (320×640

516) is identical to the 128×400 mesh, with a refined rectangular patch of elements in the
517 neighborhood of the point load. The patch extends longitudinally from $x = 0.6L$ to $x = 0.8L$
518 and from $s = -\pi R/2$ to $s = \pi R/2$ in the circumferential direction and consists of 256×320
519 elements (instead of 64×80 elements in the 128×400 elements model), and is intended to
520 capture localized deformations around the point load. In Abaqus, a mesh study indicated that
521 convergence was achieved for 128×400 S4R mesh. A finer 320×640 mesh resulted only in
522 a maximum difference of 0.23% in the displacements predicted.

523 The longitudinal, circumferential, and radial displacements at the generator at which the point
524 load is applied ($s = z = 0$) are respectively depicted in Figure 12 to Figure 14. As the number
525 of Fourier terms increases in the present solution, and as the number of elements increase in
526 the Abaqus solution, both models are observed to approach one another, albeit in the limit,
527 both converged solutions do not exactly coincide. The rather minor differences observed arise
528 from the fact that an exact representation of the point load requires an infinite number of modes
529 ($N \rightarrow \infty$ & $K \rightarrow \infty$), which is unachievable as the number of modes taken has to be
530 truncated. To confirm this hypothesis, an additional Abaqus solution is conducted (denoted as
531 A. 128×640 $N = K = 7$). The applied point loads (originally modelled as nodal forces in the
532 shell model) were replaced by applied tractions that follow the exact distribution based on the
533 first seven Fourier modes, in a manner consistent with the load characterization in the present
534 solution (denoted as P. T. $N = K = 7$). As expected, both solutions (P. T. $N = K = 7$), and
535 (A. 128×640 $N = K = 7$) predict nearly identical displacements.

536 Locally, the Abaqus shell solution exhibits minor displacement fluctuations within the vicinity
537 of the applied point load while the present solution predicts smoother displacement profiles.
538 Two additional Abaqus models were developed based on the S8R shell element and the
539 C3D20R brick element (both elements using quadratic interpolations as opposed to linear
540 interpolation in the S4R element). For the C3D20R, two layers of elements were taken across
541 the pipe thickness. Both models (not shown on Figure 12 to Figure 14 for clarity) were found
542 to exhibit less fluctuation within the vicinity of the point load than the S4R model, i.e., their
543 response is found closer to that of the present model.

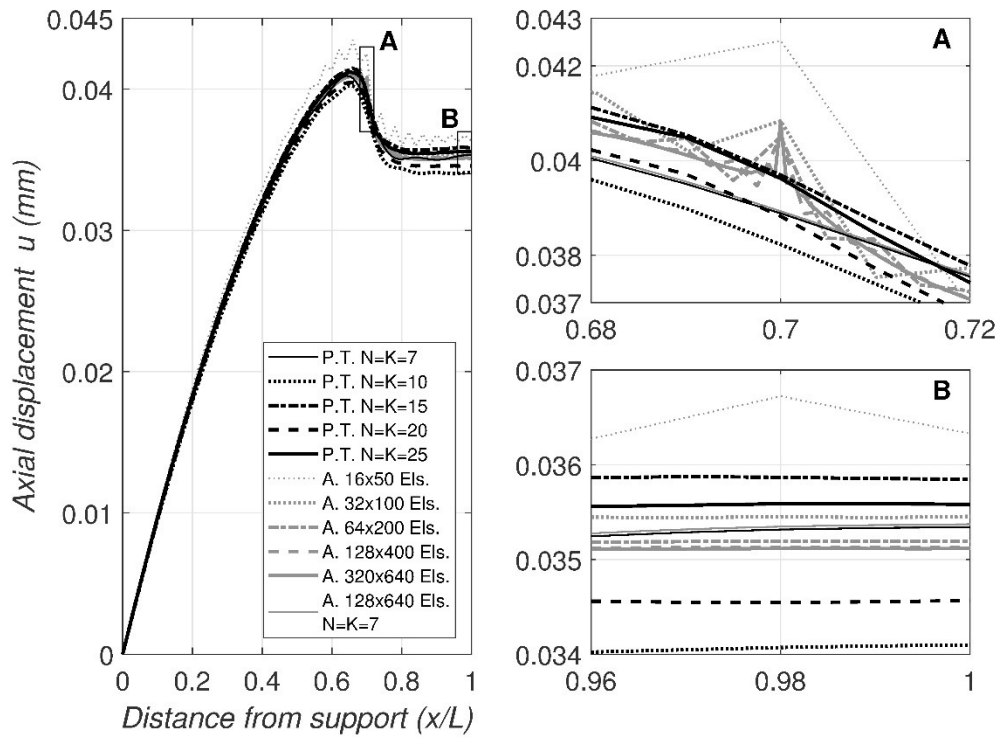


Figure 12 – Comparison of longitudinal displacements at top generator

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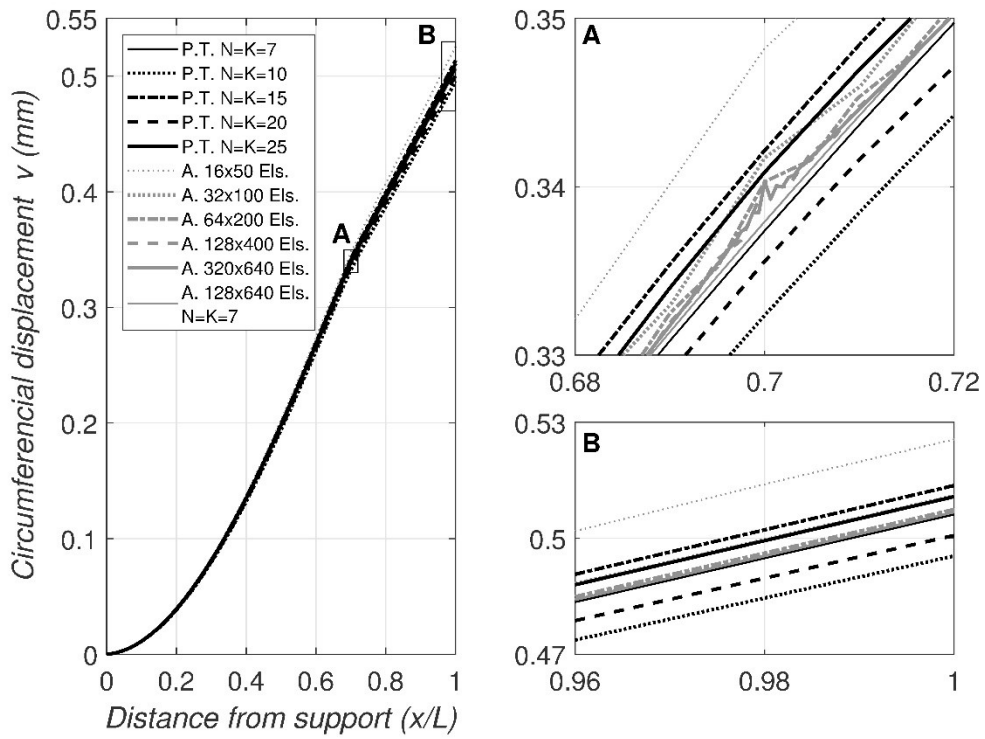
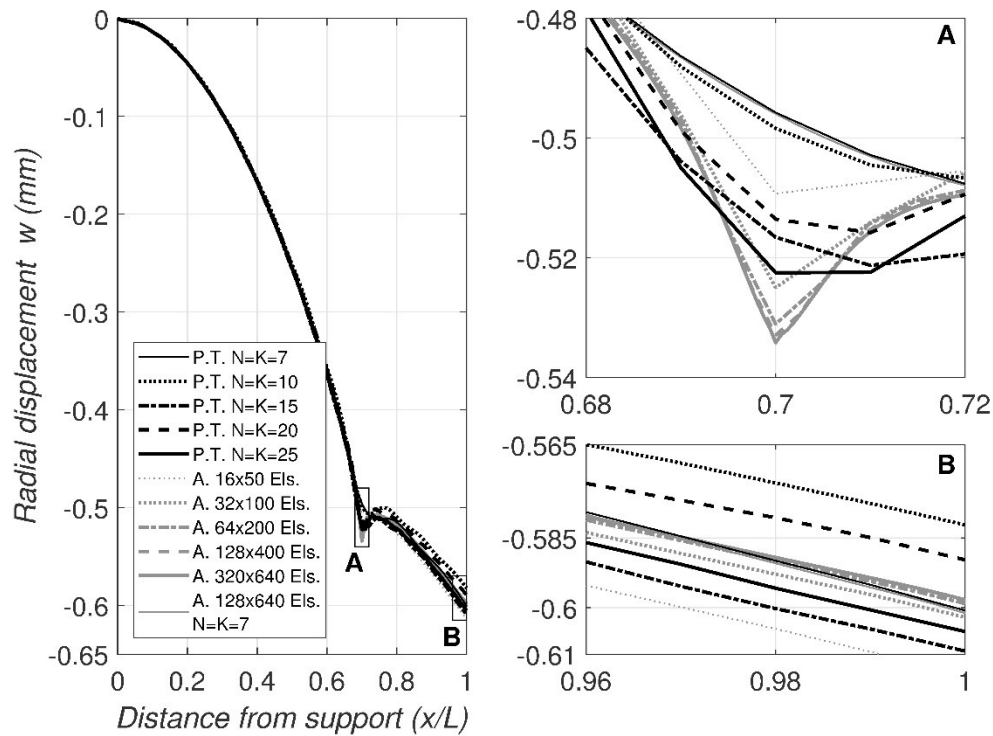


Figure 13 – Comparison of circumferential displacements at top generator

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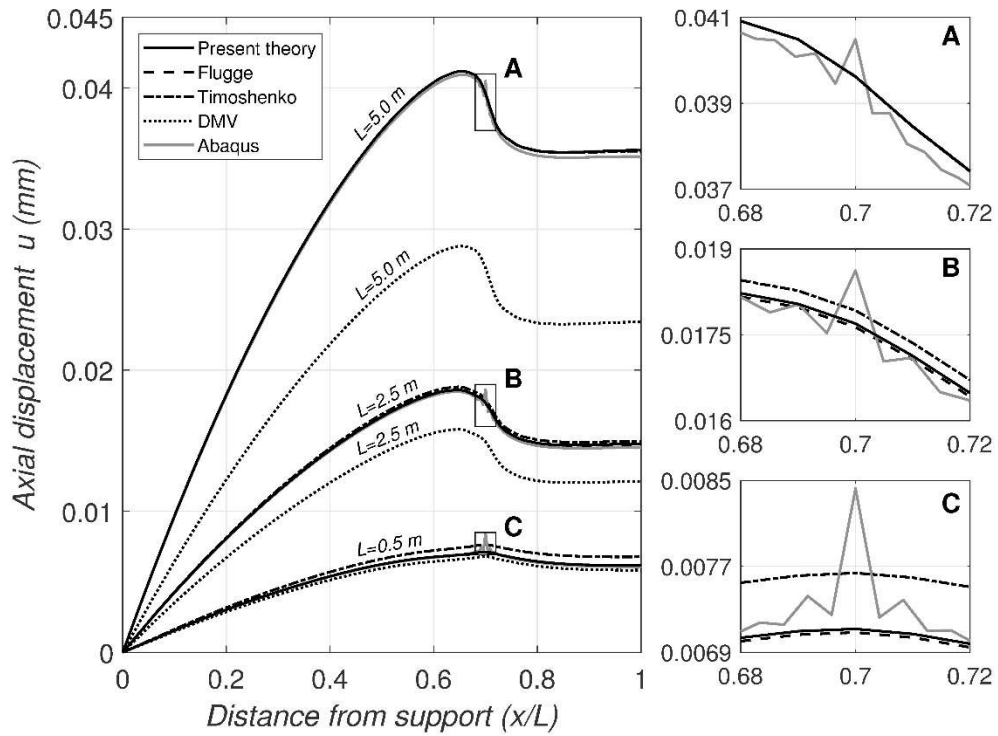
551
552 **Figure 14 – Comparison of radial displacement at top generator**

553 To investigate the effect of span on the results, three spans are considered; $L = 0.5\text{ m}$,
554 $L = 2.5\text{ m}$ and $L = 5\text{ m}$. The displacement components of the top generator ($s = z = 0$) versus
555 the normalized longitudinal coordinate x/L are provided in Figure 15 to Figure 17. The
556 number of modes taken is $N = K = 25$, which is beyond the number of modes required for
557 convergence. Abaqus S4R meshes were based 240 elements in the circumferential direction
558 and 400 elements in longitudinal directions (a 240×400 mesh) for span $L = 0.5\text{ m}$, a
559 128×200 mesh for span $L = 2.5\text{ m}$, and a 128×640 mesh for span $L = 5\text{ m}$).

560 As a general observation, all predictions are in very good agreement with the exception of the
561 DMV theory. In addition, the difference between the responses predicted by the DMV solution
562 and those of the other solutions tend to grow with the pipe span. In all cases, the DMV solution
563 underestimates the displacements compared to other solutions.

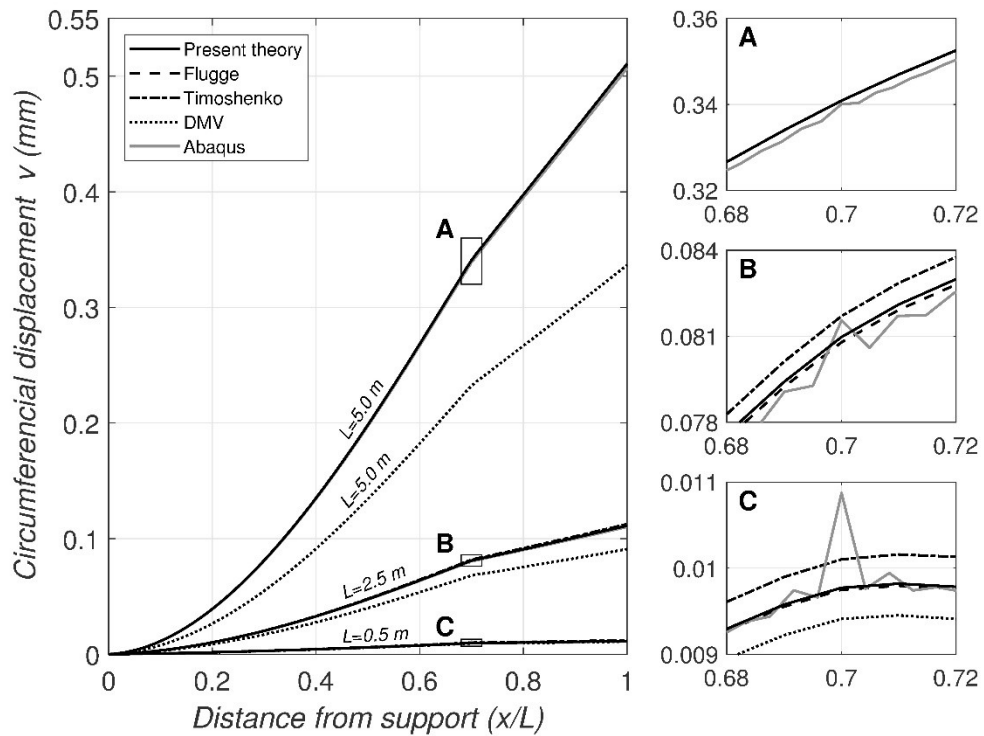
564 The maximum difference between the present solution results and those based on Abaqus are
565 negligible. In general, the present theory predicts a slightly more flexible response compared
566 to that of the Abaqus model with fine meshes. Irrespective of the span, the present theory's

567 predictions are in excellent agreement with Flugge's predictions, while the predictions of the
 568 Timoshenko's theory tend to deviate slightly from those of the two theories for shorter spans.



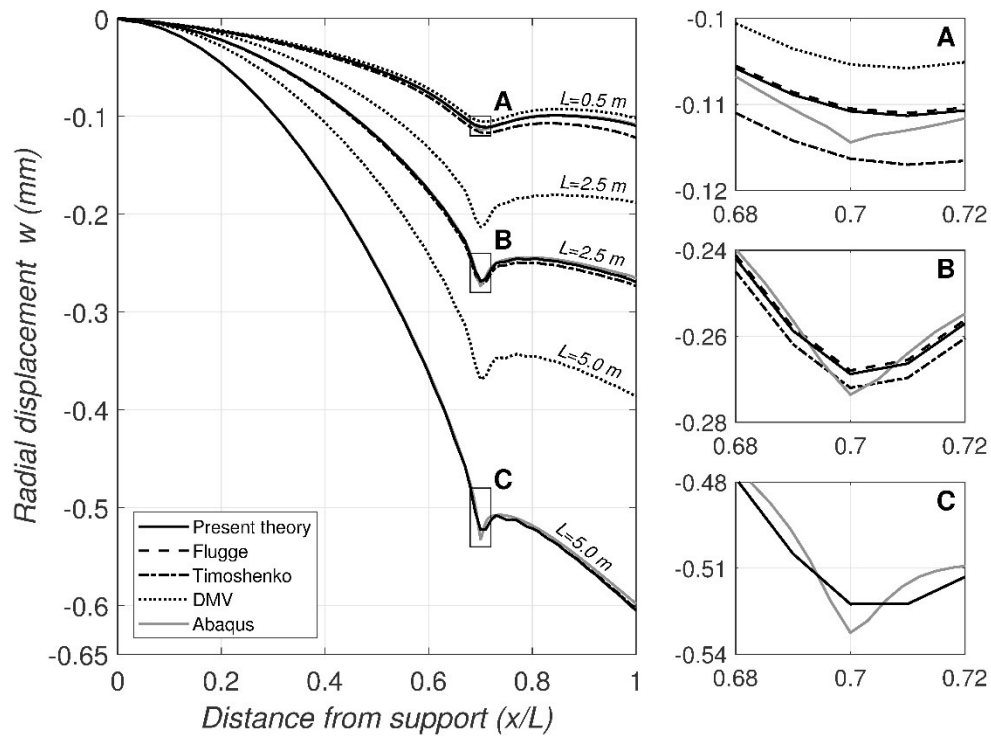
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Figure 15 - Longitudinal displacement at the top generator



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Figure 16 - Circumferential displacement at the top generator



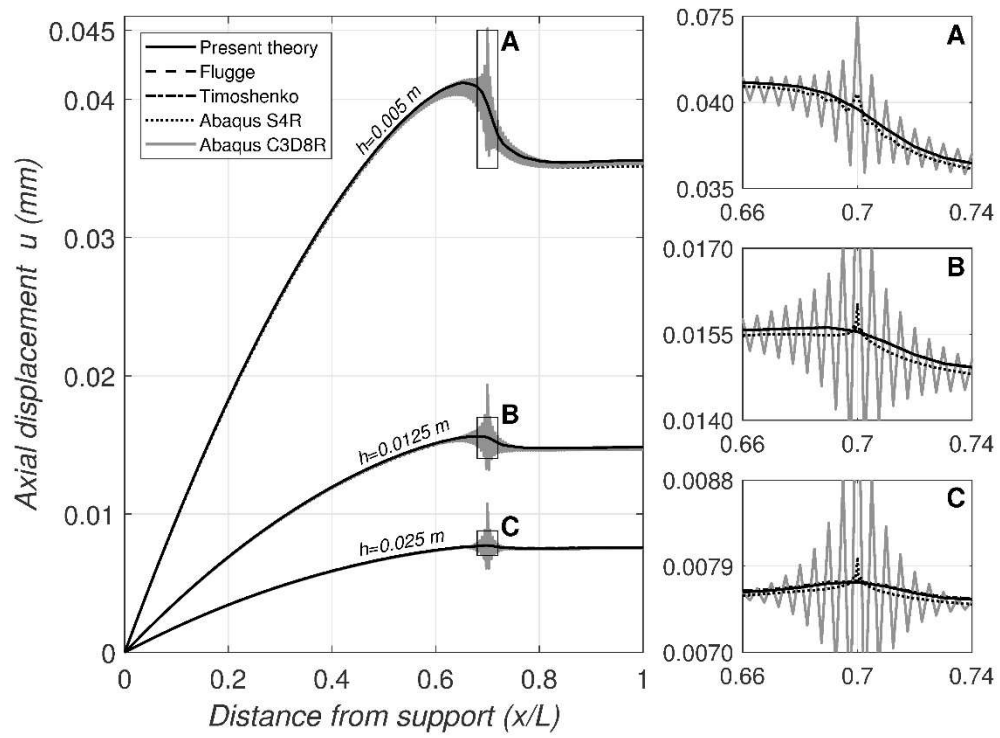
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Figure 17 - Radial displacement at the top generator

576 To investigate the effect of thickness on various theories, the problem defined in the current
 577 example ($L = 5\text{ m}$ and $R = 250\text{ mm}$, with the same point load) is solved again while varying
 578 the thickness to $h = 5.0, 12.5, 25.0\text{ mm}$, which correspond to radius to thickness values of
 579 $R/h = 50, 20, 10$, respectively). The problems are solved using two types of Abaqus models:
 580 one is based on 128×400 S4R Shell elements and the other is based on $4 \times 128 \times 400$ C3D8R
 581 Brick elements.

582 As shown in Figure 18 to Figure 20, predictions of the present theory, Flugge, and Timoshenko
 583 are in near perfect agreement, while Abaqus solutions provide an indistinguishable stiffer
 584 response. As the DMV results were far from the comparable range, they were not included in
 585 the plots. For the thicker pipe, the Timoshenko theory shows a negligibly more flexible
 586 response. In the proximity of the point load application, the finite element solutions exhibit
 587 localized oscillations, particularly for the brick element solution, which contrasts with the
 588 smoothness of the shell responses.



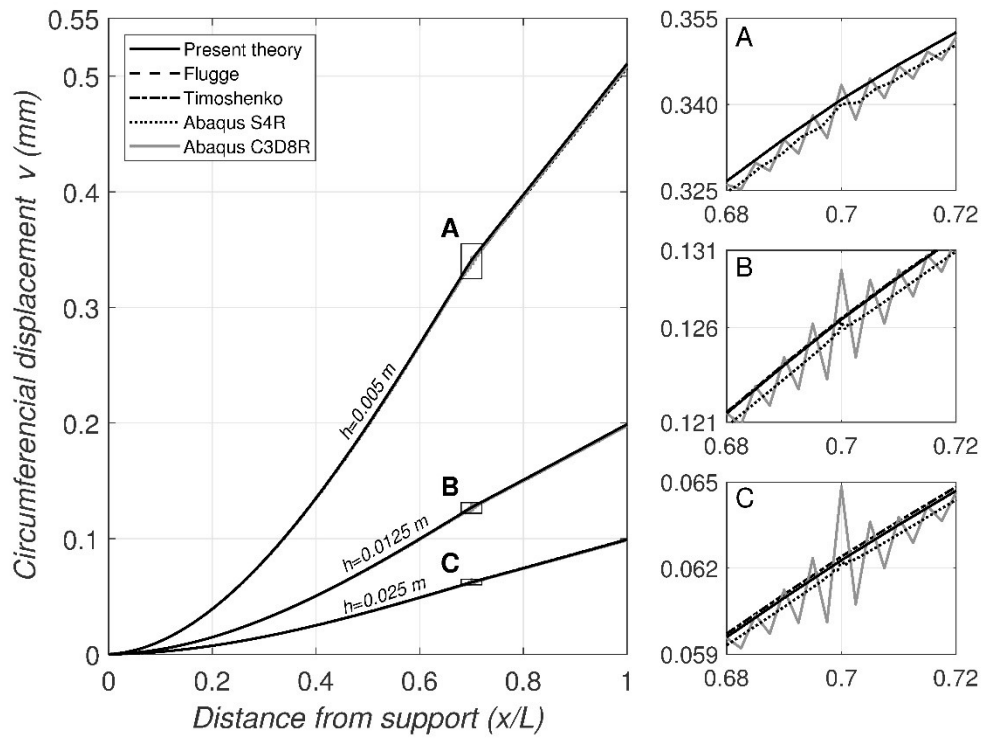
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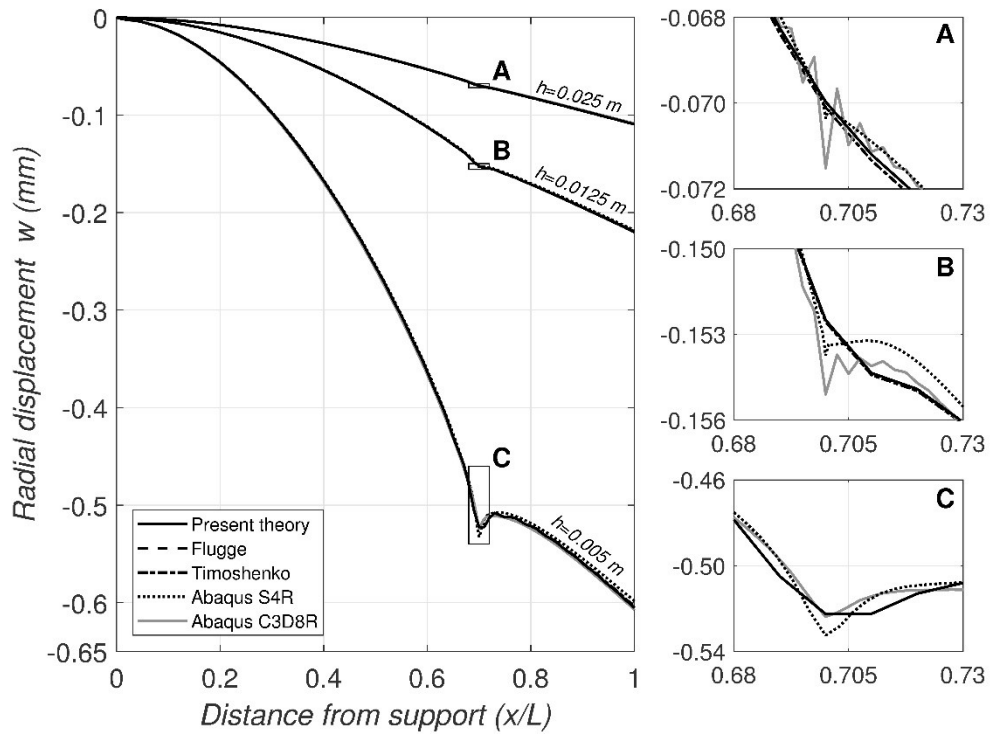
Figure 18 - Longitudinal displacement on the top generator for various theories and different Abaqus elements

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Figure 19 - Circumferential displacement on the top generator for various theories and different Abaqus elements

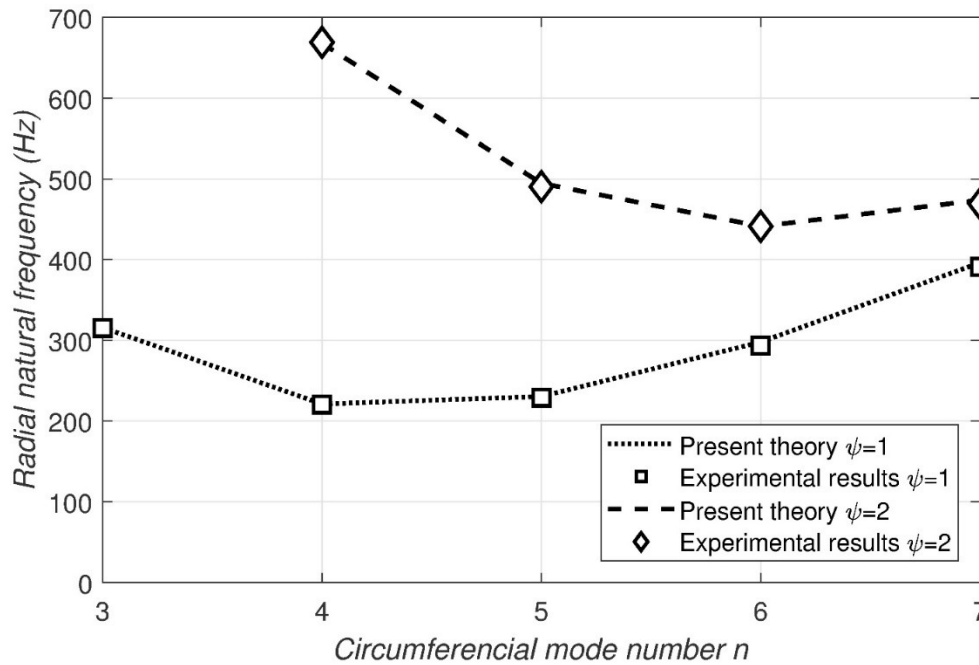


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Figure 20 - Radial displacement on the top generator for various theories and different Abaqus elements

599 **3.9.4 Example 4 – Comparisons against experimental results and**
 600 **analytical predictions**

601 The natural frequencies based on the present theory are compared to the experimental results
 602 reported in [42]. The specimens tested were made of steel ($E = 200\text{ GPa}$, $\rho = 7760\text{ kg/m}^3$,
 603 $\nu = 0.29$) with span $L = 0.664\text{ m}$, radius $R = 175\text{ mm}$, thickness $h = 1.02\text{ mm}$. Figure 21
 604 depicts excellent agreement between the predictions of the present theory and the experimental
 605 results. Symbol n denotes the circumferential wave number and symbol ψ denotes the
 606 longitudinal half-wave number and satisfies the relation $m = \psi\pi/L$. The present theory tends
 607 to predict slightly higher natural frequencies for higher circumferential modes. In all cases,
 608 the maximum difference between the present theory predictions and experimental results is
 609 1.5%.



610
 611

Figure 21 – Comparing natural frequencies

612 Additional comparisons are made against the analytical predictions in [43] which reported the
 613 natural frequencies for a simply supported pipe made of rubber ($E = 0.45\text{ GPa}$, $\nu = 0.45$,
 614 $\rho = 1452\text{ kg/m}^3$), span $L = 0.2\text{ m}$, radius $R = 100\text{ mm}$, and thickness $h = 2\text{ mm}$. Table 5 shows
 615 that, for $\psi = 1, \dots, 8$ and $n = 1, \dots, 10$, the present theory is found to yield natural frequencies
 616 of up to 1.36% lower than the values reported in [43].

Table 5 – Comparing natural frequencies based on the present theory with analytical results

		ψ								
		1	2	3	4	5	6	7	8	
<i>n</i>	1	Present	523.6	782.2	849.8	889.8	938.8	1011.8	1117.4	1260.3
		Ref. [43]	524.1	783.7	852.8	894.8	946.2	1021.9	1130.4	1276.5
		Diff. %	-0.09	-0.19	-0.35	-0.55	-0.78	-0.99	-1.15	-1.27
	2	Present	303.4	607.2	752.4	834.8	907.6	995.5	1111.2	1261.0
		Ref. [43]	303.9	609.1	755.8	840.1	915.3	1005.8	1124.3	1277.3
		Diff. %	-0.17	-0.32	-0.44	-0.64	-0.84	-1.02	-1.17	-1.28
	3	Present	188.1	456.1	641.2	763.7	865.6	973.9	1104.1	1264.0
		Ref. [43]	188.6	458.4	645.2	769.7	873.8	984.6	1117.4	1280.5
		Diff. %	-0.28	-0.50	-0.63	-0.78	-0.94	-1.08	-1.19	-1.28
	4	Present	149.5	355.8	546.7	695.7	823.9	953.6	1100.0	1272.0
		Ref. [43]	150.0	358.2	551.4	702.5	832.9	964.9	1113.8	1288.8
		Diff. %	-0.33	-0.66	-0.85	-0.97	-1.08	-1.17	-1.24	-1.30
	5	Present	167.3	309.9	485.4	645.6	792.9	941.4	1103.4	1287.6
		Ref. [43]	167.6	312.1	490.2	653.0	802.7	953.4	1117.9	1305.0
		Diff. %	-0.21	-0.69	-0.99	-1.13	-1.22	-1.26	-1.29	-1.33
	6	Present	218.4	314.5	463.9	622.9	780.9	943.2	1118.5	1313.7
		Ref. [43]	218.7	316.2	468.4	630.4	791.0	955.8	1133.6	1331.7
		Diff. %	-0.13	-0.53	-0.96	-1.18	-1.28	-1.32	-1.33	-1.35
	7	Present	288.7	358.4	481.9	631.7	792.5	963.3	1148.3	1352.5
		Ref. [43]	289.0	359.7	485.7	638.7	802.6	976.2	1163.9	1371.1
		Diff. %	-0.11	-0.36	-0.79	-1.10	-1.26	-1.32	-1.34	-1.36
	8	Present	372.9	429.2	533.1	670.9	829.4	1003.7	1194.8	1405.6
		Ref. [43]	373.3	430.4	536.3	677.2	839.0	1016.5	1210.7	1424.7
		Diff. %	-0.11	-0.27	-0.60	-0.93	-1.15	-1.26	-1.31	-1.34
	9	Present	469.4	519.1	610.0	736.9	890.3	1064.7	1258.9	1474.0
		Ref. [43]	470.0	520.2	612.8	742.5	899.3	1077.2	1274.8	1493.5
		Diff. %	-0.13	-0.22	-0.45	-0.76	-1.00	-1.16	-1.25	-1.31
	10	Present	577.6	623.9	706.9	825.2	972.9	1145.4	1340.5	1557.9
		Ref. [43]	578.5	625.2	709.6	830.3	981.4	1157.6	1356.4	1577.8
		Diff. %	-0.16	-0.21	-0.38	-0.62	-0.86	-1.05	-1.18	-1.26

618 4. Summary and Conclusions

619 A variational expression is developed for the dynamic analysis of thin shells of general
620 geometries based on tensor calculus in conjunction with the Hamilton variational principle.
621 The solution is based on a minimal number of assumptions at the outset of the formulation
622 while avoiding non-essential approximations advocated in past theories. The variational
623 expression is then specialized to CCTS and used to formulate the governing equilibrium
624 equations and boundary conditions. The governing equations thus obtained are free from non-
625 essential approximations and can potentially serve as a benchmark to assess the accuracy of

626 past circular cylindrical thin shell theories. The field equations derived are then compared to
627 those of Flugge, Timoshenko, Novozhilov, Morley-Koiter, Niordson, Saada and DMV. A
628 number of problems were numerically investigated using the present theory and comparisons
629 were made with the Flugge, Timoshenko, and DMV theories.

630 The main conclusions of the study are:

631 (1) The governing field equations of the Flugge and DMV theories were recovered as special
632 cases from the present theory by applying consistent approximations relating to the radius-to-
633 thickness ratio to the governing equations derived herein.

634 (2) Compared to the present treatment, the theories of Timoshenko, Novozhilov, Morley-
635 Koiter, Niordson, and Saada were found to involve inconsistent approximations. In some
636 cases, such inconsistencies lead to over-simplifications in the field equations, while in others,
637 they lead to the emergence of additional/unnecessary terms.

638 (3) The inconsistent simplifications made in the above theories do not necessarily correspond
639 to a significant reduction in their predictive power. Specifically, the inconsistencies involved
640 in Timoshenko's theory are of a small order and do not significantly influence its predictive
641 ability. For DMV, while the approximations made are of a consistent order, the order of
642 approximation implied happens to be too coarse to capture the response of pipes with moderate
643 thicknesses.

644 (4) Numerical predictions of the 3D and shell FEA solutions are in excellent agreement with
645 those of the present theory for a wide range of pipe dimensions ($2 < L/R < 20$ and
646 $10 < R/h < 50$).

647 (5) The numerical predictions of the Flugge theory are found to be consistently in excellent
648 agreement with those of the present theory and Abaqus finite element solutions, and is thus
649 advocated as a more simplified CCTS theory, but yet as an accurate alternative to the present
650 theory.

651 (6) In contrast, DMV exhibits an overly stiff behaviour compared to the present and other
652 theories. In addition, it grossly overestimates the stresses in some cases. The theory is suitable
653 only for very thin pipes.

654 **Acknowledgement**

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658 (NSERC) to the second author is gratefully acknowledged.

659 5. List of Symbols

\mathbf{a}_i	Covariant base vectors of the middle surface
a_{ij}	First fundamental tensor defined on the middle surface of a thin shell
a	Determinant of the first fundamental tensor a_{ij}
$b_{\alpha\beta}$	Curvature tensor of the surfaces parallel and adjacent to the middle surface
$\bar{b}_{\alpha\beta}$	Second fundamental tensor or curvature tensor defined on the middle surface of a thin shell
\mathbf{a}^i	Contravariant base vectors of the middle surface
a^{ij}	First fundamental tensor of the second kind defined on the middle surface of a thin shell
\mathbf{g}_i	Covariant base vectors of any surface parallel and adjacent to the middle surface
g_{ij}	First fundamental tensor defined within the shell
g	Determinant of the first fundamental tensor g_{ij}
\mathbf{g}^i	Contravariant base vectors of any surface parallel and adjacent to the middle surface
g^{ij}	First fundamental tensor of the second kind defined within the shell
p^x, p^y, p^z	Physical components of the Contravariant body force components F^i as a function of (x, s, z, t)
$\bar{p}^x, \bar{p}^y, \bar{p}^z$	Physical components of the Contravariant body force components F^i as a function only of (x, s, z) .
\mathbf{R}, \mathbf{r}	Position vector of a point within the shell and on the middle surface, respectively, before deformation.
$\hat{\mathbf{R}}, \hat{\mathbf{r}}$	Position vector of a point within the shell and on the middle surface, respectively, after deformation.
\mathbf{U}, \mathbf{u}	Displacement of a point within the shell and on the middle surface, respectively.
U_i, U^i	Covariant and contravariant components of the displacement of a point on a surface parallel and adjacent to the middle surface along axis x^i , respectively.
u_i	Covariant components of the displacement of a point on the middle surface along axis x^i
u, v, w	Components of the displacement of a point on the middle surface along axes x^1, x^2 and x^3 , respectively
χ^i	Coordinate system/Coordinates of the system
x^i	Coordinate system/Coordinates of the system
x, s, z	Coordinates of the system, equivalent to x^1, x^2 and x^3 , respectively
α, α, \dots	All Greek superscripts or subscripts range from 1 to 2
i, i, \dots	All Italic superscripts or subscripts range from 1 to 3

$\bar{\mathbf{i}}_i, \bar{\mathbf{j}}_i$	Covariant unit vectors along axis x^i in middle surface and adjacent surfaces, respectively
$\bar{\mathbf{i}}^i, \bar{\mathbf{j}}^i$	Contravariant unit vectors along axis x^i in middle surface and adjacent surfaces, respectively
Γ_{ij}^k	Christoffel symbols in terms of g_i and g_{ij}
$\bar{\Gamma}_{ij}^k$	Christoffel symbols in terms of a_i and a_{ij} (defined for the middle surface)
\cdot	Inner product
\otimes	Cross product
$ $	Covariant derivative
$\ $	Covariant derivative in two dimensional spaces
T^*	Kinetic energy
V^*	Potential energy
V	Volume
W^*	External work
Π	Mechanical energy of the system
ε_{ij}	Components of the strain tensor on the middle surface
η_{ij}	Components of the strain tensor on surfaces adjacent to the middle surface
$\eta_{\alpha\beta}$	In-plane components of the strain tensor
$\tilde{\eta}_{\alpha\beta}$	Physical components of $\eta_{\alpha\beta}$
σ^{ij}	Stress tensor
$\sigma^{\alpha\beta}$	In-plane components of the stress tensor σ^{ij}
Ψ	Longitudinal half-wave number
E^{ijlm}	Generalized Hooke's Law
$E^{\alpha\beta\lambda\delta}$	In-plane components of the generalized Hooke's Law E^{ijlm}
$\cdots_{,i}$	Partial derivative with respect to x^i
δ_{β}^{α}	Kronecker delta function
μ_{β}^{α}	Mixed shifter tensor connecting the covariant base vectors of an arbitrary point within the shell to corresponding base vectors of the middle surface.
$\lambda_{\alpha}^{\gamma}$	Inverse of mixed shifter tensor μ_{β}^{α}
ρ	Density of the mass
E	Modulus of elasticity
ν	Poisson's ratio
t	time

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663 6. **References**

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$$\begin{aligned}
& {}^1_1\bar{H} = -1 \quad , \quad {}^1_2\bar{H} = -\nu \quad , \quad {}^1_3\bar{H} = -\nu/R \quad , \quad {}^1_4\bar{H} = h^2/12R \\
& {}^2_1\bar{H} = -(1-\nu)/2 \quad , \quad {}^2_2\bar{H} = -(1-\nu)(1+h^2/4R^2)/2 \quad , \quad {}^2_3\bar{H} = (1-\nu)h^2/8R \\
& {}^3_1\bar{H} = -h^2/12R \quad , \quad {}^3_2\bar{H} = (1-\nu)R(\Phi-1)/2 \quad , \quad {}^3_3\bar{H} = -(3-\nu)h^2/24R \\
& {}^4_4\bar{H} = h^2/12 \quad , \quad {}^3_5\bar{H} = [(3-\nu)h^2/24 + (1-\nu)R^2(\Phi-1)/2] \\
& {}^6_6\bar{H} = \rho(1-\nu^2)h^2/12RE \quad , \quad {}^7_7\bar{H} = -\rho(1-\nu^2)h^2/12E \\
& {}^8_8\bar{H} = (1-\nu^2)/Eh \\
& {}^4_1\bar{H} = h^2/12R \quad , \quad {}^4_2\bar{H} = \nu h^2/12R \quad , \quad {}^4_3\bar{H} = -h^2/12 \\
& {}^4_4\bar{H} = -\nu h^2/12 \\
& {}^1_1\bar{G} = 1 \quad , \quad {}^1_2\bar{G} = (1-\nu)\Phi/2 \quad , \quad {}^1_3\bar{G} = (1+\nu)/2 \\
& {}^4_4\bar{G} = \nu/R \quad , \quad {}^1_5\bar{G} = -h^2/12R \\
& {}^6_6\bar{G} = (1-\nu)R(\Phi-1)/2 \quad , \quad {}^7_7\bar{G} = -\rho(1-\nu^2)/E \quad , \\
& {}^8_8\bar{G} = \rho(1-\nu^2)h^2/12RE \quad , \quad {}^9_9\bar{G} = (1-\nu^2)/Eh \\
& {}^2_1\bar{G} = (1+\nu)/2 \quad , \quad {}^2_2\bar{G} = (1-\nu)(1+h^2/4R^2)/2 \quad , \quad {}^2_3\bar{G} = 1 \\
& {}^2_4\bar{G} = 1/R \quad , \quad {}^2_5\bar{G} = -(3-\nu)h^2/24R \quad , \quad {}^2_6\bar{G} = 0 \\
& {}^2_6\bar{G} = -\rho(1-\nu^2)(1+h^2/12R^2)/E \quad , \quad {}^2_7\bar{G} = \rho(1-\nu^2)h^2/6RE \quad , \\
& {}^2_8\bar{G} = (1-\nu^2)/Eh \\
& {}^3_1\bar{G} = -\nu/R \quad , \quad {}^3_2\bar{G} = -1/R \quad , \quad {}^3_3\bar{G} = -\Phi/R^2 \\
& {}^4_4\bar{G} = -2(\Phi-1) \quad , \quad {}^3_5\bar{G} = h^2/12R \\
& {}^3_6\bar{G} = -(1-\nu)R(\Phi-1)/2 \quad , \quad {}^3_7\bar{G} = (3-\nu)h^2/24R \quad , \quad {}^3_8\bar{G} = 0 \\
& {}^3_8\bar{G} = -h^2/12 \quad , \quad {}^3_9\bar{G} = -[(3+\nu)h^2/24 + (1-\nu)R^2(\Phi-1)/2] \\
& {}^{3}_{10}\bar{G} = -R^2(\Phi-1) \quad , \quad {}^{3}_{11}\bar{G} = -\rho(1-\nu^2)/E \\
& {}^{3}_{12}\bar{G} = \rho(1-\nu^2)h^2/12E \quad , \quad {}^{3}_{13}\bar{G} = \rho(1-\nu^2)h^2/12E \\
& {}^{3}_{14}\bar{G} = -\rho(1-\nu^2)h^2/12RE \quad , \quad {}^{3}_{15}\bar{G} = -\rho(1-\nu^2)h^2/6RE \\
& {}^{3}_{16}\bar{G} = -(1-\nu^2)/Eh \quad , \quad {}^{3}_{17}\bar{G} = -(1-\nu^2)/Eh \\
& {}^{3}_{18}\bar{G} = (1-\nu^2)/Eh
\end{aligned}$$

757

758 where $\Phi = \Phi(h/R) = (R/h) \ln[(2+h/R)/(2-h/R)]$

Appendix B - Force terms, Boundary Conditions and Field Equations

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760

761 The terms $f^{xu}, f^{xw}, f^{sv}, f^{sw}, f^{zw}$ are related to the physical force components p^x, p^s, p^z per
 762 unit volume (of the contravariant external force components F^i , respectively) through the
 763 integrals

$$764 \quad \begin{Bmatrix} f^{xu} \\ f^{sv} \\ f^{zw} \\ f^{xw} \\ f^{sw} \end{Bmatrix} = \int_{-\frac{h}{2}}^{+\frac{h}{2}} \begin{Bmatrix} p^x (1+z/R) \\ p^s (1+z/R)^2 \\ p^z (1+z/R) \\ p^x z (1+z/R) \\ p^s z (1+z/R) \end{Bmatrix} dz \quad (\text{B.1a-e})$$

765 In Eq. 25, since t_1 and t_2 are arbitrary times and the variations $\delta u, \delta v, \delta w$ and $\delta w_{,1}$ are
 766 arbitrary within the domain ($0 < x < L$ and $0 < s < 2\pi R$), the first and second integrals have
 767 to vanish independently. The former integral yields eight boundary conditions,

$$768 \quad \begin{aligned} & \left[\left({}^1\bar{H}u_{,1} + {}^2\bar{H}v_{,2} + {}^3\bar{H}w + {}^4\bar{H}w_{,11} \right) \delta u \right]_0^L = 0 \\ & \left[\left({}^2\bar{H}u_{,2} + {}^2\bar{H}v_{,1} + {}^3\bar{H}w_{,12} \right) \delta v \right]_0^L = 0 \\ & \left[\left({}^3\bar{H}u_{,11} + {}^3\bar{H}u_{,22} + {}^3\bar{H}v_{,12} + {}^3\bar{H}w_{,111} + {}^3\bar{H}w_{,122} + {}^3\bar{H}\ddot{u} + {}^3\bar{H}\ddot{w}_{,1} + {}^3\bar{H}f^{xw} \right) \delta w \right]_0^L = 0 \\ & \left[\left({}^4\bar{H}u_{,1} + {}^4\bar{H}v_{,2} + {}^4\bar{H}w_{,11} + {}^4\bar{H}w_{,22} \right) \delta w_{,1} \right]_0^L = 0 \end{aligned} \quad (\text{B.2})$$

769 in which either the bracketed expressions vanish, yielding the natural boundary conditions, or
 770 their respective coefficient does, yielding the essential boundary conditions, and the second
 771 integral of Eq. 25 yields the following three equilibrium equations.

$$772 \quad \begin{aligned} & \left({}^1\bar{G}u_{,11} + {}^2\bar{G}u_{,22} + {}^3\bar{G}v_{,12} + {}^4\bar{G}w_{,1} + {}^5\bar{G}w_{,111} + {}^6\bar{G}w_{,122} + {}^7\bar{G}\ddot{u} + {}^8\bar{G}\ddot{w}_{,1} + {}^9\bar{G}f^{xu} \right) = 0 \\ & \left({}^2\bar{G}u_{,12} + {}^2\bar{G}v_{,11} + {}^3\bar{G}v_{,22} + {}^4\bar{G}w_{,2} + {}^5\bar{G}w_{,112} + {}^a\bar{G}w_{,222} + {}^6\bar{G}\ddot{v} + {}^7\bar{G}\ddot{w}_{,2} + {}^8\bar{G}f^{sv} \right) = 0 \\ & \left({}^3\bar{G}u_{,1} + {}^3\bar{G}v_{,2} + {}^3\bar{G}w + {}^a\bar{G}w_{,11} + {}^4\bar{G}w_{,22} + {}^5\bar{G}u_{,111} + {}^6\bar{G}u_{,122} + {}^7\bar{G}v_{,112} + {}^b\bar{G}w_{,222} \right. \\ & \quad \left. + {}^8\bar{G}w_{,1111} + {}^9\bar{G}w_{,1122} + {}^{10}\bar{G}w_{,2222} + {}^{11}\bar{G}\ddot{w} + {}^{12}\bar{G}\ddot{w}_{,11} + {}^{13}\bar{G}\ddot{w}_{,22} + {}^{14}\bar{G}\ddot{u}_{,1} + {}^{15}\bar{G}\ddot{v}_{,2} \right. \\ & \quad \left. + {}^{16}\bar{G}f^{xw}_{,1} + {}^{17}\bar{G}f^{sw}_{,2} + {}^{18}\bar{G}f^{zw} \right) = 0 \end{aligned} \quad (\text{B.3})$$