

COMPUTER SIMULATION OF
TRICKLING FILTER PLANT OPERATION

by

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ABSTRACT

The extensive review of the literature on the trickling filter process revealed that four basic approaches had been adopted in the performance evaluation of this particular wastewater treatment process, namely the graphical, first order reaction, statistical, and mechanistic approaches. Of these four techniques, the statistical regression approach proposed by Galler and Gotaas in 1964 has received much attention within the last years, and treatment plant performance evaluation using such concepts is obviously justified.

The high rate trickling filter plant of Alexandria, Virginia, U.S.A., consisting of primary settlers, followed by a parallel arrangement of three trickling filter units, and secondary settlers was adopted for computer performance simulation. Seven years of field data, constituting a total of eighty-four monthly average observations formed the basis for the performance evaluation of this high rate trickling filter plant.

Linear and non linear regression algorithms were evaluated so as to best describe both the physical and biological performance of the treatment facility. The multiple linear regression technique was finally adopted because the response of the intrinsically linear models was as representative as that obtained through non linear regression techniques. Simulation of all major treatment units within the treatment

plant was performed, evaluating both BOD and suspended solids removal efficiencies.

Two subsystem arrangements were examined: a first subsystem was analysed taking into consideration the trickling filter and final settling tank as a single treatment unit, while a second subsystem arrangement was examined by considering the trickling filter and final settling tank as two distinct treatment units.

A total of eight mathematical expressions resulted from the present investigation. High multiple correlation coefficients (R) were achieved, ranging from 0.9184 to 0.9779, thus giving an indication of the goodness of fit of the field data.

The arrangement in series of the mathematical expressions developed, subjected to a convergence criterion in the recirculation of the trickling filter effluent, essentially describes the performance of the wastewater treatment plant. A computer program written in Fortran IV, and referred to as CSTF, performs the required mathematical manipulations for computer performance evaluation.

A sensitivity analysis of the sewage characteristics of the wastewater treatment plant as a whole, and of the treatment units individually is also presented. The simulation program is supplied with an input matrix describing maximum, minimum, and average expected values for primary influent BOD, suspended solids, flow, recirculation,

and sewage temperature. Changes in the performance of the treatment plant while subjected to input parameter variations are also reported and analysed. In addition, the input matrix also takes into consideration weather flow variations, such as dry and wet weather flow conditions. Of the input conditions examined, variations in the suspended solids concentration and in the weather flow conditions, affected most significantly the BOD and suspended solids concentration of the plant effluent.

A sludge production sensitivity analysis was also performed. The analysis indicated that secondary sludge production accounted for 10 to 15% of the total sludge to be disposed, while primary sludge made up the remaining 85 to 90%. Variations in the suspended solids concentrations, in the rates of flow and in the weather flow conditions, produced the greatest variations in the total sludge production of the treatment plant.

The performance of the Alexandria sewage treatment plant was also compared with other mathematical models. The mathematical formulations of Eckenfelder and Galler and Gotaas were however observed to disagree in some instances with predicted plant performance, but could still be considered as the most representative models from the ones analysed.

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LIST OF SYMBOLS AND ABBREVIATIONS

A_p	specific surface area of filter media in ft^2/ft^3
a	coefficient equal to 0.079 in the model of Schulze
a_0	temperature conversion model coefficient equal to 41.96217
a_1	temperature conversion model coefficient equal to 0.18005
a_2	temperature conversion model coefficient equal to 0.26993
α	probability of making a Type I error while testing statistical hypothesis
ASA	Alexandria Sanitation Authority
b	$= 1.035^{(T_c - 20)}$
b_i 's	approximations of the true value of B_i 's for $i=0, \dots, n^*$
b_o^*	coefficient equal to $\ln B_o$
B_i 's	general coefficients for $i=0, \dots, n^*$
β	probability of making a Type II error in the testing of statistical hypotheses
BMD	computer package developed at the University of California at Los Angeles
BOD	Biochemical Oxygen Demand
c	parameter defined by the series $1 + (1-f) + (1-f)^2 + \dots$
c^*	coefficient in Eckenfelder's model equal to 2.5
C_{D-X}	unitless parameter function of the coefficient of biosorption
C_{O_2}	oxygen concentration
d	coefficient equal to 0.67 in the model of Schulze
D	depth of filter media in feet
D_L	oxygen diffusivity through the film thickness

Δ_{L_r}	removable BOD actually removed, in pounds per unit area
E_1	fractional efficiency of BOD removal for the first stage trickling filter system
E_2	fraction efficiency of BOD removal for the second stage trickling filter system
e_i	residual
e_i^*	transformation of the residual equal to $\ln e_i$
f	coefficient of biosorption
F	factor reflecting increased BOD removal due to recirculation $F = (1+r_1)/(1+r_1/10)^2$
h	aerobic film thickness in centimeters
HR	hydraulic rate of flow for the ASA sewage treatment plant models in MGAD
k	Thériault's proportionality constant
K	Velz's rate of removal constant (base e)
K'	Velz's rate of removal constant (base 10)
K^*	Roesler and Smith rate constant equal to 0.69
K_E	Eckenfelder's modified rate constant
K_H	Howland's rate constant
K_r	unit oxygen utilization rate
K_{20}	experimentally established rate constant
L	organic BOD concentration
L'	removable BOD applied in pounds per day per unit area
L^*	total removable fraction of BOD in mg/l
L_D^*	removable BOD remaining at depth D of the filter media in mg/l
L_e	BOD of the filter system effluent in mg/l

L_{el}	BOD of the effluent from the trickling filter system in pounds per acre per day
L_o	BOD applied to the filter in mg/l
L_{ol}	BOD applied to the filter in pounds per acre per day
L_{pe}	BOD leaving the primary tank in pounds per day
L_{po}	BOD applied to the primary tank in pounds per day
L_{se}	BOD load leaving the secondary settling tank in pounds per day per 1000 cubic foot
L_{so}	BOD load entering the secondary settling tank in pounds per day
L_{te}	BOD load leaving the trickling filter unit in pounds per day per 1000 cubic foot
L_{to}	BOD load applied to the trickling filter unit in pounds per day per 1000 cubic foot
L_{sel}	BOD load leaving the secondary settling tank in pounds per day
m	coefficient in Eckenfelder's model equal to 0.33
m'	coefficient in the model of Roesler and Smith equal to 1.0
m_1	value appearing to reflect the kinetic properties of the waste entering a first stage trickling filter system
m_2	value appearing to reflect the kinetic properties of the waste entering the second stage of a trickling filter system
n	coefficient in Eckenfelder's model equal to 0.5
n'	coefficient in the model of Roesler and Smith equal to 0.67
n^*	number of observations used in the regression analysis
n_1	value equal to $m_1/(m_1-1)$
n_2	value equal to $m_2/(m_2-1)$

OR ₁	primary settling tank overflow rate in GPD/ft ²
OR ₂	secondary settling tank overflow rate in GPD/ft ²
P	number of parameters within a model
ϕ	sum of squares of the deviations
Q	hydraulic rate of flow in MGAD
r ₁	ratio of recirculated flow to influent flow
r ₂	second stage recirculation ratio
R	multiple correlation coefficient
R _*	ratio of the flow through the filter to the flow entering the treatment plant
R ²	coefficient of determination
R of R	Range of Residuals
s	standard error of the estimate
$\overline{s_D}$	standard error of the mean difference
$\tilde{s_D}$	standard deviation of the mean difference
S	saturation capacity of a unit volume in pounds per unit volume
s ²	residual mean square
σ^2	standard deviation
SS _{po}	suspended solids load applied to the primary settling tank in pounds per day
SS _{pe}	suspended solids load leaving the primary settling tank in pounds per day
SS _{se}	suspended solids load leaving the secondary settling tank in pounds per day per thousand cubic foot
SS _{sel}	suspended solids load leaving the secondary settling tank in pounds per day
SS _{so}	suspended solids load applied to the secondary settling tank in pounds per day
SS _{te}	suspended solids load leaving the trickling filter unit in pounds per day per thousand cubic foot

SS_{to}	suspended solids load applied to the trickling filter unit in pounds per day per thousand cubic foot
t	time parameter
t_H	time of travel within the filter media in minutes
't'	t Test Parameter
T_c	sewage temperature in degree Centigrade
T_f	sewage temperature in degree Fahrenheit
T_{ai}	observed average monthly air temperature for month i
$Ta(i-1)$	observed average monthly air temperature for month i-1
T_{si}	predicted average monthly sewage temperature for month i
UND	Unit Normal Deviate
V	volume of filter media in acre-foot
W	BOD loading of settled raw sewage in pounds per day (exclusive of recirculation)
W'	BOD loading in pounds per day applied to a second stage trickling filter system
X	number of saturated unit depths
X_i	independent variable
Y_i	true value of the dependent variable
$\hat{Y}_i$	predicted value of the dependent variable

CHAPTER 1

INTRODUCTION

1.1 Historical Background of the Trickling Filter Process

Trickling filters, also referred to as biological filters and percolating filters, evolved from a nineteenth century England practice of treating domestic sewage. The process is different however from present practices; it consisted in passing settled sewage onto areas of land, using the soil as a media for biological activity (1).

Modifications to the above process led the Lawrence Experiment Station of the Massachusetts State Board of Health in 1888 to the concept of treating settled sewage by using a bed of porous media such as gravel, broken rock, clinker or slag. This form of treatment was soon very popular with municipal officials (1,2).

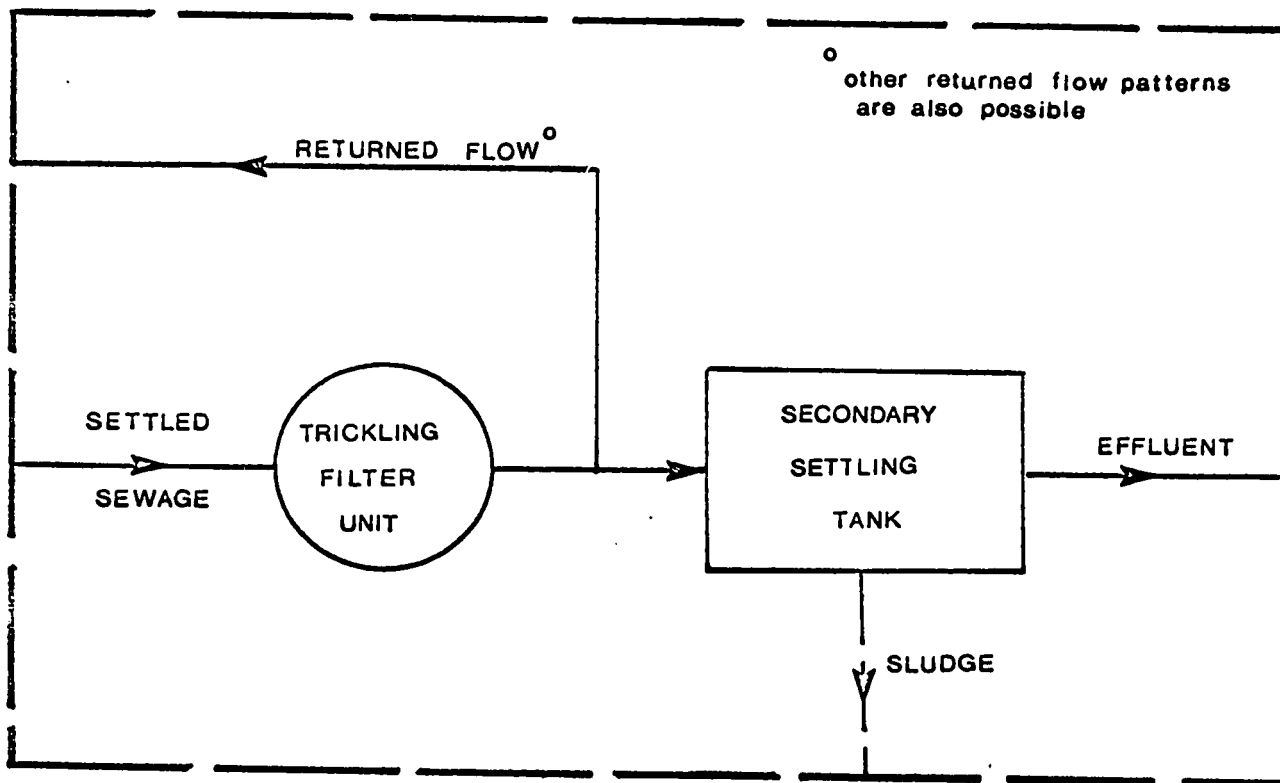
Although trickling filters, as they are known today originated in the United States, English engineers are reported to have developed many of the technical features of the process, such as: i) wastewater distribution by spray, and ii) the use of false bottom floors to support the media, thereby eliminating clogging of the filter floor (2).

In 1903, this form of treatment was used for the first time in the United States to treat sewage on a municipal level. Even if today the treatment of domestic sewage using trickling filter systems is less popular than the activated sludge process, the process itself still receives much attention from all levels, mainly because of some of the following advantages: i) ease of operation, ii) resistance to shock loads, iii) rapid recovery when disturbed, iv) ideal process for treating wastewaters that present a sludge bulking problem, and v) the introduction of plastic filter media, allowing the application of higher hydraulic and organic loads. Obviously the system does also present serious disadvantages, namely: i) high capital investments, ii) large land requirements, iii) odors and iv) filter fly breeding (Psychoda) (2).

1.2 The Trickling Filter Process - Physical and Biological

The trickling filter process usually consists of a trickling filter unit followed by a secondary settling tank, as illustrated schematically in Figure 1.1. The filter unit itself, usually circular but sometimes rectangular, consists of a bed of stone, slag, gravel, coke, or plastic media, varying in depth from three to ten feet (3). The stone or rock media accounts presently for over 75% of the municipal trickling filter installations in the United

FIGURE 1.1 TRICKLING FILTER SYSTEM



States (4). The grading of the stone media ranges commonly from one and a half to four inches. The available filter media (stone, plastic) provide a ratio of surface area per unit volume, ranging from fifteen to thirty square feet per cubic foot, with a void ratio close to 50% for the stone media, and 95% for the plastic media (1).

Trickling filters with plastic media are increasing in popularity because they tend to offer more flexibility, namely deeper bed depths, larger surface area per unit volume, and less chance of clogging due to increased aeration and void space. However, the effluent is generally of lower quality making it a roughing filter as opposed to a polishing filter (5).

The rate of flow onto a filter, commonly referred to as the hydraulic loading, and the amount of organic matter applied, defined as the organic loading, are commonly used for the classification of either standard or high rate trickling filter installations. Table 1.1 adopted from the Water Pollution Control Federation, Manual of Practice No. 8 (6) illustrates the basic differences between these two popular types of trickling filter systems.

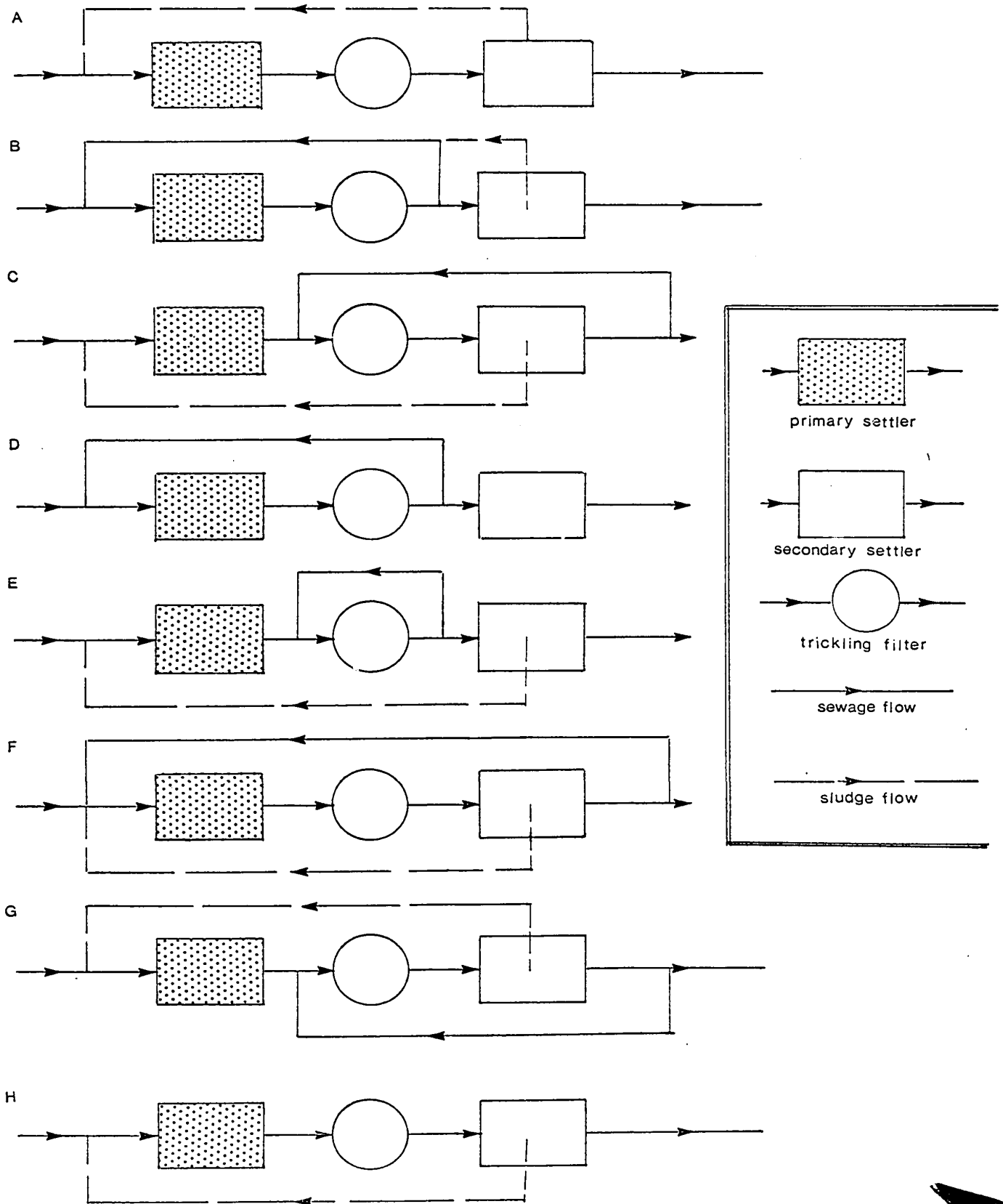
Figure 1.2 illustrates diagrammatically the most popular physical arrangements for single stage trickling filter systems. This illustrates both the flexibility of the process along with the inability of engineers to isolate the most effective installation. Similar flow pattern

TABLE 1.1

Trickling Filter Classification

Type of trickling Filter installation	Hydraulic Load	Organic Load
Standard Rate	1.1 to 4.4 MGAD or 25 to 100 GPD/ft ²	5 to 25 #/D/ 1000 ft ³ 220 to 1100 #/D/acre ft
High Rate	8.7 to 44.0 MGAD 200 to 1000 GPD/ ft ²	25 to 300 #/D/1000 ft ³ 1100 to 13000 #/D/acre ft

FIGURE 1.2 SINGLE STAGE TRICKLING FILTER FLOW PATTERNS

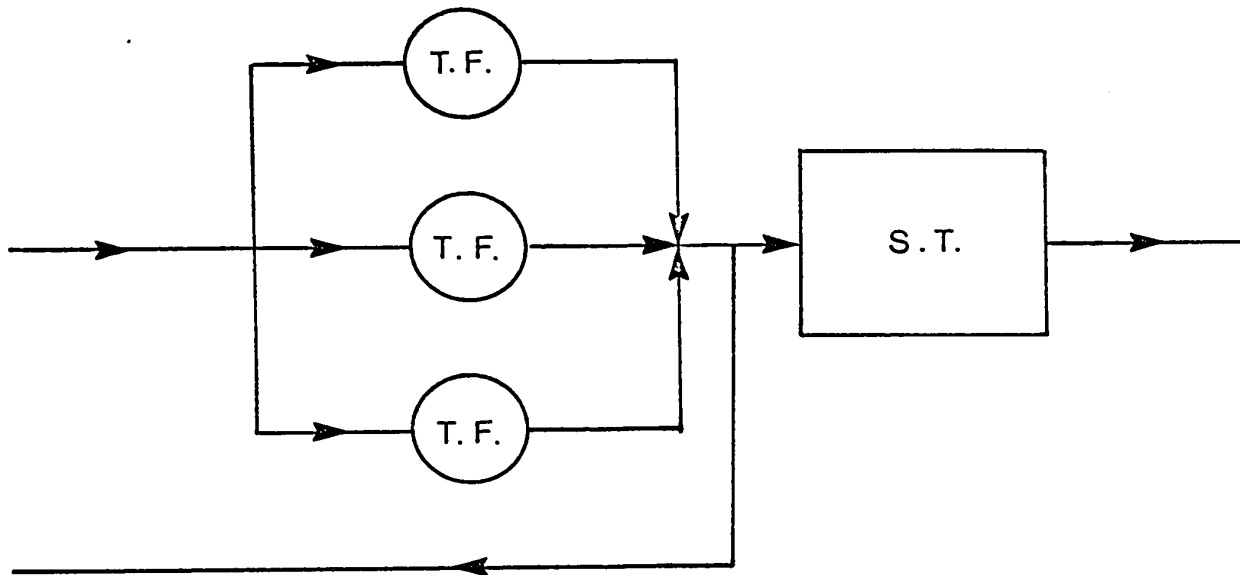


variations also apply to the two-stage biofiltration installations.

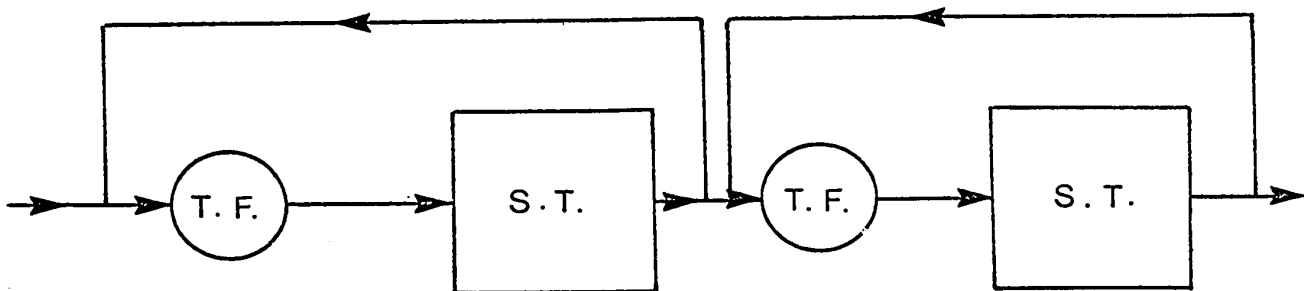
Two or more trickling filter units in parallel, followed by a secondary settler, are termed 'parallel single stage installations,' while two trickling filter systems in series are termed 'double or two-stage trickling filter installations.' Typical arrangements are presented in Figure 1.3. Figure 1.4, illustrates a cutaway view of a stone media trickling filter unit, thus defining the physical components of the treatment unit.

The principles involved in the removal of organic matter through biological filtration are similar to those of the activated sludge. Both the trickling filter and the activated sludge systems depend on the bio-chemical oxidation of complex organic matter from the settled sewage (6). Thus in a trickling filter treatment unit, the settled sewage from the primary settling tank, is sprayed through the air onto the surface of the filter media after which it trickles or percolates through the voids undergoing biological oxidation while in contact with the aerobic portion of the slime or film layer attached to the filter media. Figure 1.5 illustrates the biological oxidation mechanism of the waste by the slime layer. The slime layer, depending on its thickness, usually consists of aerobic and anaerobic sublayers. The microorganisms in the aerobic sublayer use

FIGURE 1.3 PARALLEL AND TWO-STAGE TRICKLING
FILTER INSTALLATIONS



a) Parallel trickling filter system



b) Two-stage trickling filter system

FIGURE 1.4 SECTION OF A TRICKLING FILTER UNIT

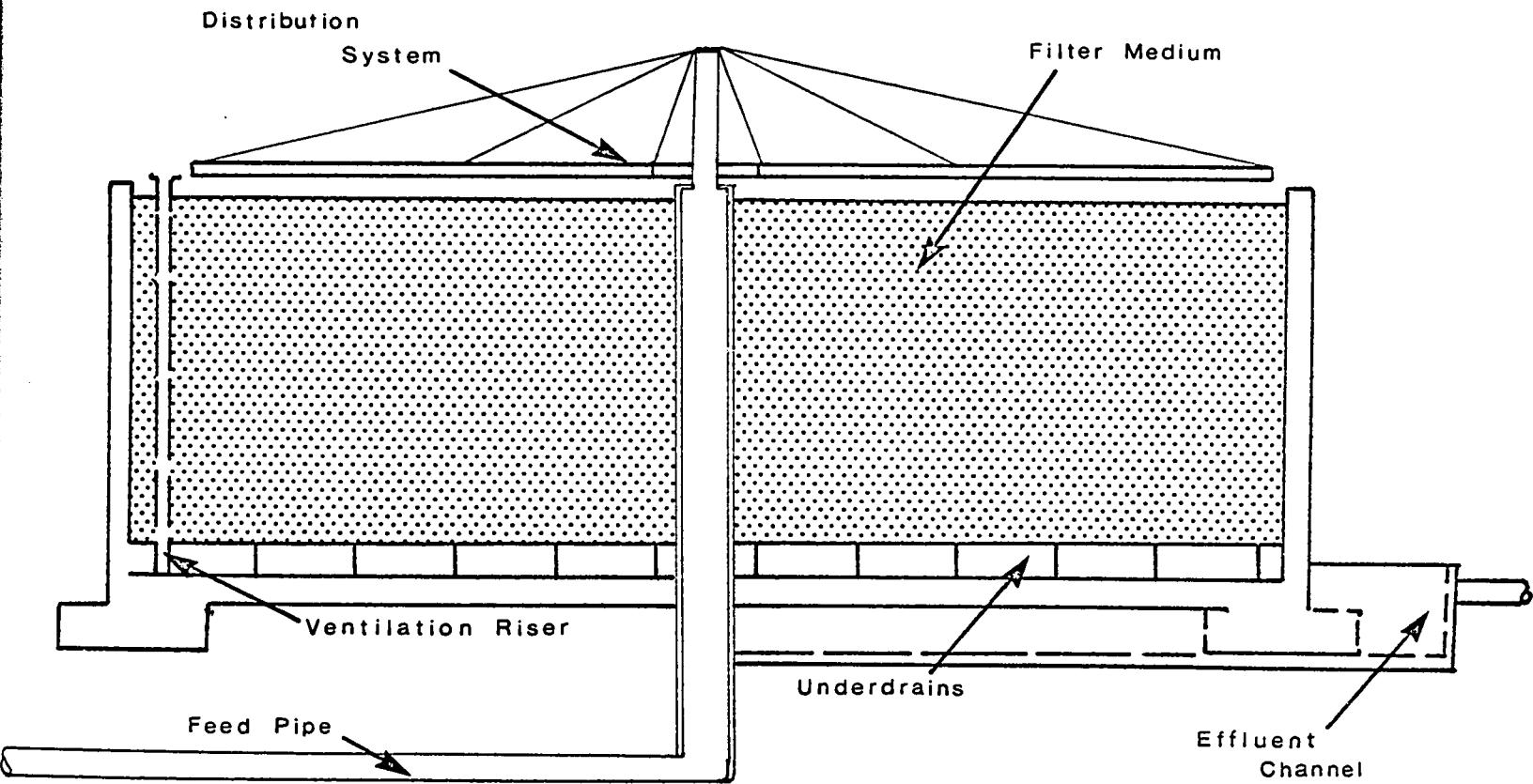
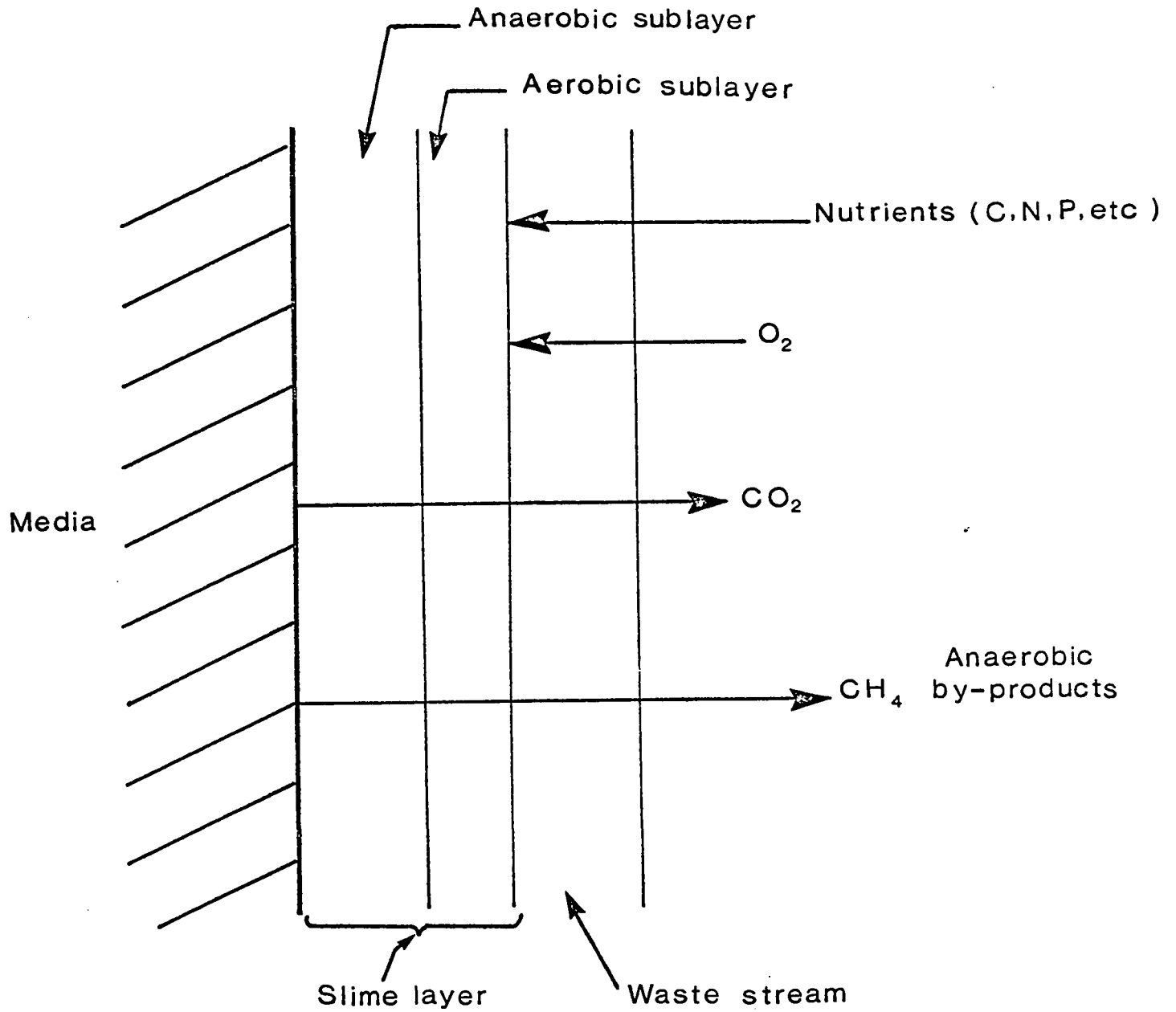


FIGURE 1.5 TRICKLING FILTER REMOVAL MECHANISM



the food (C, O_2 , nutrients, etc.) for synthesis, while those in the anaerobic sublayer undergo endogenous respiration.

The microorganism population also varies with filter depth, reflecting the facultative character of the treatment unit. Predominant microorganisms are bacteria. Fungi, being aerobes, are observed only where oxygen is predominant. The surface of the filter can also support algae but the need for sunlight limits the extent of their growth to that section of the filter only. The protozoa are the predominant animal population, with the ciliata observed in all aerobic sections of the filter. Higher order animals such as worms, snails, insects, and larvae are also present, and feed upon the microorganism population. Not essential to the substrate removal mechanism, higher order animals play a more significant role in the sloughing mechanism or, what can be referred to as the rejuvenation of the filter bed (7,1).

Film or slime removal is also an integral part of the trickling filter removal process, in the sense that it rejuvenates the filter bed. Excessively thick film layers inhibit oxygen transfer, interfere with the passage of solid particles, and sometimes result in ponding of the filter bed. Various organisms, along with the high hydraulic loadings, accompanied by some endogenous activity within the anaerobic sublayer, contribute to the sloughing of the slime layer (7, 1). The sloughing may be intermittent as in the case of

standard rate filters, or continuous as observed in high rate trickling filter installations.

CHAPTER 2

LITERATURE REVIEW

2.1 Introduction

2.1.1 Design History

This section will serve to establish the historical trend followed in the design of single stage trickling filter systems. No attempt has been made at this stage to discuss in any great detail the implications and structure of the reported mathematical formulations.

Data collected during World War II, from several trickling filter wastewater treatment plants at various military installations, resulted in the formulation by the National Research Council (U.S.) of a single stage BOD removal equation, referred to as the 'NRC Formula' (Equation #1, Table 2.1)(8).

In a 1948 article, Velz (9) presented what is referred to as the basic law in the trickling filter design. Velz postulated that "the rate of extraction of organic matter per interval depth of a biological filter is proportional to the remaining concentration of organic matter, measured in terms of its removability." Velz's mathematical formulation appears as Equation #2 in Table 2.1.

Applying mass balance principles, and respecting the criteria set forth by the Upper Mississippi River

TABLE 2.1

Common Trickling Filter Design Formulations

#	Equation Name	Mathematical Formulation	Definition of terms
1	NRC	$E_1 = \frac{1}{1 + 0.0085 \sqrt{\frac{W}{VF}}}$	E_1 = fractional efficiency of BOD removal for the first stage W = BOD loading in #/D, excluding recirculation V = volume of filter media in acre foot F = factor reflecting increased removal due to recirculation $F = (1 + r_1)/(1 + r_1/10)^2$ r_1 = ratio of recirculation flow to influent flow
2	Velz	$\frac{L_D^*}{L} = 10^{-K^1 D}$	L_D^* = removal BOD at depth D, in mg/l L_D^* = total removable fraction of BOD, in mg/l K^1 = rate of removal constant D = depth of filter in feet
3	Rankin	$E_1 = \frac{r_1 + 1}{r_1 + 1.5}$	E_1 = BOD removal efficiency of the first stage r_1 = first stage recirculation

Table 2.1 cont'd

#	Equation Name	Mathematical Formulation	Definition of terms
4	Howland	$\frac{L_e}{L_o} = e^{-K_H D / Q^{2/3}}$	L_e = BOD of filter effluent in mg/l L_o = BOD applied to the filter in mg/l K_H = rate constant of Howland D = depth of filter in feet Q = hydraulic rate of flow in MGAD
5	Schulze	$\frac{L_e}{L_o} = 10^{-b K_{20} D / Q^m}$	L_e = BOD of filter effluent in mg/l L_o = BOD applied to the filter in mg/l K_{20} = constant = 0.3 D = depth of filter in feet Q = hydraulic rate of flow in MGAD m = 0.67 b = 1.035 ($T_c - 20$) T_c = sewage temperature in °C
6	Eckenfelder	$\frac{L_e}{L_o} = \frac{1}{1 + C \frac{D(1-m)}{(Q)^n}}$ $L_o = \frac{L_i + r_1 L_e}{1 + r_1}$	L_i = influent BOD in mg/l L_o = BOD applied to the filter mg/l L_e = BOD of the effluent from the filter system r_1 = recirculation C = 2.5 D = depth of filter in feet Q = hydraulic rate of flow in MGAD n = 0.5

Table 2.1 cont'd

#	Equation Name	Mathematical Formulation	Definition of terms
7	Stack	$\Delta_{L_r} = XfS + f(L' - XfS)$ $\frac{(N+1)(C_{D-X})}{(1 + NC_{D-X})}$	Δ_{L_r} = removable BOD actually removed in pounds per unit area X = number of saturated unit depths f = coefficient of biosorption S = saturation capacity of a unit volume in pounds per unit volume
8	Stack	$\Delta_{L_r} = XfS + f(L' - XfS) \quad (c)$	L' = removable BOD applied in pounds per unit area c = $1 + (1+f) + (1+f)^2 + \dots$ C_{D-X} = unitless parameter, function of the coefficient of biosorption N = recirculation ratio
9	Galler and Gotaas	$L_{el} = 0.310L_{o1}^{1.189} \left(\frac{Q}{I}\right)^{.278}$ $(D+1)^{-.669} T_c^{-.154} Q^{-.062}$	L_{el} = BOD leaving the filter system in pounds per day per acre L_{o1} = BOD applied to the filter in pounds per day per acre Q = hydraulic rate of flow in MGAD I = influence rate of flow in MGAD T_c = sewage temperature in degree Centigrade D = depth of filter in feet
10	Ten State Standards	Graphical Method see Figure 2.1	

Boards of State Engineers, Rankin in 1951 presented his formulation of the trickling filter BOD removal process, (Equation #3, Table 2.1).

Howland (11) in 1958 found that the removal of organic matter within a trickling filter unit was a function of the time of contact of the wastewaters with the slime layer attached to the filter media. By establishing mathematically the time of contact in a trickling filter unit, he postulated that the removal of organic matter could be described by Equation #4, Table 2.1.

Schulze (12) in 1960, and Eckenfelder (13) in 1961, following reasoning similar to the one used by Howland, presented their own version of the organic removal process (Equations #5 and #6, in Table 2.1).

Stack (15) has analysed and formulated mathematical models for trickling filter installations with and without recirculation (Equations #7 and #8, Table 2.1). The extent of this application has been mostly restricted to research, and is mainly of academic interest.

It was only in 1964, that a totally new approach was adopted. Galler and Gotaas (16) presented a design model based on the multiple linear regression of some 322 observations obtained from existing trickling filter installations, (Equation #9, Table 2.1). High regression coefficients, along with a wide range of operational data, has given much merit to the work of these researchers from Northwestern University.

The 1971 edition of the Ten State Standards, offers a graphical technique for the design of trickling filter systems discussed and presented in a later section of this chapter.

Therefore, several mathematical formulations are available to practicing engineers in sizing and evaluating organic removal efficiencies within a trickling filter installation.

2.1.2 Controllable Parameter

In contrast with the activated sludge process which is often considered to be a very sensitive and difficult system to operate, a trickling filter plant offers a rather more simple and straightforward operation. As part of the operations within an activated sludge plant the operator has to control the volume of air supplied to the aeration tank, sludge recirculation, and the portion of sludge to be wasted daily. Trickling filter plant operation however offers only the control of the recirculated flow from either the filter effluent, final settling tank effluent, or secondary settling tank sludge, depending on the configuration of the process as shown in Figure 1.2.

Recirculation rates are usually dictated by either:

- i) the degree of treatment required, ii) the size of the pumps, iii) the size of the settling tanks, iv) the capacity of the distribution system, and v) capital and operating

cost (19). There exists, however, much controversy as to whether recirculation benefits or hinders the removal of organic matter as will be pointed out in Section 2.4.3 of this chapter.

2.2 Review of the Formulated Models

2.2.1 General

In attempting to survey the previously developed mathematical models for the removal of BOD within a trickling filter system, the following classification, based upon various approaches used in developing such formulations will be adopted:

i) Graphical or Curve Fitting Approach - Such an approach differs from others in the sense that it is not based on any theoretical derivations; in fact it relies solely on experiment and/or observations and attempts to represent graphically or through curve fitting of the removal of organic matter from a trickling filter system. Models such as the NRC formula, Rankin's, and the Ten State Standards were developed using such a concept.

ii) First Order Reaction Approach - Models developed while assuming that the substrate removal process can be represented by first-order reactions.

iii) Statistical Approach - This approach is rather unique and was presented for the first time in 1964. A general mathematical model generated on the basis of

operational results, using multiple linear regression techniques, forms the basis of the technique.

iv) Mechanistic Approach - The mechanistic approach involves the study of the mechanism of substrate removal.

2.2.2 Graphical or Curve Fitting Approach

As reported in a previous section of this chapter the National Research Council (U.S.) developed in 1946, an empirical formulation for the trickling filter removal process, based upon data collected from 34 existing trickling filter plants among various military stations. The efficiency of a single stage plant was reported as:

$$E_1 = \frac{1}{1 + 0.0085 \sqrt{\frac{W}{VF}}} \quad (2.1)$$

where E_1 = fractional efficiency of BOD removal for the first stage.

W = BOD loading of settled raw sewage in pounds per day (excluding recirculation).

F = factor reflecting increased removal due to recirculation $F = (1+r_1)/(1+r_1/10)^2$.

r_1 = ratio of recirculated flow to influent flow.

V = volume of the filter media in acre-foot

and that for second stage could be described as:

$$E_2 = \frac{1}{1 + \frac{0.0085}{(1-E_1)} \sqrt{\frac{W'}{VF}}} \quad (2.2)$$

where E_2 = fractional efficiency of BOD removal for the second stage.

W' = BOD loading applied to the second stage in pounds per day, exclusive of recirculation.

Equation (2.2), is a transformation of Equation (2.1) and takes into consideration the reduced treatability of the first stage effluent. Examining the components of such a formulation, one realizes immediately that the BOD removal efficiency is dependent upon recirculation and organic loading, while other parameters are kept constant for any specific installation. This therefore implies that significant variations in the hydraulic rate of flow, while organic loading remains constant, through corresponding variations in the BOD concentration, will not affect the removal of organic matter from the wastewaters. Moreover, both Equations (2.1) and (2.2), do not take into consideration the change in the activity of the biological organisms with corresponding wastewater temperature variations.

Rankin (10), formulated the BOD efficiency removal of a single stage trickling filter system as:

$$E_1 = \frac{r_1 + 1}{r_1 + n_1} \quad (2.3)$$

where E_1 = BOD removal efficiency of the first stage.

r_1 = first stage recirculation.

$n_1 = m_1 / (m_1 - 1)$.

m_1 = value appearing to reflect the kinetic properties of the waste.

Subjected to constraints established by the Ten State Standards (17), Rankin assigned a value of 3 to parameter m_1 . His reasoning based on a mass balance of the process assumes that the change in BOD between input and output equals the BOD depletion. The efficiency of the second stage was reported as:

$$E_2 = \frac{r_2 + 1}{r_2 + n_2} \quad (2.4)$$

where E_2 = BOD removal efficiency of the second stage.
 r_2 = second stage recirculation.
 $n_2 = m_2 / (m_2 - 1)$.
 m_2 = value appearing to reflect the kinetic properties of the waste entering the second stage.

Again constraints established by the Ten State Standards (17) led Rankin to assign a value of 2 to parameter m_2 .

In 1953, Rankin (10) compared his equation with the formulations of Velz and that of the National Research Council (U.S.), using supporting data from existing treatment plants. Because single stage treatment plants are more of interest in the present investigation, only the results of such systems are reported herein. Eight plants formed the basis for the comparison. The actual efficiencies of these single stage treatment plants were reported to vary from 84% to 92%. The models studied presented the following standard deviation:

- i) Rankin's Model: 2.53%
- ii) NRC Formula : 3.03%
- iii) Velz's Formula: 2.10% ,

revealing to some extent the accuracy with which each model was able to predict actual plant performance.

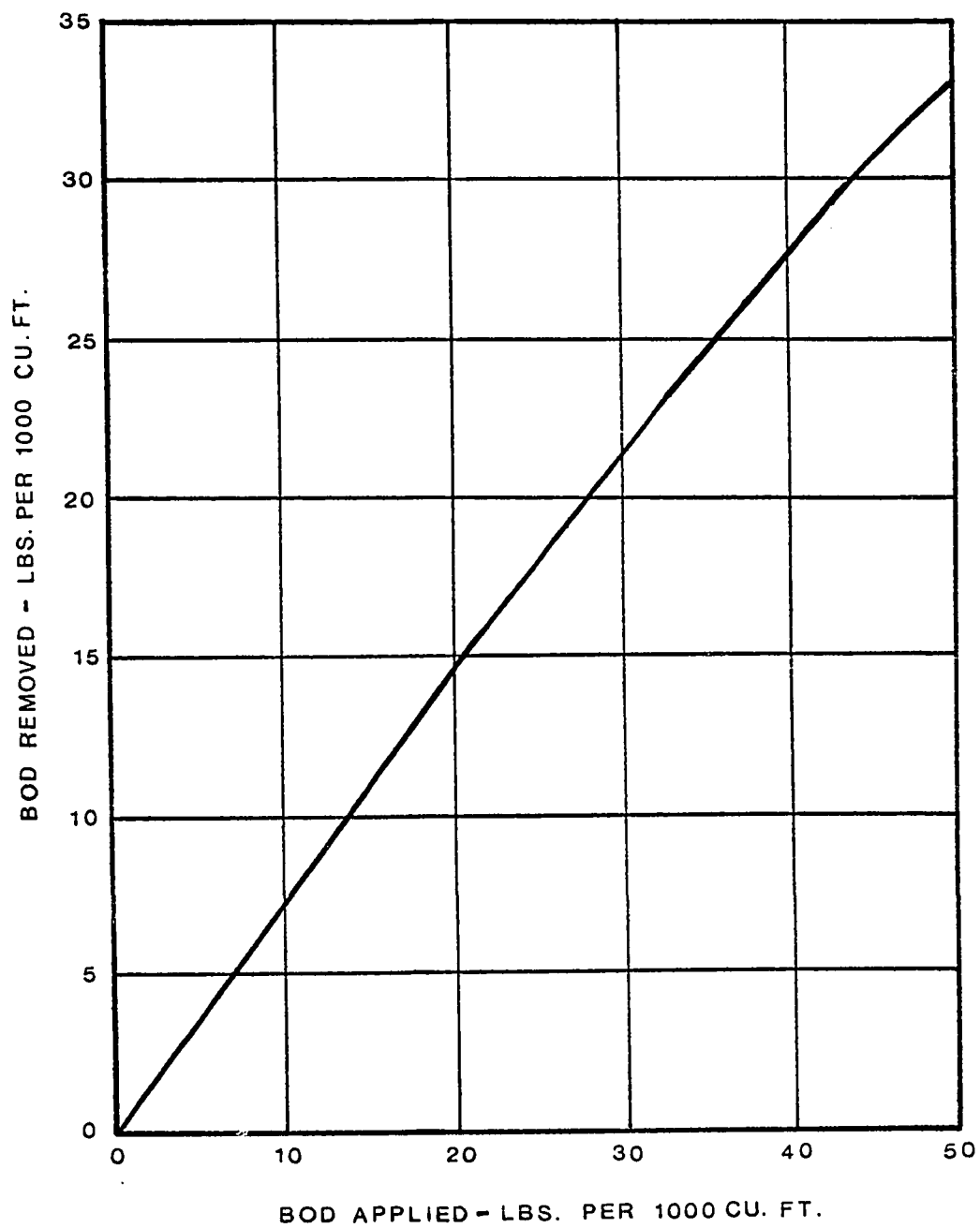
The 1971 edition of the Recommended Standards for Sewage Works (Great Lake-Upper Mississippi River Board of State Engineers) (17) dictates graphically BOD removal efficiency of the trickling filter system, as represented in Figure 2.1. However, the suggested BOD removal offers no consideration whatsoever to the contribution brought about through recirculation. In fact, design curves showing the effects of recirculation have not yet been developed.

Serving merely as a guide to practicing engineers, such a model is evidently an oversimplification of a very complex process. One immediate drawback of the proposed design method is the lack of consideration given to the effects of temperature, hydraulic loading and recirculation. Furthermore, the Ten State Standards design curve was derived using data from the northern regions of the United States, which furthermore limits the applicability of the model.

2.2.3 First Order Reaction Approach

Thériault (20) in 1926, assumed that the BOD exertion could be approximated through a first-order reaction

FIGURE 2.1 TEN STATE STANDARD DESIGN CURVE



formulation such as described by:

$$\frac{dL}{dt} = -kL \quad (2.5)$$

where L = organic BOD concentration.

t = time.

k = proportionality constant.

A similar reasoning to that of bio-oxidation was applied by Velz (9) in 1948, in attempting to explain theoretically the removal of organic matter within a trickling filter system. In fact he reported that "the rate of extraction of organic matter per interval of depth of a biological bed is proportional to the remaining concentration of organic matter measured in terms of its removability." In deriving his mathematical model, Velz made the following assumptions:

i) The trickling filter is basically a self regenerating absorption tower.

ii) Removable BOD, is the fraction of BOD measured that can be removed by absorption.

iii) Each unit of depth of the filter will remove a constant fraction of the removable BOD applied to it.

iv) The quantity of BOD that can be absorbed by one unit volume of filter has a maximum limit.

Expressed mathematically:

$$\frac{dL^*}{dD} = -K L^* \quad (2.6)$$

or

$$\frac{L_D^*}{L^*} = 10^{-K' D} \quad (2.7)$$

where L_D^* = removable BOD at depth D in mg/l.
 L^* = total removable fraction of BOD in mg/l.
D = depth in feet.
K = rate of removal constant.
 K' = rate of removal constant equal to 0.175
for low rate filters or standard rate,
and 0.150 for high rate filters.

Other values for K' have been reported elsewhere in the literature based upon a similar reasoning (21).

In 1957, Stack (22) presented a detailed analysis of the theoretical performance of the biological filtration process. Based on the assumptions of Velz, and on the concept of removable BOD Stack derived BOD removal equations for the cases of recirculated and non recirculated effluent.

Performance with no recirculation was described as:

$$\Delta_{L_r} = XfS + f(L' - XfS)(c) \quad (2.8)$$

where Δ_{L_r} = removable BOD actually removed in pounds per unit area.
X = number of saturated unit depths.
f = coefficient of biosorption.
S = saturation capacity of a unit volume in pounds per unit volume.

L' = removable BOD applied in pounds per unit area.

$$c = 1 + (1-f) + (1-f)^2 + \dots$$

while performance with recirculation was expressed as:

$$\Delta_{L_r} = XfS + f(L' - XfS) \frac{(N+1) (C_{D-X})}{(1+N) C_{(D-X)}} \quad (2.9)$$

where N = recirculation ratio.

$C_{(D-X)}$ = unitless parameter function of the coefficient of biosorption.

Conscious of the effects of temperature on the removal of organic matter, Stack suggested it to be reflected into the value of the saturation capacity of a unit depth, referred to as S , and expressed in pounds per unit volume.

The major drawback of the models developed by Stack lies in the fact that BOD removals are expressed as functions of removable BOD, defined earlier, and therefore require the experimental evaluation of the removable BOD, along with the values of S and f appearing in Equations (2.8) and (2.9).

In 1958, Howland (11) postulated that the removal of organic matter from a biological filter bed was a function of the time of contact between the wastewater and the slime layer, or time of travel within the filter bed. It was then established that:

$$t_H = \frac{(1.035^{T_c - 20}) D}{Q^{0.67}} \quad (2.10)$$

where t_H = time of travel in minutes.
 T_c = temperature of the sewage in $^{\circ}\text{C}$.
 D = depth of filter in feet.
 Q = hydraulic rate of flow in MGAD.

and BOD removal expressed as:

$$\frac{L_e}{L_o} = e^{-K_H D / Q^{.67}} \quad (2.11)$$

where L_o = BOD applied to the filter in mg/l.
 L_e = BOD of the filter system effluent in mg/l.
 K_H = rate constant.

Confronted with the fact that recirculation had improved removal efficiencies, Howland suggested three possible explanations to this phenomena: i) a dilution effect, ii) the continuing action of the freely floating film particles, and iii) increased time of contact of a portion of the flow. In summarizing the applicability of his theory to recirculation, the author pointed out that it raised more problems than it solved.

Schulze (12) in 1960, experimentally established a mathematical model for the removal of BOD in a trickling filter system using screen filters instead of the conventional filter media. The developed model was reported as:

$$\frac{L_e}{L_o} = a Q^d \quad (2.12)$$

where L_e = BOD in mg/l of the filter system effluent.
 L_o = BOD in mg/l applied to the filter.
 a = constant equal to 0.079 (experimentally).
 d = constant equal to 0.67 (experimentally).
 Q = hydraulic rate of flow in MGAD.

Schulze also established statistically that the BOD loading did not affect the BOD removal efficiency of the system. Investigating the effects of recirculation he postulated that "according to the theoretical data, recirculation does not improve the quality of the final effluent," and suggests that the increase in efficiency is attributable to secondary factors. In an attempt to relate BOD removal efficiency to such parameters as depth and temperature, Schulze suggested the following equation:

$$\frac{L_e}{L_o} = 10^{-bK_{20}D/Q^{.67}} \quad (2.13)$$

where L_e = BOD in mg/l, in the filter system effluent.
 L_o = BOD in mg/l applied to the filter.
 D = depth of the filter in feet.
 Q = hydraulic rate of flow in MGAD.
 $b = 1.035^{T_c - 20}$.
 K_{20} = experimentally established constant equal to 0.3.

This formulation is also corroborated by the work of other researchers (9,14,21).

Grantham (23), pointed out, however, that even though it appears that the BOD removal efficiency is unaffected by changes in the BOD loadings, nitrification of the filter effluent decreases with higher BOD loadings, thereby decreasing the overall BOD quality of the effluent.

In 1961, Eckenfelder (13), described the removal of organics within the trickling filter system using principles established by Velz and Howland. Combining the efforts of these people, Eckenfelder proposed the following equation:

$$\frac{L_e}{L_o} = e^{-K_E (1/D_m) (D/Q^n)} \quad (2.14)$$

where L_e = BOD of the effluent from the filter system in mg/l.

L_o = BOD applied to the filter in mg/l.

K_E = modified rate constant.

D = depth of the filter in feet.

Q = hydraulic loading in MGAD.

The expression $1/D_m$ is related to the mean active filter film per unit volume throughout the filter depth. If the filter is uniform, the $1/D_m$ approaches 1.0 and Equation (2.14) becomes:

$$\frac{L_e}{L_o} = e^{-K_E D/Q^n} \quad (2.15)$$

Eckenfelder also suggested that, in the case of complex organic wastes, the removal decreases with concen-

tration, because readily degradable components are removed first. Equation (2.15) was thereby transformed so as to include a retardation effect brought about by such waste characteristics:

$$\frac{L_e}{L_o} = \frac{100}{1 + \frac{C^* D^{1-m}}{Q^n}} \quad (2.16)$$

From the analysis of filter performance data, Eckenfelder determined the following values: $C^* = 2.5$; $(1-m) = 0.67$, and $n = 0.50$. Commenting on the effects of recirculation, Eckenfelder postulated that it provided only a dilution effect.

Because of the dependence of the process on sewage temperature variations, an equation relating to the thickness of the aerobic film layer (active film thickness) with sewage temperature was also developed by Eckenfelder:

$$h = \sqrt{\frac{D_L C_{O_2}}{K_r}} \quad (2.17)$$

where h = aerobic film thickness in cm.

D_L = oxygen diffusivity through the film.

C_{O_2} = oxygen concentration.

$K_r = K_{20}(1.08)^{T_c - 20}$ = unit oxygen utilization rate.

T_c = sewage temperature in $^{\circ}\text{C}$.

2.2.4 Statistical Approach

The only statistical approach, i.e. one based on multiple regression of existing trickling filter installations, was developed by Galler and Gotaas (3,16), in 1964, at Northwestern University. Their objectives were two-fold:

i) develop a mathematical model for the BOD removal process within a trickling filter system, in terms of physical parameters readily available, and based upon existing plant operation data, and ii) optimize the design of the trickling filter system so as to minimize cost while subjected to several specific constraints.

Following a basic mathematical model building procedure, Galler and Gotaas established relationships between variables by plotting assumed independent variables against the percent BOD removal. Investigating the use of multiple linear regression techniques, Galler (3) reported several mathematical formulations. After evaluating the models statistically, the following formulation was proposed as input into the second phase of the investigation:

$$L_{e1} = \frac{0.3099 L_{o1}^{1.18896} R_*^{0.27762}}{(D+1)^{0.66916} T_c^{0.15440} Q^{0.06170}} \quad (2.18)$$

where L_{e1} = BOD of the effluent from the filter system in pounds per acre per day.

L_{o1} = BOD applied to the filter in pounds per acre per day.

R_* = $(1+r_1)$: ratio of the flow through the filter to the flow entering the plant.

T_c = sewage temperature in $^{\circ}\text{C}$.

Q = flow through the filter in MGAD.

D = depth of the filter bed in feet.

The model reported yielded a multiple correlation coefficient (R) equal to 0.9743, with a standard error of the estimate (s) of 0.11964, and a range of residuals of 0.802. Galler further reported at this stage that the hydraulic rate of flow had only a 'chance' influence on the BOD removal process, as determined by a 5% α -level 't' test. However, the proposed mathematical formulation included the hydraulic rate of flow as one of its parameters on the basis that the range of residuals was least using such a formulation, i.e. 0.802 as opposed to 0.815.

The latter portion of Galler's investigation dealt with the optimization of trickling filter system. The optimization was subjected to the following set of constraints: i) biological, ii) radius, iii) depth, and iv) recirculation constraints. Four cases were reported: i) a trickling filter unit, ii) a trickling filter system, iii) a deep filter with forced ventilation, and iv) minimization of the BOD remaining in the effluent subjected to limited cost (3). Applying linear programming techniques the following relevant conclusions were reported: i) recirculation ratio and depth were constant for any influent rate having a given BOD and desired BOD reduction, ii) the hydraulic rate of flow should be as high as possible provided recirculation ratio is less

than 4:1 and iii) trickling filters should be as deep as possible provided pumping is not required.

Galler's investigation is certainly the latest most significant contribution into the performance evaluation and design of the trickling filter system. It has established grounds for further research in a similar line of thought.

2.2.5 Mechanistic Approach

The mechanistic approach such as the one adopted by Ames et al (24), Maier et al (25), Atkinson et al (26), and others (27,28,29), was developed because of the inability of the former approaches to incorporate all of the significant variables within the process into a single specific model (30). Their objective was therefore to determine and describe mathematically the specific substrate removal mechanisms.

In 1962, Ames, Behn and Collings (24) assuming that first-order reactions were taking place in the metabolism of organic matter within the slime layer, and using mass balance principles, developed a mathematical model incorporating the following parameters: hydraulic loading, BOD concentration, specific surface area of the media, a mass transfer coefficient, and a biological rate constant. This formulation was further modified by Amado (37) and finally resulted in an expression very similar to those presented in Section 2.2.3. No experimental data however were available to correlate actual and predicted performances.

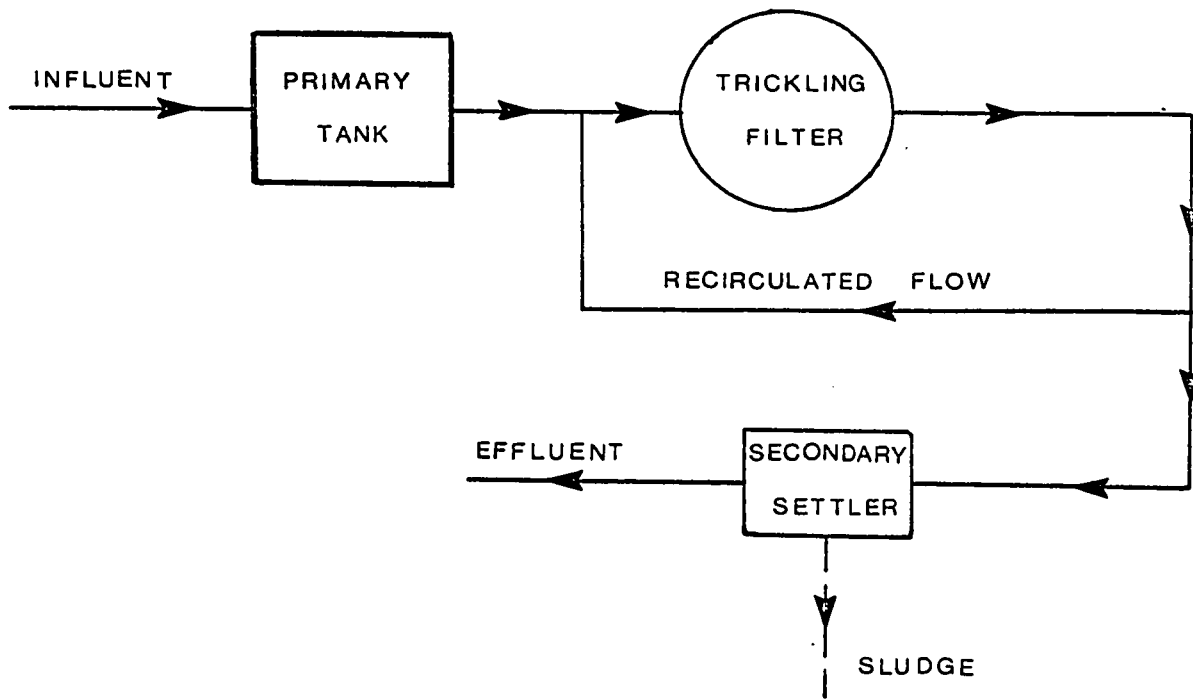
In 1963, Atkinson, Busch and Dawkins (26) using a vertical film flow reactor, differential mass balance and first-order kinetics, developed a mathematical model again similar to those presented in Section 2.2.3. Their model however was of limited applicability, because it assumed that biological reactions were taking place throughout the entire film thickness, rather than through the aerobic sub-layer solely. Moreover, they postulated that the time of contact of the wastewaters within the trickling filter media was irrelevant, and served only to confuse the issue.

Atkinson and Swilley (28), developed in 1963 what was referred to as a diffusion controlled model. The model was derived from differential equations describing mass transfer based on molecular diffusion. Monadjemi (27), and Maier (25) also adopted a reasoning similar to that of Atkinson and Swilley. Others have developed reaction controlled models, but these, along with the diffusion controlled models, will not be presented hereafter; instead, a rather detailed and comprehensive résumé appears in Jank (30).

2.3 The Economic Analysis of Roesler and Smith

In a 1969 Federal Water Pollution Control Administration report (31), Roesler and Smith have described a mathematical model for the trickling filter-final settling tank system. The flow diagram of the simulated plant is presented in Figure 2.2.

FIGURE 2.2 ROESLER AND SMITH FLOW DIAGRAM



Their formulation was based on first-order kinetics, and on the work of Howland (11). The following mathematical model was formulated:

$$\frac{L_e}{L_o} = e^{-K^* A_p^{m'} D / Q^{n'}} \quad (2.19)$$

where L_o = BOD applied to the filter in mg/l.
 L_e = BOD of the effluent from the filter system in mg/l.
 A_p = specific surface area ft^2/ft^3 .
 Q = hydraulic loading in MGAD.
 K^* = constant equal to 0.69.
 m' = exponent assumed to equal 1.0.
 n' = a function of the specific surface area established at 0.67.

Sewage characteristics at various stations were reported, and stream vectors described a total of seventeen pieces of information. The information offered at each stream vector related to: flow, carbon, nitrogen, and phosphorus content and composition, alkalinity, suspended solids, and BOD.

In addition to performing a mass balance over the complete system, Roesler and Smith reported capital and operating costs as functions of both volume and type of filter media. Final settler cost and pumping cost functions were also reported.

Several technical and economic conclusions based on the performance of the postulated model were presented. The authors were able to illustrate graphically, that increased hydraulic loadings were best handled with media that provided large surface area per unit volume, such as synthetic packings. Even though it might appear beneficial to always provide large surface areas, a graphical representation of depth required for 85% removal of BOD, versus the hydraulic loading, showed that in the neighbourhood of 8 MGAD and over, surface areas in the order of $10 \text{ ft}^2/\text{ft}^3$ required the least depth. This phenomenon can best be explained by the fact that at high hydraulic loadings, and low surface area, there exists a greater probability of encountering turbulent flow conditions, thus bringing better contact between the wastewater and the slime layer, therefore requiring less depth. Such reasoning is also supported by Atkinson, Busch and Dawkins (26), where it is postulated that turbulent flow regime can reduce the required filter volume. Kehrhberger and Busch (29) also point out the possibility of increased BOD removal under turbulent flow conditions.

Thus, the work of Roesler and Smith provides excellent tools to establish preliminary BOD removal efficiencies and cost estimates of proposed trickling filter operations. Because of the assumptions established throughout the report such an analysis should only be considered as a preliminary design.

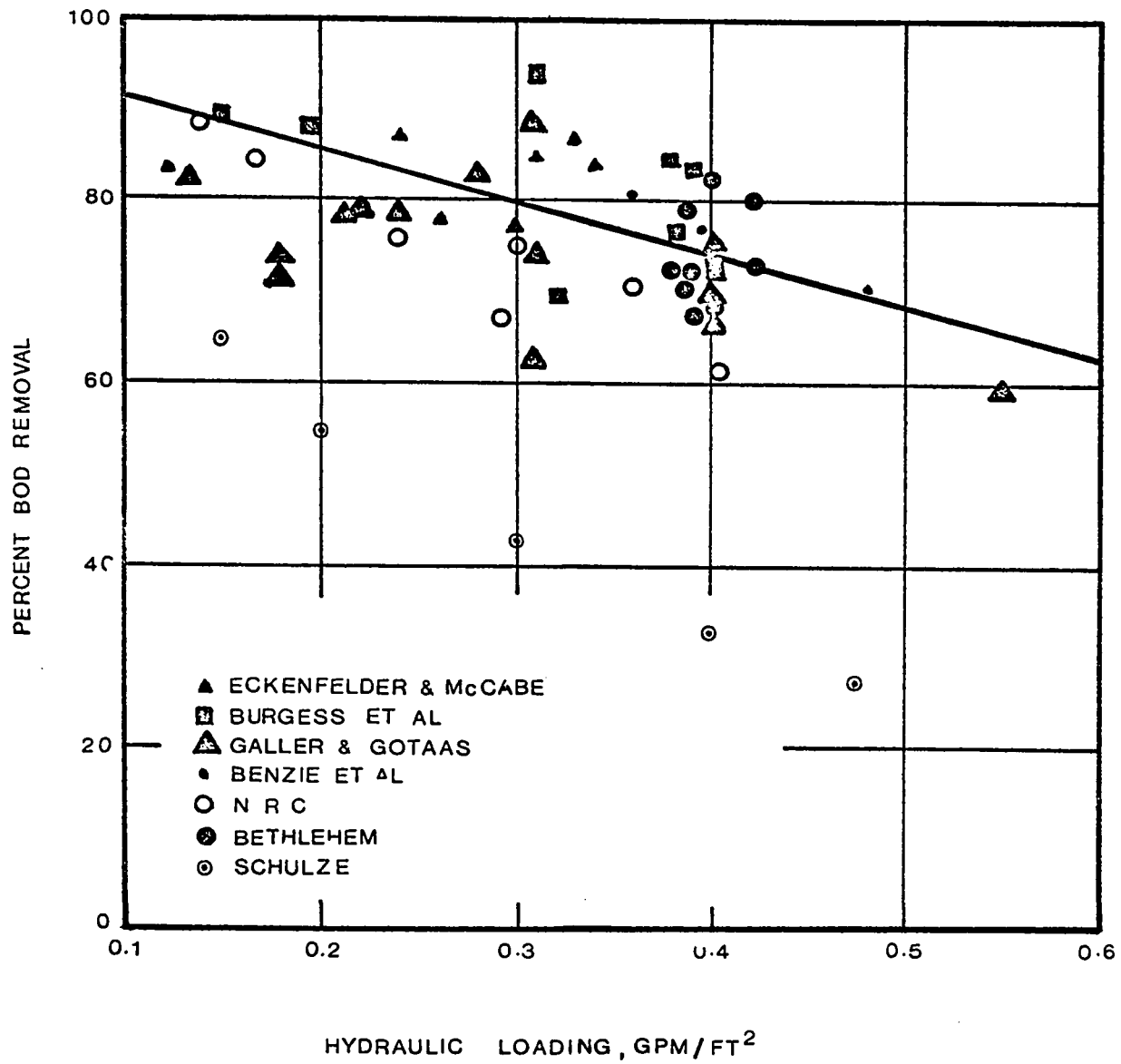
2.4 Trickling Filter Performance Factors

2.4.1 Hydraulic Loading or Hydraulic Rate of Flow

The hydraulic loading has been previously defined as the rate of flow per unit area. There are several controversial opinions as to whether such a parameter influences the removal of organic matter within a trickling filter system. The disparities among opinions are best illustrated graphically in Figure 2.3, taken from a recent report of R. F. Weston Inc. to the Environmental Protection Agency (18). Thus graphically, there appears to be a definite trend within the data to support the hypothesis that the hydraulic rate of flow does bear a direct influence on the removal of organic matter within a trickling filter system. Therefore, as illustrated in Figure 2.3 increased hydraulic loadings have a tendency to hinder BOD removal efficiency of the trickling filter system.

Other have advanced the idea that BOD removal is not sensitive to any hydraulic flow variations, as long as the organic loading remains constant by appropriate BOD concentration variations. In fact Velz's (9) mathematical model is solely a function of the organic loading and of the depth of the filter bed. Stack (15) also reported BOD removal to be independent of the hydraulic loading. In a recent investigation, Galler and Gotaas (16) demonstrated statistically that the hydraulic rate of flow onto a filter had little, if any, influence on the BOD removal process.

FIGURE 2.3 BOD REMOVAL VERSUS HYDRAULIC LOADING



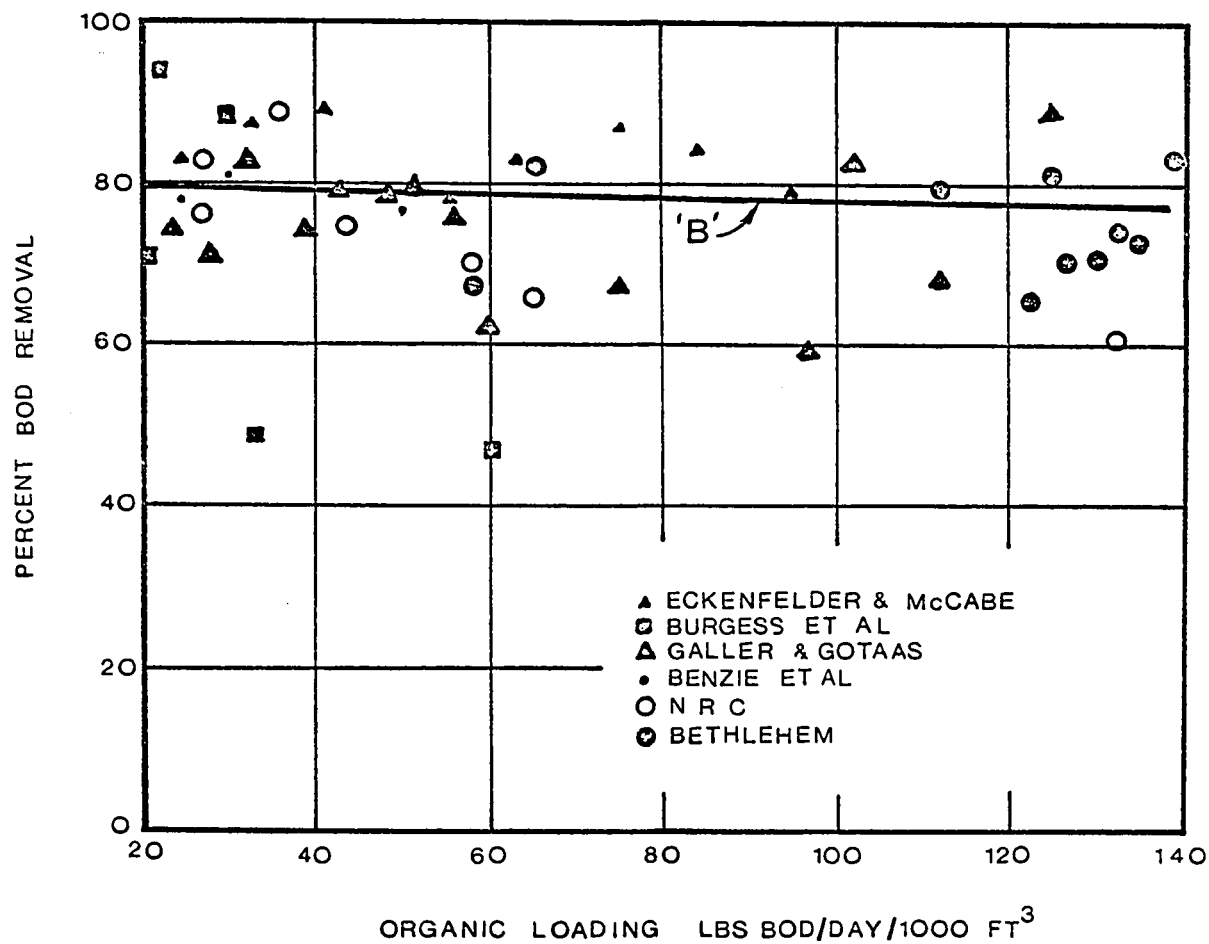
In direct opposition to the above postulate, Schulze (12), Eckenfelder (13), and Howland (11) maintain that the removal of organic matter is a direct function of the hydraulic rate of flow applied to the filter unit. In fact, higher BOD removal was observed under low hydraulic loadings as shown in Figure 2.3.

2.4.2 Organic Loading

Defined as the quantity of organic matter applied per unit volume, the organic loading has been and still is, in some cases, used as the sole basis for establishing required filter media volumes. The graphical analysis of the effects of organic loading variations on the BOD removal efficiency in a trickling filter system, is presented in Figure 2.4, adopted from Weston (18). The almost negligible influence of the organic loading approximated by line 'B' in Figure 2.4 contradicts somewhat the conclusions of Galler and Gotaas (16). This group, in agreement with the assumptions presented by Velz (9), and Stack (15), illustrated statistically that the removal of organic matter was a direct function of the applied organic load. In fact increased organic loadings were observed to decrease organic removal efficiency.

However, as was pointed out by Grantham (23) even though it appears (Figure 2.4) that the removal of organic matter is not affected by variations in the organic loading,

FIGURE 2.4 BOD REMOVAL VERSUS ORGANIC LOADING



the quality of the effluent in terms of BOD, is affected due to a decrease in nitrification. In this respect, variations in the organic loading can be considered to influence effluent BOD quality.

The general trend presented in Figure 2.4 is however supported by many researchers. Eckenfelder (13), Schulze (12) and Howland (11), in their models only give indirect consideration to the effects of organic loading variations upon the BOD removal.

2.4.3 Recirculation

Whether or not recirculation provides a detrimental or beneficial performance efficiency in terms of BOD removal, still has not been categorically answered. Many investigators have postulated the effect to be beneficial, (13,9,16,8,10,26), accounting for the increased BOD removal efficiency in one of several ways: i) a dilution effect, ii) seeding, iii) increased residence time, and iv) the continuing action of the freely floating slime particles in the secondary settler. In fact Moore et al (32) as reported by Galler (3) showed that the greatest increase in plant BOD removal efficiency had been obtained when sludge from the secondary settler was recirculated back to the primary clarifier. A smaller organic matter removal increase resulted when unsettled effluent from the trickling filter effluent was again recirculated to the primary tank. However,

no increase in BOD removal efficiency resulted when the clarified effluent from the secondary clarifier was recirculated to the primary tank. Such observations therefore tend to favour the hypotheses of increased BOD removal efficiency through a seeding effect rather than through a dilution effect.

On the other hand, many researchers have postulated the effects of recirculation on the removal of organic matter to be detrimental, (4,11,12,28,29). In fact, Kehrberger and Busch (29) maintain that their theoretical heterogeneous models corroborated such a statement. This group of researchers further report that the overall effects of recirculation were to decrease residence time, and increase resistance to mass transfer. However, they recognized the possibility of a decrease in diffusional resistance with increased volumetric flow. It is however important to realize that their conclusions were related to a heterogeneous film flow reactor consisting of a continuous flow over a plate.

2.4.4 Temperature

The trickling filter process being an adsorption process dependent on the capability of microorganisms to assimilate organic compounds, has steadily been reported to be temperature sensitive.

Schroepfer et al (33), investigated the qualitative and quantitative temperature effects upon the process. The behaviour of the slime producing zooglea bacteria to changes in temperature essentially describes the qualitative portion of the analysis, whereas quantitatively the author examined the BOD removal efficiencies of existing trickling filter installations subjected to sewage temperature variations. Thus, the activity of the most important group of microorganisms within the trickling filter unit was reported to practically double with a 10°C raise in temperature. For the second portion of their investigation, they analysed graphically percentage BOD efficiency removal with sewage temperature in degree Fahrenheit. The slopes of the lines thereby obtained, represented a measure of the temperature effect on BOD removal efficiency. These ranged from $0.18\%/^{\circ}\text{F}$ to $0.74\%/^{\circ}\text{F}$, with coefficients of correlation in the order of 0.75. Other researchers (13,15,16,34), have also reported in one way or another the sensitivity of the system to sewage temperature variations. In general, all of the researchers that have examined the effects of temperature on the removal of organic matter within a trickling filter system agree in saying that a raise in temperature enhances BOD removal within a trickling filter system, and vice versa.

2.4.5 Ventilation

An adequate supply of air or oxygen is essential to maintain aerobic conditions in a trickling filter unit, because the process is essentially aerobic.

In an intensive study, Johnson (35), concluded that by using a safety factor of 20 in the BOD reduction, a relatively low rate of air flow was required to maintain aerobic conditions. In fact the natural forces in a well designed filter would easily provide these estimated minimum requirements. Where ventilation is concerned the Ten State Standards (17) recommend that all drains, channels, and pipes be sized such that no more than 50% of their cross sectional area be submerged at design hydraulic loadings.

2.4.6 Media

In a 1971 Environmental Protection Agency report R. F. Weston Inc. (18) presented a table comparing the physical properties of several filter media, Table 2.2.

Synthetics, or plastic media filters offer two advantages over the conventional stone media units, as shown in Table 2.2. A larger specific surface area per unit volume, along with an increased percent void space, allow the application of higher hydraulic loadings. Increased void space also permits the application of higher hydraulic loadings and favors oxygen transfer. However, as was pointed out by Roesler and Smith (31), synthetic media will require deeper

TABLE 2.2

Filter Media Characteristics

Packing	Nominal Size inches	Units Per Sq.Ft	Unit Wt. #/ft ³	Specific Surface Area sq.ft/cu.ft	Void %
Plastic Media	20x48	2-3	2-6	25-30	94-97
Del Pak Redwood Media	47½x47½35-3/4	-	10.3	14	-
Granite	1-3	-	90	19	46
Granite	4	-	-	13	60
Blast Furnace Slag	2-3	51	68	20	49

depths at high hydraulic loadings to produce equivalent effluent quality, because of prevailing laminar flow regime.

Many industrial applications have been found for synthetic filter media. These recently introduced media have been shown more economical and more flexible in some instances than the conventional stone filter media. In fact, one of the most important applications might be in the treatment of unseizable industrial liquids (36).

2.4.7 Depth

Most trickling filter units are designed with depths varying from three to ten feet. Greater depths however offer additional opportunity for waste oxidation and thus enhance BOD removal, as was pointed out by Schulze (12), Velz (9), Eckenfelder (13), Howland (11), Stack (15), and Galler and Gotaas (16).

Some producers of synthetic plastic media maintain that there is no limit to maximum filter depth, except those imposed by physical constraints, such as adequate ventilation and structural requirements (53).

However, deep filters (stone media) often require mechanical ventilation devices so as to maintain aerobic conditions. Galler (3) concluded that "Depth contributes significantly to BOD reduction... Deep filters having forced ventilation are more economical only for very high reductions in BOD and when pumping of the influent to the top of the filter is unnecessary."

CHAPTER 3

THE STUDY AND ITS OBJECTIVES

The controversy evolving in the selection of significant parameters that affect the treatment of waste-waters by means of trickling filters, illustrates that though the process itself is considered as one of the oldest and common methods of treatment in North America, the basic principles underlying its operation are not generally agreed upon. In general, the process is relatively straightforward, but embraces a realm of unknowns which lead to considerable variations in the performance of even similar treatment plants. It would therefore be unwise to consider applying the results of the present investigation to operate and/or design other trickling filter installations, without careful examination of all components and parameters of both systems. Furthermore, this study does not pretend to delineate once and for all the performance dependency of the process, as there exists evidence and data to support practically any conclusions.

Few authors and/or researchers have ever paid much attention to the mathematical formulation of an existing high rate trickling filter plant as a whole, and even fewer have ever considered the mathematical formulation of the removal efficiency of both BOD and suspended solids for the

major physical and biological units in a trickling filter plant. In fact, most researchers have analysed the trickling filter-final settling tank process, rather than examining each unit separately. In this investigation, however, separate mathematical models will be developed for both the trickling filter unit and the trickling filter-final settling tank subsystem.

In carrying out this analysis the high rate trickling filter installation of Alexandria, Virginia, U.S.A., will serve as the basis for the building of all mathematical models.

The objectives of the present investigation can be summarized as follows:

- i) Collect field data from the presently operating Alexandria sewage treatment plant, and assemble this bulk of information into proper format.
- ii) Conceive a computer program to perform a mass balance on the selected trickling filter installation, so as to summarize and manipulate by computer the available information.
- iii) Develop mathematical formulations, for both BOD and suspended solids removal efficiencies for most of the important physical and biological treatment units.
- iv) Compare under various levels of operation, the performance of the Alexandria Sanitation Authority sewage treatment plant, with other postulated mathematical

models currently available for the design of such systems.

v) Examine the overall performance and sensitivity of the plant subjected to both normal and extreme operating conditions. Examine also sewage plant effluent characteristics and sludge production variations within the sensitivity analysis, so as to fully appreciate the extent of the plant's operation.

The relevance of this study is further accentuated by the fact that the plant designed in the early 1950's has now reached its ultimate design flows, predicted for 1973. The careful examination of the reported operational characteristics can yield valuable information as to possible expansion of the plant, assuming the applicability in series or in parallel of the suggested mathematical models. Such an investigation, however, can be considered an extension of the present report, and to be of significance, would constitute a report in itself. However, the present investigation will establish grounds for such an extension.

CHAPTER 4

ALEXANDRIA SANITATION AUTHORITY SEWAGE TREATMENT PLANT

4.1 Geographical Location

Alexandria, with a population of little over 100,000 is located in the north east sector of the State of Virginia, adjacent to the District of Columbia. A map of the State of Virginia is provided in Figure 4.1.

The sewage treatment plant, providing a design flow of 18 MGD, situates itself on the north shore of Hunting Creek, near its junction with the Potomac River. The ultimate design population of 1973 is expected to total 150,000 people, consisting of 105,000 in Alexandria and 45,000 in Fairfax County. Extensions to the plant will enable the handling of an additional 15 MGD, bringing the total plant capacity to 33 MGD (38).

4.2 Treatment Plant Characteristics

The Alexandria Sanitation Authority (ASA) sewage treatment plant, consists of a high rate trickling filter system, recirculating part of its filter effluent back to the primary settling tank influent. A flow diagram of the plant is presented in Figure 4.2.

FIGURE 4.1 MAP OF THE STATE OF VIRGINIA - U.S.A.

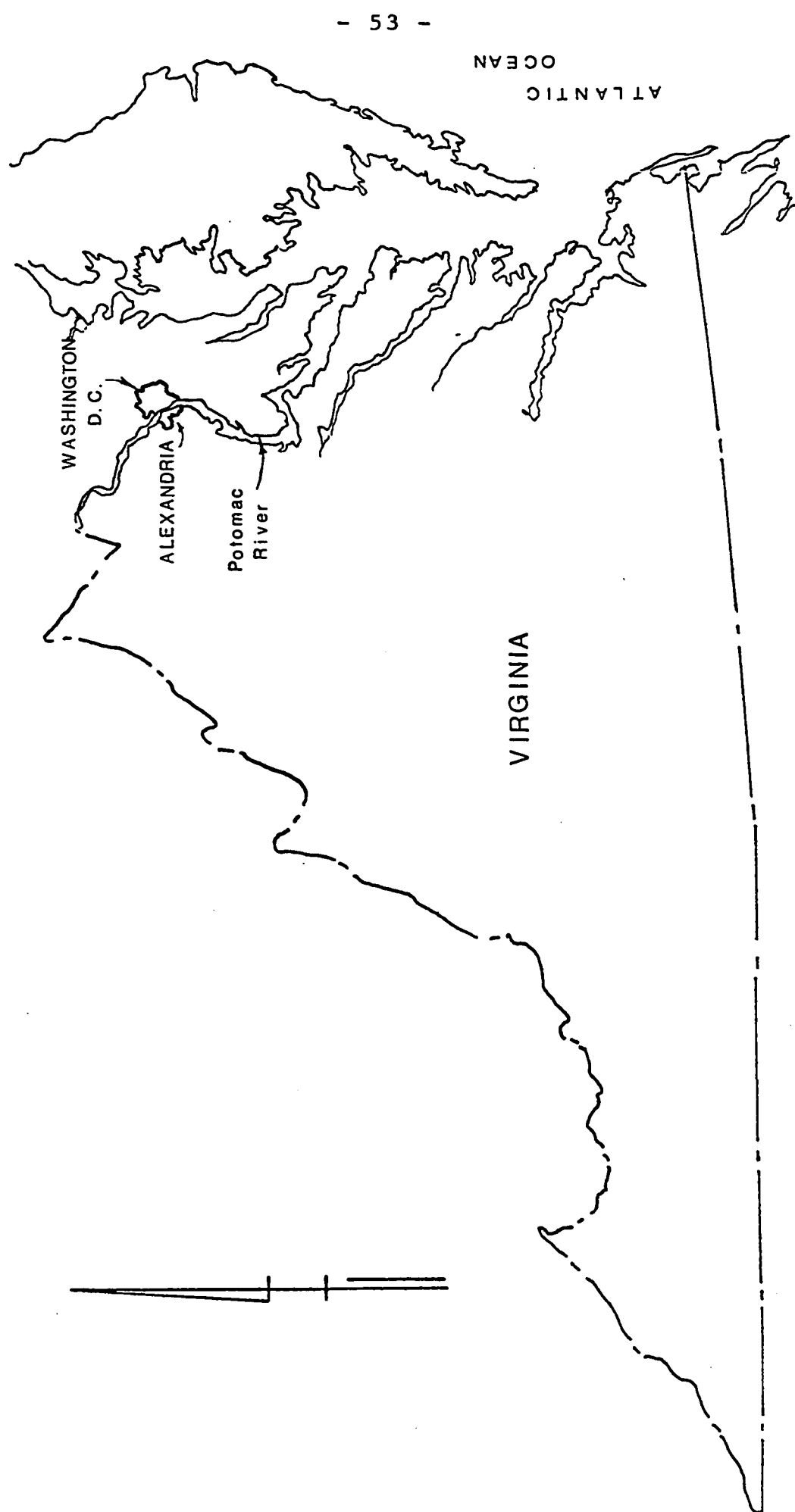
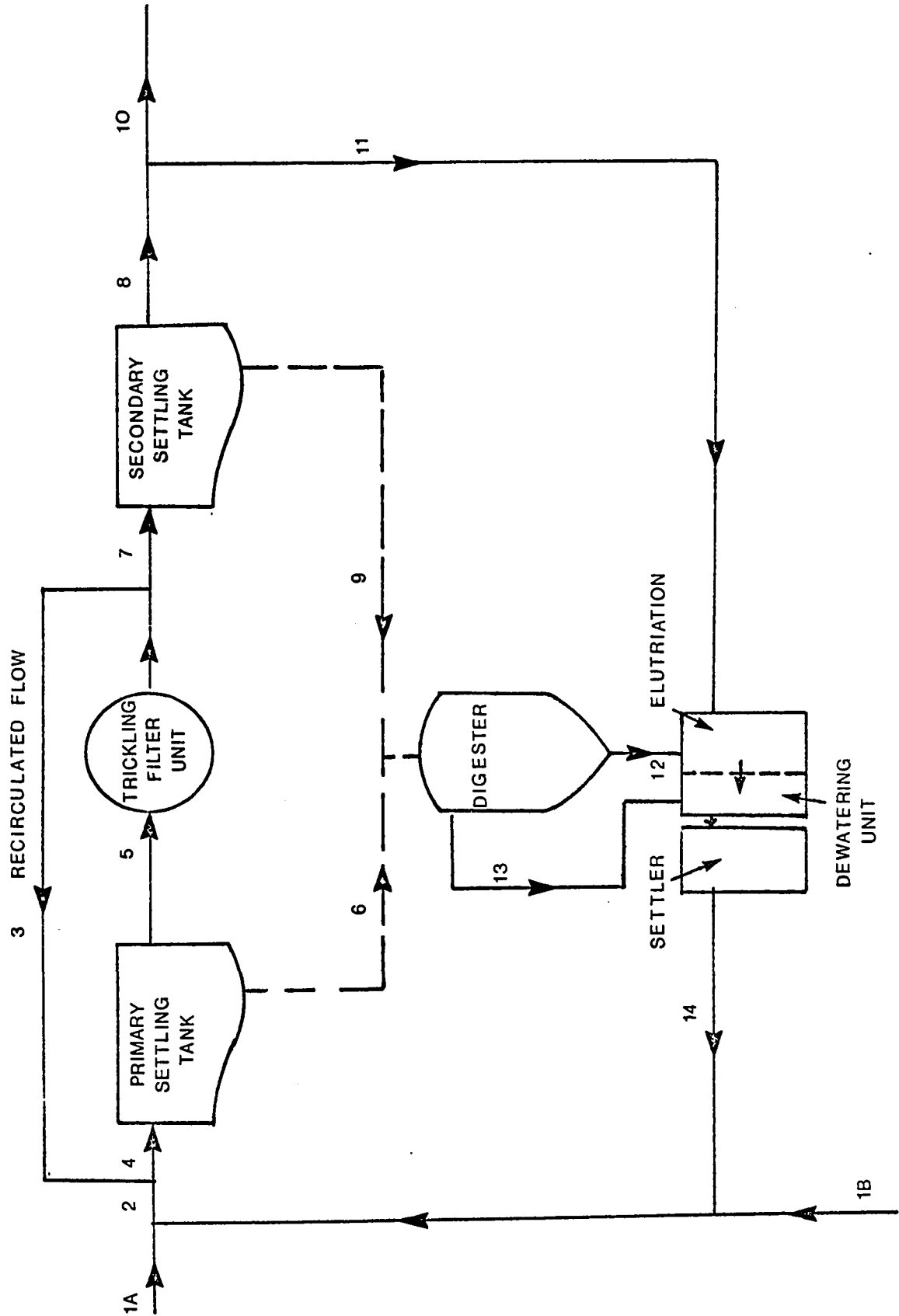


FIGURE 4.2 FLOW DIAGRAM OF THE ALEXANDRIA SEWAGE TREATMENT PLANT



The sewage enters the plant from two different collectors, identified as '1A' and '1B', in Figure 4.2. It is then pumped through mechanically-cleaned screens, where solid particles three quarters of an inch and larger are removed, ground, and returned to the flow. Mechanically cleaned grit chambers are then provided for the removal of sand, gravel, and other inorganic material, to be finally disposed of in a sanitary landfill.

The effluent from the grit chamber then joins with the recirculated trickling filter effluent before entering into the primary settling tanks. These settling tanks, four in all, consist of eight channels, 18' x 173' and 9' deep. The primary sludge is pumped to the digesters while the settled sewage proceeds to the three parallel trickling filter units. Each filter unit is 160' in diameter and 4.25' deep. Of a stone media, the filters provide a total volume of 256,400 cubic feet, with a surface area of 60,300 square feet. Part of the trickling filter effluent is recirculated back to the primary settling tanks, while the rest proceeds to the five final settling tanks. The final or secondary settling tanks, consist of ten channels, 18' x 125' and 10' deep, and allow filter effluent solids to be removed from the wastewaters. The sludge resulting from the settling of the filter effluent is then pumped to the digesters along with the primary sludge.

Two digesters, 95' in diameter and 28' deep, are

used for sludge stabilization. The gas produced is used for heating, and to power the main sewage pumps.

Elutriation then follows digestion. This operation proceeds in one of two tanks, the other being used to pre-settle the elutriation effluent and the supernatant liquor from the secondary digester prior to returning to the plant influent through interceptor '1B', in Figure 4.2.

The final physical process is sludge dewatering. This operation is performed on two stainless steel coil spring vacuum filters, 10' in diameter, by 10' in length. The resulting sludge cake is then conveyed to a hopper, loaded into trucks, and finally disposed of in a landfill area (38).

4.3 Operation Data

4.3.1 General

The Environmental Engineering Department, School of Civil Engineering of Purdue University, receives annually copies of the operation report of the Alexandria Sanitation Authority sewage treatment plant. In cooperation with Dr. Pierre Lacroix, a former graduate student of Purdue University and supervisor of the present investigation, and Professor Don E. Bloodgood, Professor Emeritus, Purdue University, copies of the annual reports covering the period from October 1964 to September 1971 were obtained for the present study. Seven years of data constituting a total of 84 monthly average observations served as the basis for this research.

4.3.2 Storage and Retrieval Mechanism

The annual reports of the ASA sewage treatment plant consist of two sections: i) a financial statement, and ii) monthly operation data. The monthly performance of the plant is described using over 90 separate pieces of information. The complete storage of this bulk of information requires the filling of over 8,000 separate entries. Careful examination of the data led to the following conclusions: i) much of the data supplied was irrelevant to the present objectives of the investigation, and ii) many entries were easily reproduced through basic mathematical manipulations. In fact, substantial reduction in the volume of information to be stored was realized. A monthly description of the plant's operation was achieved using 31 separate entries, of which a few could still have been generated within the storage-retrieval mechanism.

A computer storage-retrieval process was adopted. The data totalling over 2,600 entries was punched onto cards, primarily to ease manipulation of this bulk of information, and secondly so as to make it available to others, interested in pursuing research on the same trickling filter installation. Table 4.1 summarizes the various monthly entries giving, name, fortran coding name, location and format for each entry.

TABLE 4.1

Fortran Input Data - Storage Mechanism

Name	Fortran Coding	Location Card-Col.	Format	Units
Month	MONTH(J)	1-(1-8)	A8	-
Days/Month	DAYS(I,J)	1-(9-10)	I2	Days
Avg. Max. Air Temp.	TEMPMA(I,J)	1-(11-12)	I2	°F
Avg. Min. Air Temp.	TEMPMI(I,J)	1-(13-14)	I2	°F
Gross Flow (Influent flow and elutriation flow)	FLOW2(I,J)	1-(21-25)	F5.2	MGD
By Passed Flow	BYFLOW(I,J)	1-(26-31)	F6.2	MG/month
Elutriation Flow	FLOW14(I,J)	1-(32-35)	F4.2	MG/month
Recirculation Flow	FLOW3(I,J)	1-(36-39)	F4.2	MGD
Primary Tank Det. Time	DT1(I,J)	1-(40-43)	F4.2	Hrs.
Secondary Tank Det. Time	DT2(I,J)	1-(44-47)	F4.2	Hrs.
Gross BOD (Influent BOD and elutriation BOD)	BOD2(I,J)	1-(48-50)	I3	mg/l
Primary Effluent BOD	BOD5(I,J)	1-(51-53)	I3	mg/l
Trickling Filter Eff. BOD	BOD7(I,J)	1-(54-56)	I3	mg/l
Secondary Eff. BOD	BOD8(I,J)	1-(57-58)	I2	mg/l
Elutriation BOD	BOD14(I,J)	1-(59-62)	I4	mg/l
Gross S.S. (Influent S.S. and Elutriation S.S.)	SS2(I,J)	1-(63-65)	I3	mg/l

Table 4.1 cont'd

Name	Fortran Coding	Location Card-Col.	Format	Units
Primary Eff. S.S.	SS5(I,J)	1-(66-68)	I3	mg/l
Trickling Filter Eff. S.S.	SS7(I,J)	1-(69-70)	I2	mg/l
Secondary Eff. S.S.	SS8(I,J)	1-(71-72)	I2	mg/l
Elutriation S.S.	SS14(I,J)	1-(73-77)	I5	mg/l
Primary Sludge Flow	FLOW6(I,J)	2-(1-7)	F7.2	1000G/mon
Total Solids in Prim.Sludge	SL1TSP(I,J)	2-(8-11)	F4.2	Percent
Volatile Sludge in Prim. Sludge	SL1VSP(I,J)	2-(12-16)	F5.2	Percent
Secondary Sludge Flow	FLOW9(I,J)	2-(17-23)	F7.2	1000G/mon
Total Solids in Sec.Sludge	SL2TSP(I,J)	2-(24-27)	F4.2	Percent
Volatile Solids in Sec. Sludge	SL2VSP(I,J)	2-(28-32)	F5.2	Percent
Digested Sludge Flow	FLOW12(I,J)	2-(33-39)	F7.2	1000G/mon
Total Solids in Digested Sludge	DSL1TSP(I,J)	2-(40-43)	F4.2	Percent
Volatile Solids in Digested Sludge	DSL1VSP(I,J)	2-(44-48)	F5.2	Percent
Supernatant Liquor Flow	FLOW13(I,J)	2-(49-55)	F7.2	1000G/mon
Total Solids in Sup. Liq.	SLQTSP(I,J)	2-56-59)	F4.2	Percent
Volatile Solids in Sup. Liq.	SLQVSP(I,J)	2-(60-64)	F5.2	Percent

4.3.3 Data and Mass Balance

From the flow diagram presented in Figure 4.2, a total of sixteen stream stations were identified, of which thirteen were described in physical terms, i.e. station 2 through station 14. The data, previously stored onto cards was manipulated, and a mass balance of the plant performed. A computer program, referred to as MASBAL, written in Fortran IV for use on a 360/65 IBM Computer, performed the required mathematical manipulations.

Stream vectors were selected to describe quantitatively the performance of the plant at every station. Two major categories of stream vectors were adopted, depending on whether the flow consisted of wastewater or of sludge. The components of each category of stream vectors were:

- i) wastewater stream vectors: flow, BOD and SS concentrations.
- ii) sludge flow stream vectors: flow, total solids and volatile solids in percent.

Listed in Appendix A is both listing and output from the MASBAL program, giving the relevant data required in future chapters of this investigation. Four tables are used to summarize the yearly operation data of the plant:

- i) Table 1A: Stream vectors for stations 1,2,3,4,5
- ii) Table 2A: Stream vectors for stations 6,7,8,9,10,11

iii) Table 3A: Stream vectors for stations 12,13, 14 plus trickling filter loadings.

iv) Table 4A: BOD removal, SS removal, and miscellaneous information.

4.3.4 Temperature Manipulation

Of all the information reported in the annual reports of the ASA sewage treatment plant, only average maximum, and average minimum air temperatures were reported monthly. In view of the fact that the biological process is dependent on sewage temperature, rather than on air temperature, it was decided to convert reported air temperatures to corresponding sewage temperatures. Using the values of air and sewage temperature from eight activated sludge plants in Indiana (41), it was possible to regress successfully the data so as to obtain a predictive mathematical model for sewage temperature, described as:

$$T_{si} = a_0 + a_1 T_{ai} + a_2 T_{a(i-1)} \quad (4.1)$$

where T_{si} = predicted average monthly sewage temperature for month i , in $^{\circ}\text{F}$.

T_{ai} = reported average monthly air temperature for month i , in $^{\circ}\text{F}$.

$T_{a(i-1)}$ = reported average monthly temperature for month $(i-1)$, in $^{\circ}\text{F}$.

a_0 = coefficient equal to 41.96217.

a_1 = coefficient equal to 0.18005.

a_2 = coefficient equal to 0.26993.

The reported model, yielded a multiple correlation coefficient equal to 0.9362, with a standard error of the estimate of 2.81533.

The data presented in Appendix A, show both the reported average air temperature and the predicted average sewage temperature for each month. Tests were performed to determine which of the two temperatures (air or sewage) yielded better regression coefficients. All of the runs confirmed the use of the predicted average monthly sewage temperature based on Equation (4.1).

CHAPTER 5

BASIC CONCEPTS IN MODELLING AND SIMULATION

5.1 Introduction

The objective of this chapter is to familiarize the reader with some fundamental concepts in systems engineering and model building, and summarize important criterions for statistical model evaluation. Terms such as 'systems', 'models', 'parameters', 'elements', etc. are commonly used in every field, because they provide the structure for intelligent, well designed patterns of thought, yielding answers to a variety of realistic problems. The principles underlying such concepts are not mysterious, in fact even though the use of such terms has been accentuated within the last decade, because of the advent of high speed computers, the fundamental concepts have long since been with us (39,40).

Model building and systems simulation have proven extremely valuable tools, when applied intelligently. However, the systems approach to the solution of realistic situations, possesses serious disadvantages or limitations that must be recognized early, in order to avoid possible misapplication of the techniques. It is the application of such concepts to wastewater trickling filter treatment plants, along with some basic statistical techniques that will be discussed hereafter.

5.2 Systems Simulation

5.2.1 Components of a System

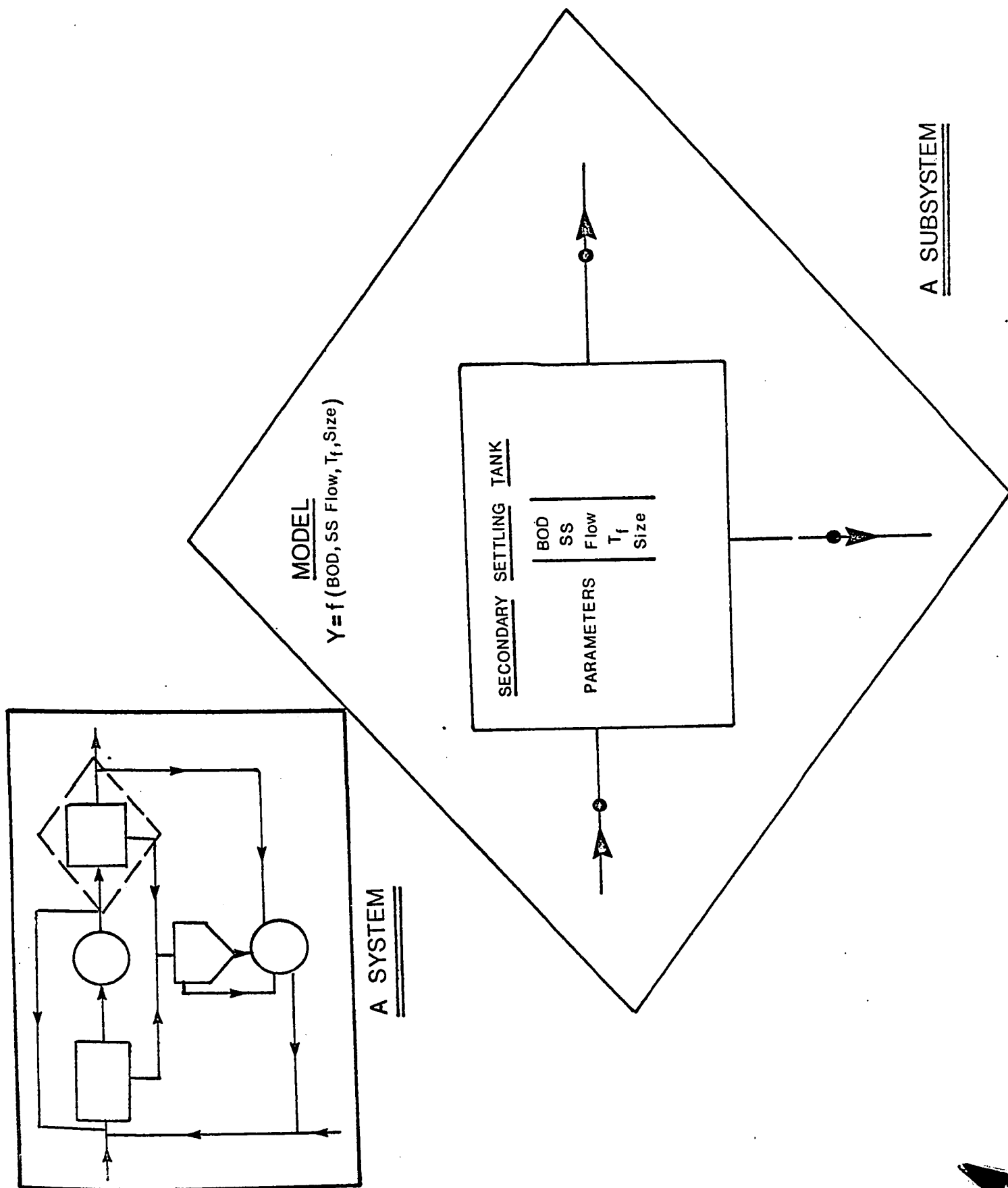
A system can be defined as the "assemblage of elements which are tied together by a common flow of material and/or information" (40). The elements of a system, also referred to as subsystems, interact together so as to determine the output of the entire system. A parameter can then be thought of as a property of the system, which can be assigned specific numerical values (40).

Furthermore, the field of systems engineering is defined by Hall and Dracup (39) as "the art and science of selecting from a large number of feasible alternatives, involving substantial engineering content, the particular set of actions which will accomplish the overall objectives of the decision makers," subjected to specific constraints.

Such concepts can very well be applied to the analysis of wastewater treatment plants. Figure 5.1 illustrates the components of the selected wastewater treatment plant when visualized as a system. It is however important to understand the relative meaning of those terms. What might be referred to herein as a system, for example, a treatment plant, can very well under a different context be referred to as a subsystem or even a parameter.

The ASA sewage treatment plant system is in fact a grouping of several elements or subsystems as illustrated

FIGURE 5.1 COMPONENTS OF A SYSTEM



in Figure 5.1. These subsystems are in turn a function of different parameters that determine their performance. The linkage of these elements with each other, establishes the overall plant performance through a technique known as simulation. The manner in which a system is planned, and its elements defined, is dependent upon the investigator's objectives. These must consequently be clearly established at the outset (39).

5.2.2 A Systems Approach

Because the applications of systems engineering tend to deal more and more with complex realistic problems, the analyst must consequently understand the mechanisms influencing the overall process performance. Himmelblau and Bischoff (40) in attempting to establish the performance dependency of a system, suggest the following steps:

1. Formulation of the problem, and delimitation of the objectives and criteria of importance. Delineation of performance requirements.
2. Inspection and classification of the process in order to break it down into subsystems.
3. Preliminary determination of the relationships among the subsystems.
4. Analysis of the variables and relationships to provide as simple and consistent a set as possible.

5. Mathematical modelling of the relationships in terms of the variables and parameters.

6. Model evaluation.

7. Application of the model and interpretation of the results.

The advantage of a systems approach, as might be inferred through the listing of the suggested steps, is that it will require the gathering and application of a variety of concepts from several disciplines, and only through a well structured program will anything meaningful evolve.

5.2.3 Systems Simulation

Hall and Dracup (39) defined simulation as a measure of reproducing the essence of a system without actually reproducing the system itself. In fact the system or part of it is examined by manipulating the mathematical expressions, i.e. the models representing it.

When the system represents a complex physical process, simulation provides an excellent opportunity to experiment on the process without having to perform such experiments on the prototype. It is in this respect that systems simulation possesses much of its advantages. Other benefits can be derived from simulation and can be described as follows:

1. Computer Control - Performance simulation aids in the operation and performance evaluation of existing

physical processes and was illustrated in the case of activated sludge plants (41).

2. Sensitivity Analysis - The sensitivity of basic parameters can be tested. For example, a 10% change in the input rate can either produce beneficial or detrimental effects on plant performance.

3. System Stability - The stability of the system can be examined when subjected to disturbances within the parameters.

4. Extrapolation - Although very dangerous, extrapolation can be performed so as to appreciate possible plant performance under unusual circumstances.

Obviously as suggested by Hall and Dracup (39) computer simulation can have serious limitations, depending on the particular situations. One of the most serious limitations of the process lies in the validity of the postulated models. Means of evaluating proposed models form an essential part of the systems approach.

5.3 Model Building

5.3.1 The Model and Its Relevance

As mentioned in Section 5.2 a model is a mathematical representation of what is happening in a subsystem or in an element of a system. The components of the model are referred to as parameters. Of the whole concept of systems analysis, model building figures as a key step in

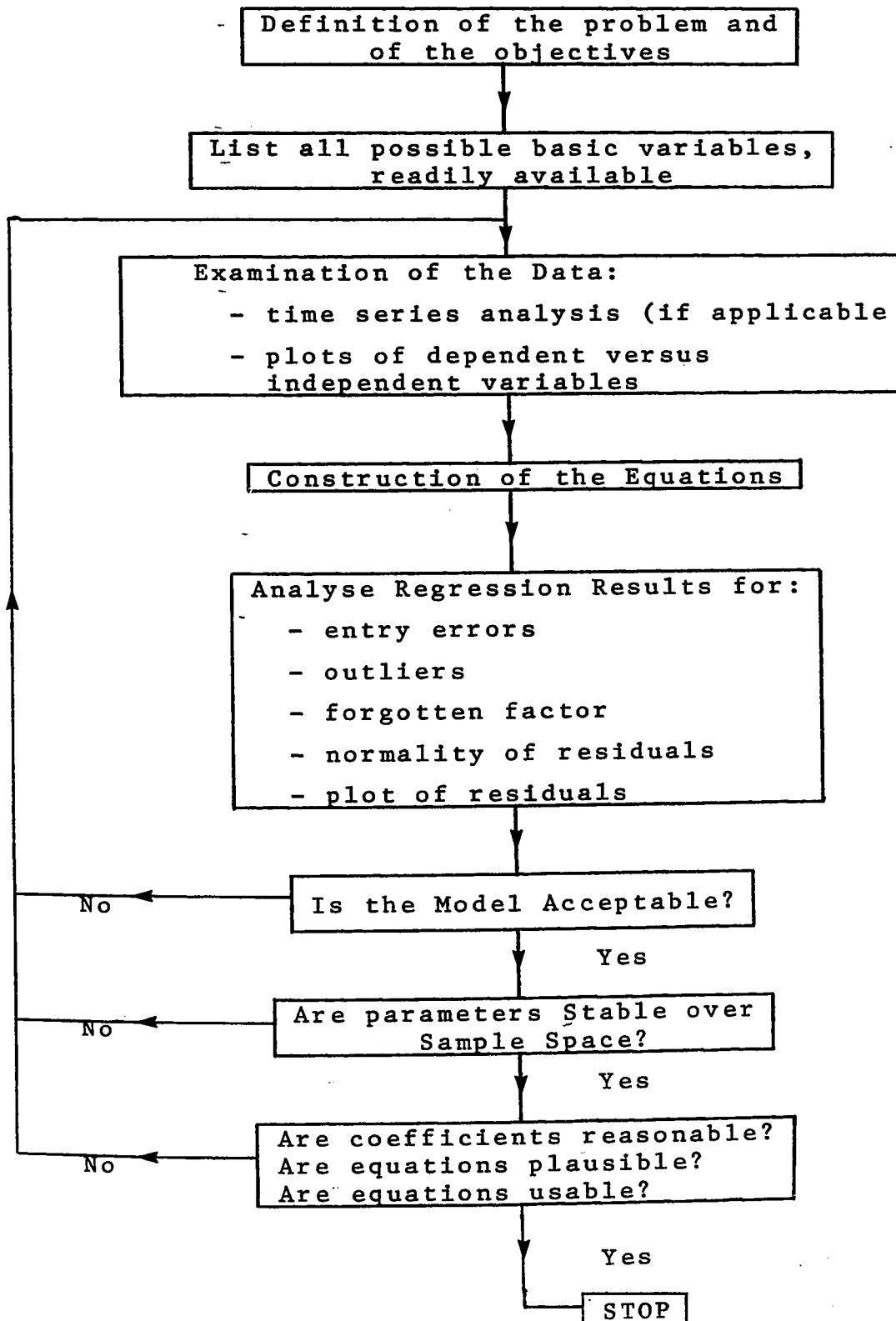
the successful implementation of a systems simulation study. If the models are not valid, then computer simulation is totally irrelevant.

5.3.2 Model Building Approach

A model building procedure which is in fact a combination of the efforts of Draper and Smith (42), and Daniel and Wood (43) is presented in Figure 5.2. Obviously, not all of the suggested steps are relevant to every model building situation. However, these steps are presented here in order to provide guidelines to successful mathematical model formulation.

Many techniques are available to regress data into some specific mathematical expressions. Some of the most common are available in computer program packages such as the BMD series of the Health Sciences Computing Facility of the University of California at Los Angeles, and the Scientific Subroutine Package (SSP) available from IBM. If the problem is one where the parameters are linear, then multiple linear regression techniques must be used. If, however, parameters are non linear, then non linear regression techniques, such as the one presented by Marquardt (44) must be adopted to perform the required manipulations.

FIGURE 5.2
MODEL BUILDING PROCEDURE



5.4 Statistical Concepts

5.4.1 General

It is not the purpose of this section to offer a complete statistical background necessary for the understanding of the latter chapters, but rather to survey some of the most important testing concepts essential in the evaluation of the mathematical models.

5.4.2 Evaluation of the Model

In order to determine whether the postulated mathematical models represent adequately the performance of the subsystems and/or systems, two methods are available: i) plot of residuals, and ii) examination of transformed residuals.

The residuals are defined as the difference between actual and predicted performance. They can be plotted in several ways such as discussed by Draper and Smith (42), in order to possibly detect particular trends. Such patterns would question the validity of the model because of previously established assumptions regarding the statistical characteristics of these residuals.

The use of normal plots and the examination of the 'Unit Normal Deviate' form of the residuals, can also establish the adequacy of a postulated model. Such an analysis is based on the assumption that for least squares

regression residuals are usually assumed to follow normal distributions with mean equal to zero, and standard deviation equal to σ^2 . Thus, if the postulated model is correct, then the residual mean square

$$s^2 = \frac{\sum_{i=1}^n e_i^2}{(n^* - p)} \quad (5.1)$$

where s^2 = residual mean square.

e_i = residual.

n^* = number of observations.

p = number of parameters within the model.

will estimate σ^2 . Therefore, the quantity expressed as (e_i/s) , and referred to as the Unit Normal Deviate form of the residual, can be examined so as to determine whether $e_i/\sigma \approx N(0,1)$. For example, with a 95% confidence level, not more than 5% of the Unit Normal Deviate (UND) form of the residuals must be outside the range of ± 1.96 .

5.4.3 Evaluation of the Parameters of a Model

Investigators often propose models with several parameters, not knowing, in fact, if all parameters are significant to the characterization of the process. Means of determining if parameters of a model are significant are available.

Consider the following simple linear model:

$$Y_i = B_0 + \sum_{j=1}^p B_j X_{ji} + e_i \quad (5.2)$$

Y_i = dependent variable.

X'_s = independent variables.

B'_s = coefficients.

p = number of parameters.

e_i = residuals.

and a specific coefficient, say B_j which can be approximated by b_j . Assuming that:

$H_0 ; B_J = 0$ (Null Hypothesis)

$H_1 ; B_J \neq 0$

then ' t_a ' = critical t value at a α probability level,

and ' t ' =
$$\frac{b_J - B_J}{\text{est. s.e. } (b_J)}$$

then if ' t ' is greater than ' t_a ', the null hypothesis is rejected, and the parameter is said to be significant in the regression. If, however, the computed ' t ' value is less than ' t_a ', then the null hypothesis cannot be rejected, and the parameter is said to have only a chance influence in the regression.

In this manner all parameters within a model can be tested as to their probable chance of having some influence in the mathematical expression.

5.4.4 A Note on the Types of Errors and Probability Levels

When involved in the testing of statistical hypotheses, two types of errors can be made, referred to by statisticians as Type I error (also Type α), and Type II error (also Type β). A Type I error involves the rejection of a null hypothesis that is actually true, and α is referred to as the probability of committing a Type I error. Thus if α is set at 0.05, i.e. 5%, the null hypothesis will be mistakenly rejected 5% of the time. However, the lower the value of α , the more chance there is of committing a Type II error.

Therefore, as can be inferred, a Type II error results when a null hypothesis that is actually false is maintained. The probability of making a Type II error is given by the value of β . Table 5.1 summarizes very clearly the errors previously discussed (45).

Because of the inherent conservatism of engineers and scientists, rejection criteria are set at fairly low levels, which indicates a greater willingness on the part of the researchers not to claim a good result, rather than to claim a false one (46).

Whether a Type I, or a Type II error is made, can never be asserted positively. However, researchers must always be very careful in selecting the rejection and acceptance levels for the testing of statistical hypotheses,

as they establish important criteria within the model building process.

TABLE 5.1

Errors in Testing Statistical Hypothesis

	H_0 is true	H_0 is false
Accept H_0	none	Type β Error
Reject H_0	Type α Error	none

CHAPTER 6

MODEL BUILDING

6.1 A Note on Regression and Probability Levels

Physical and biological processes, such as those described herein, seldom follow true linear relationships, as far as their efficiency and removal characteristics are concerned. In fact, Robertson, Wilson and Liebman (47) examined several trickling filter models, and concluded that most were of the non linear type, i.e. non linear in the parameters.

Many forms of mathematical (linear and non linear) models were investigated in this undertaking before finally adopting the following non linear model presented hereafter:

$$Y = B_0 \prod_{i=1}^p X_i^{B_i} e_i \quad (6.1)$$

where Y is the dependent variable.

X_i 's are the independent variables.

B_i 's are the coefficients to be evaluated.

e_i 's are the residuals.

i , and p are related to the number of parameters in the regression equation.

The objective of the least squares regression programs, whether they be linear or non linear, is to minimize the sum of squares of the deviations such that:

$$\phi = \sum_{j=1}^m (Y_j - \hat{Y}_j)^2 \quad (6.2)$$

is minimum. The variable $\hat{Y}$ refers to the predicted value of the dependent variable, while Y refers to the observed value of the dependent variable. The value of m is related to the actual number of observations.

Several methods for solving the non linear parameters are available, of which one has been investigated in this study for the purpose of the possibility of applying it to the present model building process. In cooperation with Draper (48), and Marquardt (49), a non linear algorithm referred to as NONLIN II and developed by Marquardt (50), was used.

The non linear model referred to by Equation (6.1) can be termed as intrinsically linear, because through simple mathematical manipulations the model can be transformed into a linear model, such as described by:

$$Y' = \ln Y = b_o^* + \sum_{i=1}^p B_i X_i^* + e_i^* \quad (6.3)$$

where $b_o^* = \ln B_o$

$X_i^* = \ln X_i$

$e_i^* = \ln e_i$

Examination of both linear and non linear regression techniques, applied to the Alexandria sewage treatment

plant data, revealed that by using the linearized form of the model, represented by Equation (6.3), and applying multiple linear regression to the transformed data, produced equally good results, if not better, than those obtained from the non linear regression algorithm of Marquardt. Appendix B, offers a detailed comparison of the two outputs, using the trickling filter unit as an example. It is however important to realize, that such conclusions are only relevant to the present investigation, and generalizations should be avoided.

Several multiple linear regression programs were available from the University of Ottawa Computing Center (UOCC). The BMD series developed at the University of California, offered a variety of techniques to regress data. The multiple linear regression program BMD03R, has been used successfully for the development of the models in the final stages of the investigation. The stepwise regression program, BMD02R, has been used to possibly fit second order polynomials to the data. However relatively poor improvement in the value of the multiple correlation coefficient (R), using second order polynomials, led solely to first-order model building.

As far as the adopted α -level for the possible rejection of the hypotheses, reported values applied to similar modelling processes varied from 5% (3) to 20% (41). Because of i) unknown quality control levels in the laboratory,

ii) possible errors in data transmission, and iii) unknown sampling and preservation techniques, an α -level of probability of 15% was finally decided upon. The author however realizes that a lower α -level for rejection of hypotheses could have been selected, had the data resulted from a carefully planned and controlled laboratory testing investigation.

6.2 Subsystem Arrangements

As indicated in the literature survey, most researchers have considered the analysis of the trickling filter - final settling tank system, rather than the trickling filter unit alone. There are at least two major reasons for such an approach: i) the highly dependent nature of the process on the removal, through settling, of the sewage particles from the trickling filter unit, and ii) most biological filter plants tend to recirculate part of the secondary settling tank sludge back to the primary tank.

However, in view of i) the particular physical layout of the Alexandria sewage treatment plant, ii) the particular interest given to the filter unit, iii) the specific objectives of the investigation, and iv) the need to establish means of comparing the results from the present investigation with other well-known mathematical models, both the trickling filter unit and the trickling filter - final settling tank subsystems were examined.

System configuration No. 1 is presented in Figure 6.1, and consists of a primary settling tank followed by a trickling filter - final settling tank unit. Models for both BOD and suspended solids removal have been developed and the notation referred to in Table 6.1 has been adopted in identifying the particular models.

TABLE 6.1

Labelling of Models in Configuration No. 1

Model	Primary Tank	Trickling Filter Final Settling Tank
BOD	PT1 - BOD	TF1 - BOD
SS	PT1 - SS	TF1 - SS

System configuration No. 2, shown in Figure 6.2 consists of i) a primary settling tank, ii) a trickling filter unit, and iii) a final settling tank. Table 6.2 again summarizes the corresponding model notation.

TABLE 6.2

Labelling of Models in Configuration No. 2

Models	Primary Tank	Trickling Filter	Secondary Tank
BOD	PT2 - BOD	TF2 - BOD	ST2 - BOD
SS	PT2 - SS	TF2 - SS	ST2 - SS

FIGURE 6.1 SUBSYSTEMS IN CONFIGURATION No. 1

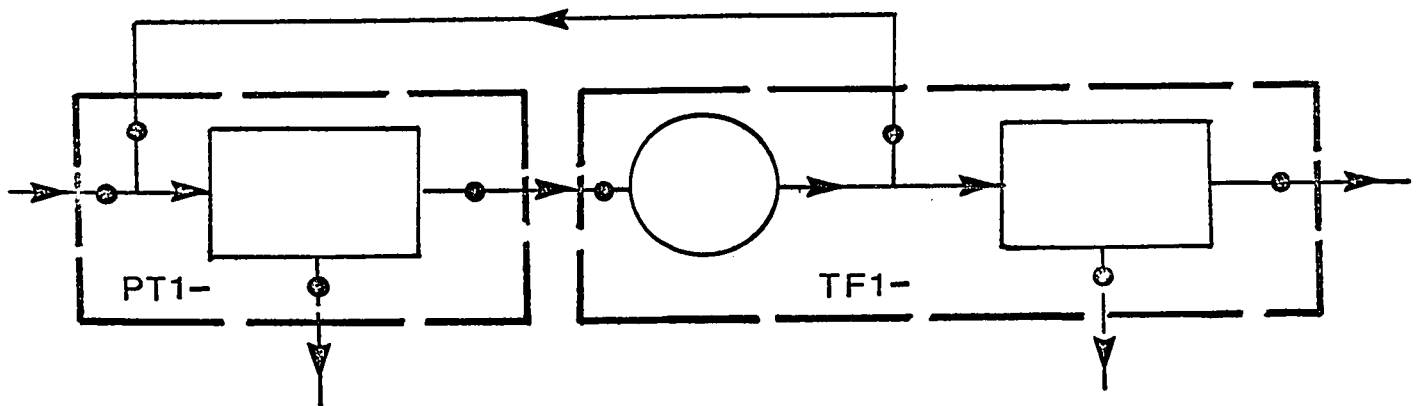
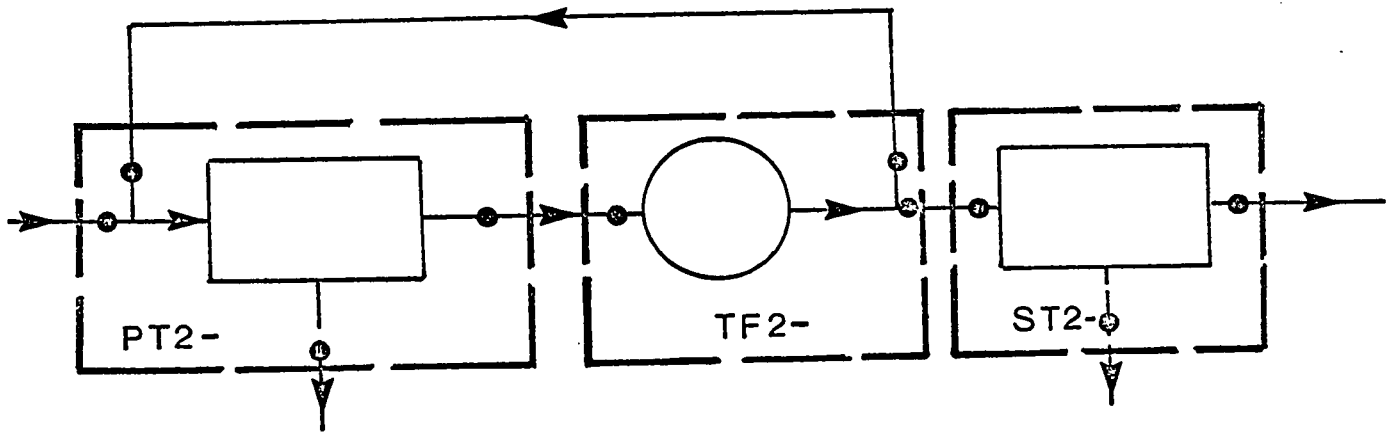


FIGURE 6.2 SUBSYSTEMS IN CONFIGURATION No.2



6.3 Models in Configuration No. 1

6.3.1 Primary Tank

The main purpose of the primary settling tank is the removal of suspended solids from the raw sewage. Because a certain fraction (approximately 70%) (51) of the BOD of domestic sewage is in the form of suspended solids a certain portion of the BOD is thereby removed through settling. Water Pollution Control Federation, Manual of Practice No. 8, (6) reports the range of suspended solids removal in existing primary settlers to vary from 40% to 70%, depending solely on the overflow rate, while BOD removals are reported to range from 20% to 50%.

In general, the effluent BOD from the different treatment units is also made up of certain solids and dissolved portions. Consequently BOD removal models for the various subsystems have taken into consideration, as a possible parameter, the effluent of suspended solids for the corresponding subsystems. This agrees with the work of other researchers that have simulated similar processes (41,51).

A. SS Model (PT1 - SS)

The following parameters have been considered as independent variables for possible inclusion into the primary tank suspended solids removal model (PT1-SS):

- 1) SS_{po} : suspended solids load applied in pounds per day,

2) ORl:overflow rate in gallons per day per square foot,
 3) T_f :sewage temperature in degree Fahrenheit, and 4) R_* :
 recirculation, defined as the ratio of the flow through
 the plant (Station 4, Figure 4.2) to the flow entering the
 system (Station 2).

In analyzing the regression models a summary and
 extension of the computer output has been presented in table
 form. The unsuccessful trials, but still of interest in
 establishing conclusions, have been presented in Appendix C.

Table C-1, of Appendix C, reports the results of
 the regression analysis using all the parameters suggested
 previously. However, such an analysis revealed that the
 recirculation parameter had only a 'chance' influence on the
 suspended solids removal efficiency. Eliminating the re-
 circulation component from the list of parameters, resulted
 in a better regression of the data as illustrated in
 Table 6.3, and expressed as:

$$\text{PT1 - SS: } SS_{pe} = \exp(1.84233) SS_{po}^{.46764} ORl^{.56284} T_f^{-0.19160} \quad (6.4)$$

B. BOD Model (PT1 - BOD)

As reported earlier, the suspended solids in the
 primary tank effluent have also been considered as having
 a possible significance in the regression. The list of
 possible parameters is therefore: 1) L_{po} : BOD applied in

TABLE 6.3

PT1 - SS Model

<u>1. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	SS _{po}	#/D	0.46764	11.57889	R = 0.9184
2	OR1	GPD/FT ²	0.56284	8.07685	R ² = 0.8434
3	T _f	°F	-.19160	-2.86648	RofR = 0.403
4	SS _{pe}	#/D			s = 0.06565
					Intercept = 1.84233

2. TESTS

a) Model

i) Residual: No definite pattern in the residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residual shows that only 2.4% of the residuals fall beyond the allowable range at 5% α-level.

b) Parameters

All computed 't' values are significant in the regression.

3. EQUATION

$$SS_{pe} = \exp(1.84233) SS_{po}^{.46764} OR1^{.56284} T_f^{-.19160}$$

pounds per day, 2) OR₁: overflow rate in gallons per day per square foot, 3) T_f: sewage temperature in degree Fahrenheit, 4) R_{*}: recirculation ratio, 5) SS_{pe}: primary effluent suspended solids in pounds per day.

Table 6.4 summarizes the relevant information pertaining to the adopted model.

$$\begin{aligned} \text{PT1 - BOD: } L_{pe} = \exp(-2.40024) L_{po}^{.62517} \text{OR}_1^{.58027} \\ T_f^{-.11353} R_*^{.30668} \text{SS}_{pe}^{.23933} \end{aligned} \quad (6.5)$$

6.3.2 Trickling Filter - Final Settling Tank Unit

A. SS Model (TF1 - SS)

The variables considered as possible candidates for inclusion in the TF1 - SS model are: 1) SS_{to}: suspended solids load applied to the filter in pounds per day per thousand cubic foot, 2) HR: hydraulic rate of flow in million gallons per acre per day, 3) R_{*}: recirculation ratio, 4) OR₂: overflow rate in the secondary settling tank, in gallons per square foot per day, 5) T_f: sewage temperature in degree Fahrenheit, and 6) BOD_{to}: BOD load applied to the filter in pounds per day per thousand cubic foot.

The first analysis, considering all of the above variables, revealed that parameters No. 2 and 4, i.e. the

TABLE 6.4

PT1 - BOD Model

<u>1. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	t value	Miscellaneous
1	L_{po}	#/D	0.62517	8.34869	$R = 0.9586$
2	OR1	GPD/FT ²	0.58027	5.10560	$R^2 = 0.9181$
3	T_f	°F	-0.11353	-1.48250	$RofR = 0.281$
4	R_*	--	0.30668	3.27843	$s = 0.06428$
5	SS_{pe}	#/D	0.23933	2.29289	Intercept = -2.40023
6	L_{pe}	#/D			

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of the residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals reveal that only 3.6% of the residuals fall beyond the allowable range at 5% α -level.

b) Parameters: Since $t_c = \pm 1.45$, all of the computed values are significant.

3. EQUATION

$$L_{pe} = \exp(-2.40023) L_{po}^{.62517} OR1^{.58027} T_f^{-.11353} R_*^{.30668} SS_{pe}^{.23933}$$

hydraulic rate of flow over the filter, and the rate of flow through the secondary settling tank, were not significant in the regression analysis, and their influence was attributed to a 'chance' effect. This is best illustrated in Table C-2, of Appendix C. The analysis of the subsystem was also performed by eliminating these variables one at a time so as to examine the possible dependence of the model to the others. In both instances the above mentioned parameters were observed to have only a chance influence. However, the withdrawal of variables No. 2 and 4, produced excellent results which are reported in Table 6.5. The adopted model for subsystem TF1 - SS was expressed as:

$$\text{TF1 - SS: } SS_{se} = \exp(0.89311) SS_{to}^{1.00295} R_*^{-1.21150} T_f^{-0.62581} L_{to}^{.26202} \quad (6.6)$$

B. BOD Model (TF1 - BOD)

The variables of possible significance in the efficiency evaluation of subsystem TF1 - BOD were selected as follows: 1) L_{to} : BOD load applied to the filter in pounds per day per thousand cubic foot, 2) HR: hydraulic rate of flow in million gallons per acre per day, 3) R_* : recirculation ratio, 4) OR2: overflow rate in the secondary settling tank in gallons per square feet per day, 5) T_f : sewage temperature in degree Fahrenheit and 6) SS_{se} : suspended solids

TABLE 6.5

TF1 - SS Model

1. REGRESSION RESULTS					
#	Symbol	Units	Regression Coefficient	t value	Miscellaneous
1	SS_{to}	#/D/1000FT ³	1.00296	6.74612	R = 0.9635
2	R_*	--	-1.21150	-10.98275	R ² = 0.9284
3	T_f	°F	-0.62581	- 5.87339	RofR = 0.413
4	L_{to}	#/D/1000FT ³	0.26202	2.30835	s = 0.09862
5	SS_{se}	#/D/1000FT ³			Intercept = 0.89311

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals shows that 3.6% of the residuals fall beyond the allowable range at 5% α -level.

b) Parameters: Since $t_a = \pm 1.45$, all parameters are significant into the regression.

3. EQUATION

$$SS_{se} = \exp(0.89311) SS_{to}^{1.00296} R_*^{-1.21150} T_f^{-0.62581} L_{to}^{0.26202}$$

load in the secondary settling tank effluent in pounds per day per thousand cubic foot.

The first analysis reported in Table C-3, in Appendix C, again revealed that variables No. 2 and 4, i.e. the hydraulic rate of flow over the filter, and the secondary settling tank overflow rate, had only a chance influence on the regression and consequently were classified as non-significant. Table 6.6 summarizes the relevant information pertaining to the adopted model, after the exclusion of the above mentioned parameters. The adopted model was expressed as:

$$\begin{aligned} \text{TF1 - BOD: } L_{se} = \exp(0.47213) L_{to}^{0.58288} R_*^{-1.42804} \\ T_f^{-0.21815} SS_{se}^{.43519} \end{aligned} \quad (6.7)$$

6.4 Models in Configuration No. 2

6.4.1 Primary Tank

As illustrated by the subsystem arrangements in Figures 6.1 and 6.2, the primary tank models for configuration No. 2, i.e. PT2 - BOD, and PT2 - SS, are identical to PT1 - BOD, and PT1 - SS, respectively. The development of these models has been presented in Section 6.3.2.

6.4.2 Trickling Filter Unit

The basic variables selected for possible inclusion

TABLE 6.6

TF1 - BOD Model

1. REGRESSION RESULTS					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	L_{to}	#/D/1000FT ³	0.58384	7.7058	$R = 0.9545$
2	R_*	--	-0.99173	-7.31843	$R^2 = 0.9770$
3	T_f	°F	-0.21830	-2.06022	$RofR = 0.399$
4	SS_{se}	#/D/1000FT ³	0.43427	5.61691	$s = 0.08473$
5	L_{se}	#/D/1000FT ³			Intercept = 0.47222

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of the residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals indicates that 4.8% of the residuals fall beyond the allowable range at 5% α -level.

b) Parameters: All computed 't' values imply the significance of the parameters in the regression model.

3. EQUATION

$$L_{se} = \exp(0.47) L_{to}^{0.58} R_*^{1.42} T_f^{-0.218} SS_{se}^{0.43}$$

in the suspended solids model for the trickling filter unit are: 1) SS_{to} : suspended solids load applied to the filter unit in pounds per day per thousand cubic foot, 2) HR: hydraulic rate of flow in million gallons per acre per day, 3) R_* : recirculation ratio, 4) T_f : sewage temperature in degree Fahrenheit, and 5) BOD_{to} : BOD load applied to the filter in pounds per day per thousand cubic foot. The analysis with all five variables resulted in the best fit, with a multiple correlation coefficient (R) of 0.9409. Table 6.7 summarizes the relevant information about the regression.

$$\begin{aligned} \text{TF2 - SS: } SS_{te} = & \exp(2.11470) SS_{to}^{.94714} HR^{-.66828} \\ & R_*^{-.26375} T_f^{-.42106} L_{to}^{.34868} \end{aligned} \quad (6.8)$$

B. BOD Model (TF2 - BOD)

After careful examination of the TF2 - BOD subsystem, the following variables were considered in the formulation of the trickling filter BOD removal efficiency (TF2 - BOD): 1) L_{to} : BOD applied to the filter in pounds per day per thousand cubic foot, 2) HR: hydraulic rate of flow in million gallons per acre per day, 3) R_* : recirculation ratio, 4) T_f : sewage temperature in degree Fahrenheit, and 5) SS_{te} : suspended solids load leaving the filter unit in pounds per day per thousand cubic foot.

The analysis examining the influence of all five parameters is reported in Table C-4, in Appendix C. This

TABLE 6.7

TF2 - SS Model

<u>I. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	SS _{to}	#/D/1000FT ³	0.94714	8.51857	R = 0.9409
2	HR	MGAD	-0.66828	-9.46715	R ² = 0.8853
3	R _*	--	-0.26370	-2.54862	RofR = 0.367
4	T _f	°F	-0.42106	-4.92861	s = 0.0731
5	L _{to}	#/D/1000FT ³	0.34868	3.70537	Intercept = 2.11470
6	SS _{te}	#/D/1000FT ³			

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of the residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals indicates that 3.6% of the residuals fall beyond the allowable range at 5% α-level.

b) Parameters: All of the computed 't' values indicate that the parameters are significant in the regression.

3. EQUATION

$$SS_{te} = \exp(2.11470) SS_{to}^{.94714} HR^{-.66828} R_{*}^{-.26370} T_f^{-.42106} L_{to}^{0.34868}$$

analysis revealed that variables No. 3 and 4 were non-significant in the regression. However, a second analysis, considering the effects of all the variables with the exception of recirculation, (variable No. 3), indicated the model to be acceptable. The computed 't' value of the temperature parameter was extremely close to the rejection level, as indicated in Table 6.8. However, the parameter was maintained in the formulation. A summary of the regression output appears in Table 6.8. The adopted model was expressed as:

$$\begin{aligned} \text{TF2 - BOD: } L_{te} = & \exp(-0.24521) L_{to}^{.69747} HR^{-0.32346} \\ & T_f^{-0.14605} SS_{te}^{0.69114} \end{aligned} \quad (6.9)$$

6.4.3 Secondary Settling Tank

In well operated high rate trickling filter plants the 'sloughing off' of the slime layer from the surface of the filter media can be assumed to be continuous. The purpose of the secondary settling tank is therefore to remove the biological mass resulting from the oxidation of the waste within the trickling filter unit. In addition, the tank provides additional settling time and surface area for the removal of inorganic matter still present in the wastewaters. The parameters of possible significance in the removal of both BOD and suspended solids within the secondary

TABLE 6.8

TF2 - BOD Model

<u>I. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	L_{to}	$\#/D/1000FT^3$	0.69747	6.33457	$R = 0.9546$
2	HR	MGAD	-0.32346	-2.36075	$R^2 = 0.9112$
3	T_f	$^{\circ}F$	-0.14605	-1.43501	$RofR = 0.465$
4	SS_{te}	$\#/D/1000FT^3$	0.69114	7.61130	$s = 0.08319$
5	L_{te}	$\#/D/1000FT^3$			Intercept = -0.24521

2. TESTS

a) Model

- i) Residuals: No definite pattern in the plot of residuals was observed.
- ii) U.N.D.: The Unit Normal Deviate form of the residuals indicate that only 1.2% of the transformed residuals fall beyond the allowable range at 5% -level.

- b) Parameters: The computed 't' value for the temperature parameter being very close to the rejection level of $t_c = 1.45$, it was decided to maintain the parameter. All other parameters were observed to be significant in the regression.

3. EQUATION

$$L_{te} = \exp(-0.24521) L_{to}^{0.69747} HR^{-0.32346} T_f^{-0.14605} SS_{te}^{0.69114}$$

settling tank can be summarized as follows: overflow rate, sewage temperature, and loadings of both BOD and suspended solids, depending on whatever model is considered. Table 6.9 summarizes the secondary settling tank parameters for both BOD and suspended solids models.

TABLE 6.9

Parameters for Secondary Settling Tank Models

ST2 Models		
Var. #	BOD Model	SS Model
1	L_{so} (#/DAY)	SS_{so} (#/DAY)
2	$OR2$ (GPD/FT ²)	$OR2$ (GPD/FT ²)
3	T_f (°F)	T_f (°F)
4	SS_{se1} (#/DAY)	SS_{se1} (#/DAY)
5	L_{se1} (#/DAY)	

A. SS Model (ST2 - SS)

In the suspended solids efficiency removal model for the secondary settling tank, the use of all four variables listed in Table 6.9, did not prove to be the best selection. Table C-5, in Appendix C, indicates that the temperature parameter appears only to have a 'chance' influence on the removal characteristics of the suspended solids from the secondary settler.

Table 6.10, illustrates the regression of the data using variables No. 1, 2 and 4, and summarizes the properties of the suspended solids efficiency removal model for the secondary settler. The adopted model expressed as follows, yielded a R value of 0.9795.

$$\text{ST2 - SS: } SS_{se_1} = \exp(-2.12146) SS_{so}^{1.04553} OR 2^{.20680} \quad (6.10)$$

B. BOD Model (ST2 - BOD)

Regression of the data with all five variables appearing in Table 6.9 did not yield the best fit to the data. Moreover, the results of such an analysis, reported in Table C-6, in Appendix C, reveal a chance influence of the sewage temperature parameter on the BOD removal within the final settling tank.

Withdrawal of the sewage temperature variable resulted in an acceptable model, whose characteristics are summarized in Table 6.11. The adopted model described as follows, yielded a R value of 0.9779.

$$\text{ST2 - BOD: } L_{se_1} = \exp(-1.56093) L_{so}^{.45429} OR 2^{.42325} SS_{se_1}^{.38054} \quad (6.11)$$

TABLE 6.10

ST2 - SS Model

I. REGRESSION RESULTS					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	SS _{so}	#/D	1.04553	22.12563	R = 0.9795
2	OR2	GPD/FT ²	0.20681	3.36102	R ² = 0.9594
3	SS _{se1}	#/D			s = 0.07344 Intercept = -2.12146

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals revealed that none of the transformed residuals fall beyond the allowable range at 5% α -level.

b) Parameters: All of the computed 't' values indicate that the parameters are all significant in the regression.

3. EQUATION

$$SS_{se1} = \exp(-2.12146) SS_{so}^{1.04553} OR2^{0.20681}$$

TABLE 6.11

ST2 - BOD MODEL

I. REGRESSION RESULTS

#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	L_{so}	#/D	0.45429	6.34489	$R = 0.9779$
2	OR2	GPD/FT ²	0.42325	5.69184	$R^2 = 0.9562$
3	SS_{se1}	#/D	0.38054	4.86739	$s = 0.08272$
4	L_{se1}	#/D			Intercept = -1.56093

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals reveal that none of the transformed residuals fall beyond the allowable range at 5% α -level.

b) Parameters: All of the computed 't' values indicate that the selected parameters are significant in the regression.

3. EQUATION

$$L_{se1} = \exp(-1.56093) L_{so}^{0.45429} OR2^{0.42325} SS_{se1}^{0.38054}$$

6.5 Summary

The mathematical models resulting from the linear regression analysis of the data are summarized in Table 6.12.

The models in subsystem configuration No. 1, (Figure 6.1) are used in a subsequent chapter (Chapter 8) of this report to compare the postulated removal efficiencies with other reported mathematical models commonly used in designing trickling filter installations.

The models in subsystem configuration No. 2, (Figure 6.2) are utilized in the next chapter for the sensitivity analysis and simulation of the treatment plant performance as a whole.

6.6 Discussion on the Actual Structure of the Selected Models

6.6.1 General

This section will attempt to explain and comment on any peculiarities found in the proposed mathematical models. The explanations put forward are based on i) biological and physical mechanisms, governing the processes, and ii) the work and experience of other researchers that have dealt with similar situations.

6.6.2 Primary Tank Models

The overflow rate, affecting the horizontal velocity within the settling tank, and the temperature, governing the viscosity of the water and consequently the

TABLE 6.12
Summary of the Developed Mathematical Models

PT1 - SS $SS_{pe} = \exp(1.84233) SS_{po}^{.46764} OR1^{.56284}$ $T_f^{-.19160}$ R: .9184	PT1 - BOD $L_{pe} = \exp(-2.40023) L_{po}^{.62517} OR1^{.58027}$ $T_f^{-.11353} R^{.30668} SS_{pe}^{.23933}$ R: .9586
TF1 - SS $SS_{se} = \exp(.89311) SS_{to}^{1.00296} R^{.21150}$ $T_f^{-0.62581} L_{to}^{0.26202}$ R: .9635	TF1 - BOD $L_{se} = \exp(0.47213) L_{to}^{.58288} R^{.42804}$ $T_f^{-0.21815} SS_{se}^{.43519}$ R: .9546
TF2 - SS $SS_{te} = \exp(2.11470) SS_{to}^{.94714} HR^{.66828}$ $R^{.26370} T_f^{-.42106} L_{to}^{.34864}$ R: .9409	TF2 - BOD $L_{te} = \exp(-0.24521) L_{to}^{.69747} HR^{-0.32346}$ $T_f^{-0.14605} SS_{te}^{.69114}$ R: .9546
ST2 - SS $SS_{se1} = \exp(-2.12146) SS_{so}^{1.04553} OR2^{0.20681}$ R: .9795	ST2 - BOD $L_{se1} = \exp(-1.56093) L_{so}^{0.45429} OR2^{.42325}$ $SS_{se1}^{.38054}$ R: .9779

settleability of the solids, are two parameters of known importance in the BOD and suspended solids removal efficiency of the primary settling tank. Therefore, as expected, both parameters have been included in both the BOD and suspended solids removal efficiency formulations.

Since the recirculated flow from the trickling filter effluent is a direct input into the primary settler, it was decided to test the significance of a dimensionless number representing the ratio of the flow through the tank to the flow entering the plant (recirculation ratio) on both BOD and suspended solids removal efficiencies. The recirculation ratio parameter proved however to be statistically non-significant for the suspended solids removal model. However, statistical evidence indicated its importance in the BOD removal model. Moreover, it was also observed that high recirculation ratios have a detrimental effect on the BOD removal process taking place in the primary tank. Explanations to such a phenomenon appear to lie in the characteristics of the sewage. The BOD of the raw sewage has been reported earlier to be approximately 70% in the suspended solid state and 30% dissolved. Trickling filter effluent is however expected to have a much lower BOD concentration in the suspended solids form with a corresponding higher dissolved BOD fraction. Consequently high recirculation ratios have a tendency to increase the percentage of dissolved BOD

fraction. Consequently, high recirculation ratios have a tendency to increase the percentage of dissolved BOD entering the primary tank, which cannot be removed through settling within the settling tank. The lower fraction of suspended solids accompanying high recirculation ratios tends, therefore, to decrease BOD removal efficiencies because large BOD fractions are in the dissolved state.

6.6.3 Trickling Filter - Final Settling Tank

Because of specific BOD removal mechanisms within the stone filter media the TFl - SS model included as one of its parameters the BOD applied to the trickling filter system. The organic matter is used by the microorganism population for their metabolism. The applied BOD is therefore a direct parameter in the suspended solids removal mechanism because it enhances the production of suspended solids. This does not however imply a net suspended solids production within the trickling filter unit. In fact Grantham (23) observed a net suspended production within the first section of the filter unit, reaching at times as much as 100%. However, such a suspended solids concentration soon decreased well below the applied levels in the latter portions of the filter. In fact, as indicated by the present model, Grantham (23) observed a net suspended solids removal ranging from 40 to 70% within the trickling filter unit alone.

The effluent suspended solids loading was also used as input parameters in the TF1 - BOD model, as explained previously. Both suspended solids and BOD models did not prove to be sensitive to the hydraulic rate of flow, nor did it seem affected by secondary settling tank overflow rate as might have been inferred through the literature survey.

6.6.4 Trickling Filter Unit

The trickling filter unit models, (BOD and suspended solids) differ from one another in only one parameter, i.e. recirculation ratio. Even though it appears that the BOD model is not sensitive to variations in the recirculation ratio, it can very well be established that there exists a recirculation influence reflected within the model. In fact the effluent suspended solids is a function of the recirculation ratio, and because the effluent BOD is also a function of the effluent suspended solids, variations in the recirculation ratio will consequently affect the BOD of the effluent.

Obviously the same comment, concerning the BOD loading applied to the unit, in the suspended solids model reported under Section 6.6.3, still applies to the trickling filter unit model.

6.6.5 Secondary Settling Tank

Secondary settling tank models are not as elaborate as the previous models because of the reduced number of

possible parameters in the performance evaluation of BOD and suspended solids removal efficiencies. In fact the overflow rate, sewage temperature and effluent suspended solids were the only possible significant parameters in the mathematical model building process.

The temperature parameter revealed to be non-significant in both the BOD and suspended solids models, while the overflow rate was observed to be the most significant parameters for both removal processes within the secondary settling tank.

CHAPTER 7

SENSITIVITY ANALYSIS

7.1 Theory

The objective of this chapter is to identify parameters of significant or critical importance in the operation of a high rate trickling filter plant, such as described mathematically in Chapter 6. Sensitivity analysis of a system, or of a mathematical model, offers means of delineating such responses. Input components to a system form the core information to the performance simulation and are considered essential to the sensitivity analysis. The concept underlying the technique is relatively straightforward: all of the input parameters are held constant with the exception of one parameter, which is allowed to vary through its entire range. The response, i.e. the system's behaviour to such variations, is observed and analysed.

A procedure adopted by MacBeath and Eliassen (52) has been used to analyse the sensitivity of mathematical models to the various input parameters (or independent variables). The selected technique is presented hereafter:

1. List the independent variables of the model. Provide three values for each - expected high, low and nominal values.

2. Calculate the response surface variable using the nominal value. This value will serve as a reference point.

3. Compute a percentage change in the response surface by substituting one of the high values of one of the independent variables. Replace the high value with the nominal value.

4. Repeat Step 3 for each of the high and low values.

Such a procedure, should then delineate the important parameters in the systems performance. Care should, however, be exercised so as to determine interdependencies among the parameters and thus possibly analyse group variations of the variables.

7.2 Sensitivity Analysis: Sewage Characteristics

7.2.1 Characteristics of the Algorithm

The objective of the algorithm described hereafter, and whose listing appears in Appendix D, is to simulate the operation of the Alexandria sewage treatment plant, so as to delineate parameters of significant importance in the overall BOD and suspended solids removal efficiencies of the system. Stream vectors at various stations are evaluated along with the efficiency of the various units. The performance evaluation proceeds using the mathematical formulations developed from configuration No. 2, in Section 6.3 (Figure 6.2).

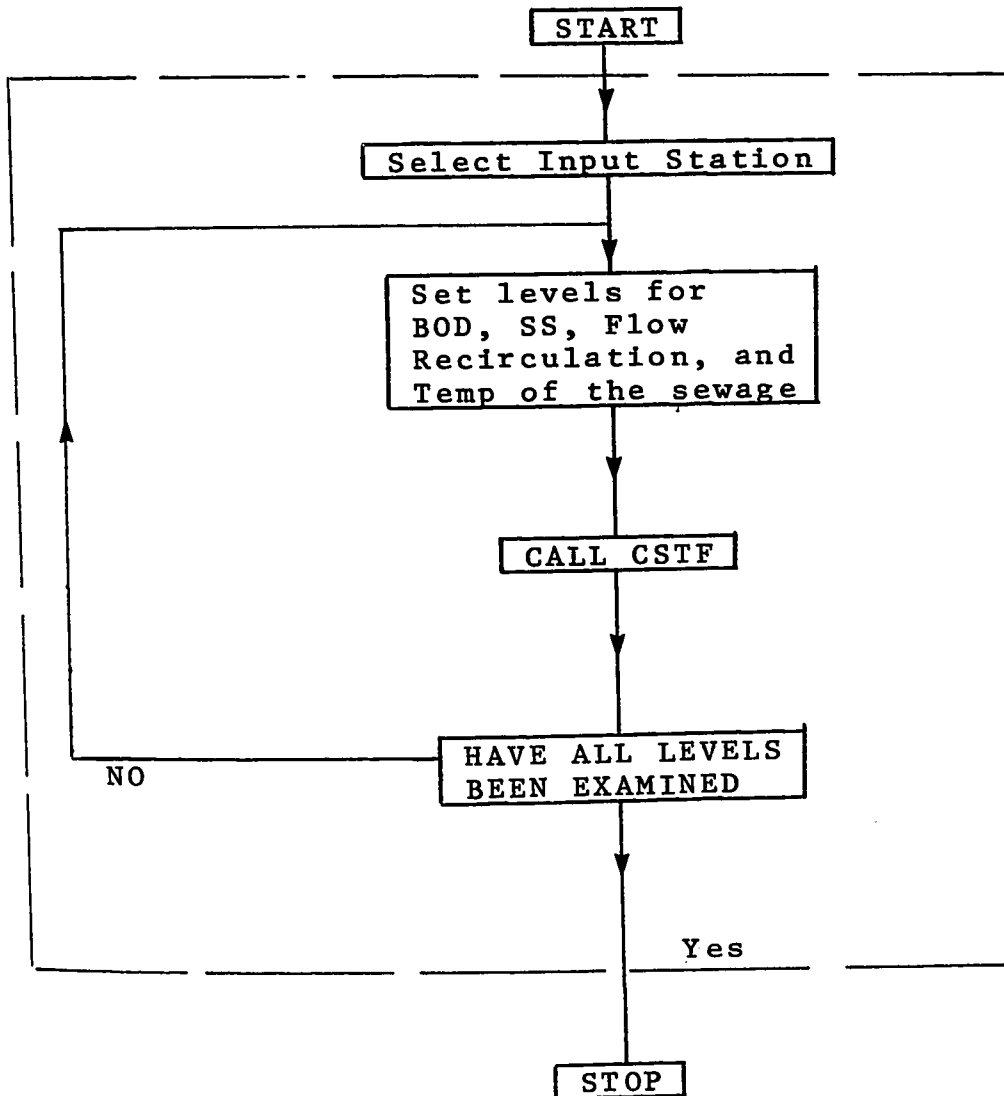
The flow chart, as shown in Figure 7.1, illustrates the principles underlying the calculations. The overall process can be viewed as three groups of equations in series subjected to a convergence criterion in the characteristics (BOD and suspended solids) of the trickling filter recirculated flow. The program referred to as CSTF (Computer Simulation of a Trickling Filter) was written in Fortran IV, for use on an IBM 360/65 computer.

7.2.2 Input Data

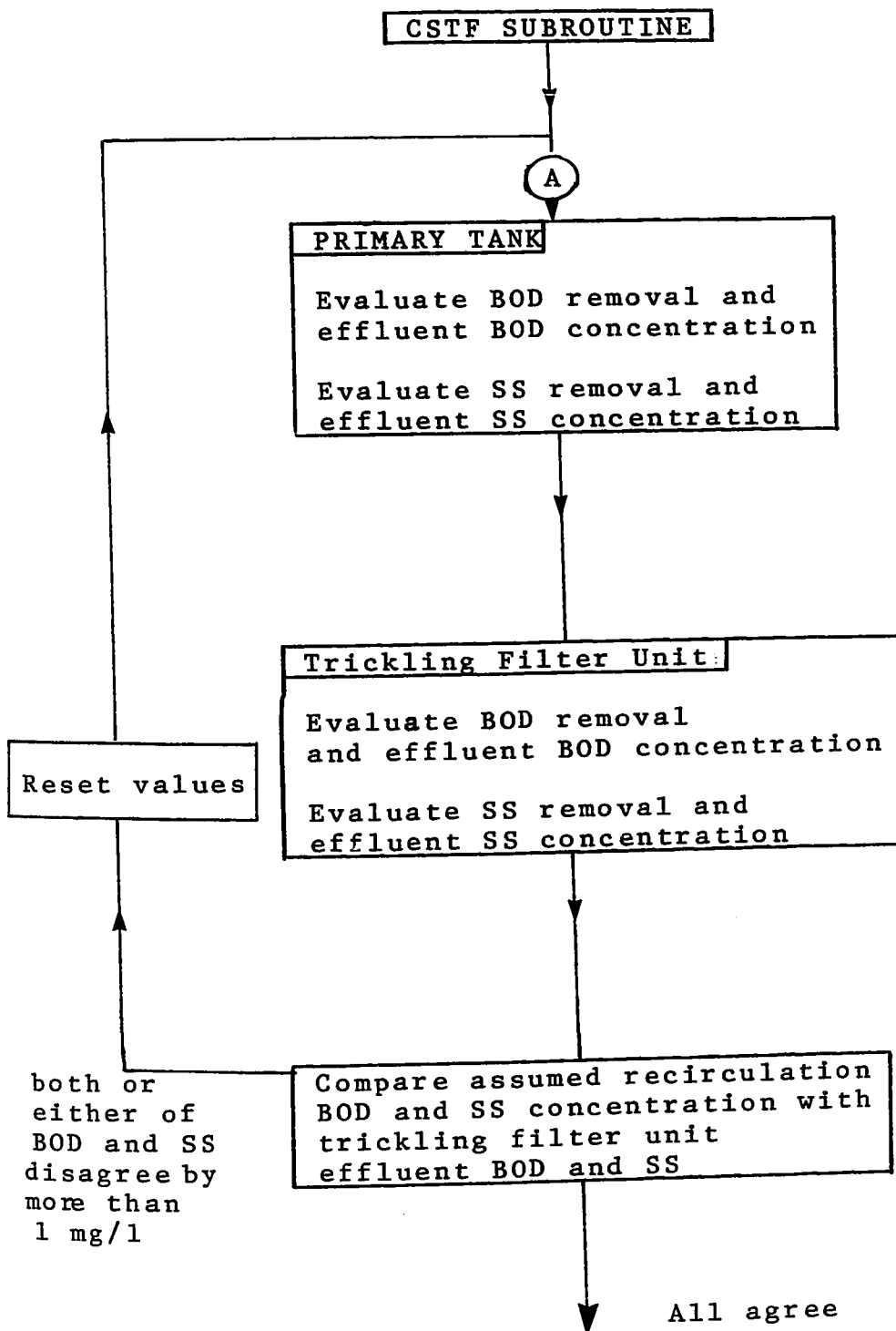
The flow diagram of the Alexandria sewage treatment plant, Figure 4.2, indicates two possible stream vector locations that could be used as input stations to the simulation and sensitivity analysis, namely station No. 2 and station No. 4, of Figure 4.2. Consideration of the objectives, presented in Chapter 3, dictates the selection of station No. 4 as input station. The input station thus selected gives the readers the opportunity to examine and appreciate more explicitly the effects of recirculation on the performance of the plant. Because of the particular physical layout of the treatment plant, the selection of the alternate input station, i.e. station No. 2, would have complicated the objectives somewhat. In fact, a change in the recirculation would have implied a simultaneous change in the flow rate through the plant. Therefore, variations in the

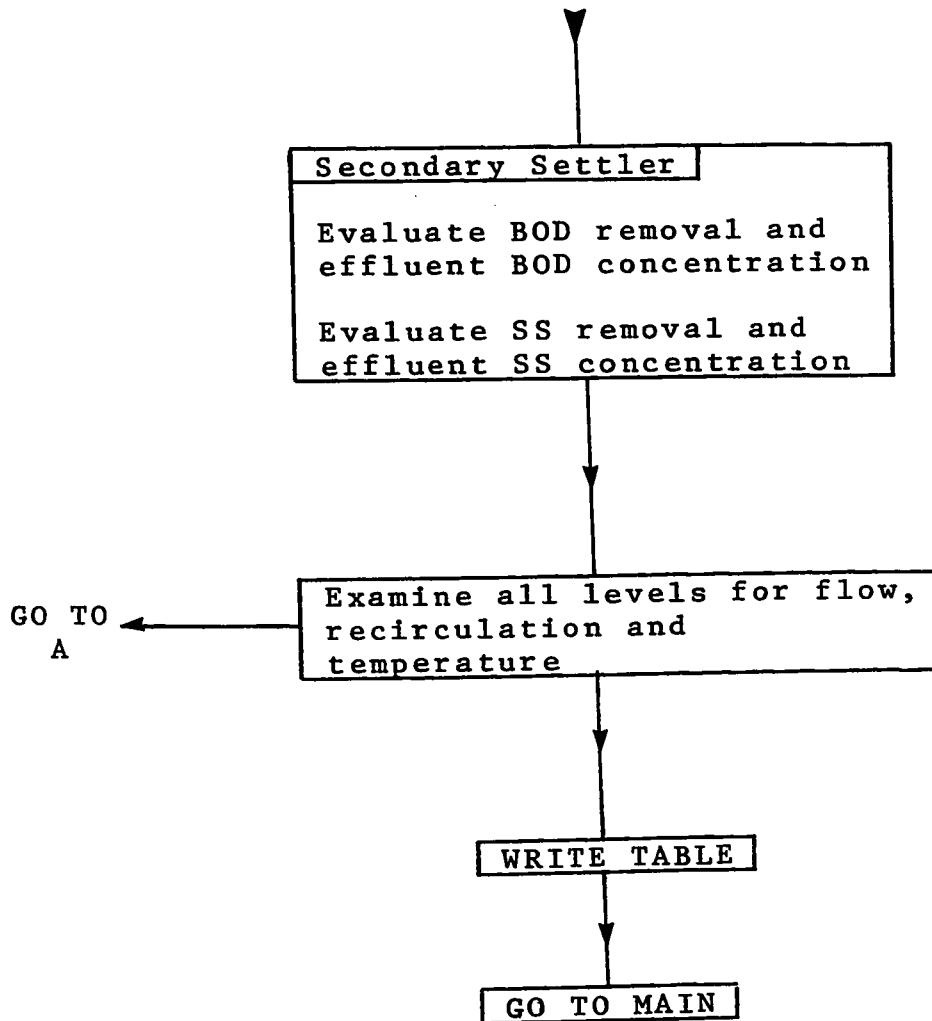
FIGURE 7.1

FLOW CHART FOR THE CSTF PROGRAM



MAIN
PROGRAM





recirculation ratio parameter would have resulted in a joint variation of the rate of flow, making it difficult to evaluate the origins of the performance variations. The reader must, however, understand that the selection of station No. 4 as the input station, has been selected only because it is the objective of this paper to ascertain whether the ratio of recirculated flow, under constant hydraulic conditions bears some influence on the removal mechanisms. Furthermore, the reader must understand that station No. 4 would not be the selected input station for a true computer performance simulation. Under such circumstances, the selection of input station No. 2 would be dictated and very minor changes in the CSTF program would be necessary. In summary, therefore, input station No. 4 has been selected for the sensitivity analysis of the models, whereas input station No. 2 must be selected for a true performance simulation of the plant as would be required for computer assisted plant operation.

Two separate sensitivity analyses for the changes in sewage characteristics were carried out. The first, consisted in evaluating the percentage change in the sewage characteristics (BOD and suspended solids) through the entire plant subjected to input parameter variations. The second, evaluated the percentage change in BOD and suspended solids removal efficiencies of the individual treatment units for which mathematical models had been developed, subjected to similar input parameter variations.

The input matrix presented in Table 7.1 was utilized for the sensitivity analysis of the treatment plant as a whole. It consists of the three stream vectors, found at station No. 4 (BOD, suspended solids, and flow) plus the recirculation and temperature parameters for high, mean and low expected conditions. A total of 243 simulations, covering all possible levels of operation within the range of the available data resulted from such an analysis. The results of this first sensitivity analysis are reported in Appendix E.

Table 7.2, describes the four input matrices used in the sensitivity analysis of each individual treatment unit, the results of which are summarized in Table 7.4.

TABLE 7.1
Input Matrix for the Sensitivity Analysis of
the Treatment Plant as a Whole

Parameters	High	Mean	Low
BOD mg/l	240	169	125
SS mg/l	278	178	119
Flow MGD	31.06	24.29	15.70
Recirculation	1.57	1.31	1.05
Sewage Temp °F	78	67	56

TABLE 7.2
Input Matrices for the Sensitivity Analysis of the Individual Treatment Units

PRIMARY SETTLING TANK				TRICKLING FILTER SYSTEM			
Parameters	High	Mean	Low	Parameters	High	Mean	Low
BOD mg/l	240	169	125	BOD mg/l	152	117	87
SS mg/l	278	178	119	SS mg/l	117	90	63
Flow MGD	31.06	24.29	15.70	Flow MGD	31.01	24.25	15.67
Recirculation	1.57	1.31	1.05	Recirculation	1.57	1.31	1.05
Sewage Temp °F	78	67	56	Sewage Temp °F	78	67	56
SECONDARY SETTLING TANK				TRICKLING FILTER UNIT			
Parameters	High	Mean	Low	Parameters	High	Mean	Low
BOD mg/l	103	79	55	BOD mg/l	152	117	87
SS mg/l	91	70	49	SS mg/l	117	90	63
Flow MGD	24.05	18.50	12.95	Flow MGD	31.01	24.25	15.67
Recirculation	1.57	1.31	1.05	Recirculation	1.57	1.31	1.05
Sewage Temp °F	78	67	56	Sewage Temp °F	78	67	56

7.2.3 Analysis and Results

Table 7.3 summarizes the percentage change in the sewage characteristics through the treatment plant when subjected to the input matrix shown in Table 7.1. Table 7.4 offers a summary of the percentage change in the sewage characteristics of each individual treatment units when subjected to specific variations in the input parameters, as described in matrix form in Table 7.2. Moreover, Tables 7.3 and 7.4 also examine the effects of dry and wet weather flow conditions on the sewage characteristics through the treatment plant, and on the efficiency of each treatment units as indicated in Tables 7.3 and 7.4, respectively. Dry weather flow conditions are characterized by high BOD and suspended solids, accompanied by low flows, while wet weather flow conditions prevail under low BOD and suspended solids, and high flows.

Analysis of the sensitivity test results will hereafter be presented. A response greater than or equal to $\pm 15\%$ has been selected as a criterion for sensitivity (42). Several situations have been considered for both the sensitivity of the treatment plant as a whole and of the treatment units individually. Table 7.5 summarizes these situations, identified as A through F, and corresponding to BOD, suspended solids, recirculation, temperature, and weather variations, respectively.

TABLE 7.3

Progressive Percentage Change in the Sewage Characteristics of the Treatment Plant
as a Whole

BOD	SS	FLOW	R	T	Primary Tank Effluent			Trickling Filter Effluent			Secondary Settling Tank Effluent		
					BOD mg/l	% Δ	SS mg/l	BOD mg/l	% Δ	SS mg/l	BOD mg/l	% Δ	SS mg/l
A	A	A	A	A	117		90	79		70	50		51
A+30					146	+24.8	90	95	+20.3	74	56	+12.0	54
-30					97	-17.1	90	66	-16.4	64	45	-10.0	46
	+30				123	+5.1	111	93	+17.7	83	58	+16.0	61
	-30				112	-4.3	75	67	-15.2	57	43	-14.0	41
		+30			131	+11.9	91	82	+3.8	66	55	+10.0	55
		-30			96	-17.9	89	71	-10.2	74	42	-16.0	48
			+20		124	+6.0	90	80	+1.3	71	48	-4.0	49
			-20		109	-6.8	90	77	-2.5	71	54	+8.0	55
				+15	114	-2.6	88	71	-10.2	62	46	-8.0	45
				-15	120	+3.3	93	88	+11.4	76	55	+10.0	55
F+30	+30	-30			126	+7.7	110	103	+30.5	98	56	+12.0	65
-30	-30	+30			103	-11.1	75	60	-24.1	51	43	-14.0	39

TABLE 7.4
Percentage Change in the Sewage Characteristics of the Individual Treatment Units

	BOD	SS	FLOW	R	T	Primary Tank		Trickling Filter- Final Settler System		Trickling Filter System		Secondary Settling Tank	
						BOD	SS	BOD	SS	BOD	SS	BOD	SS
A	+30					+24.8	0.0	+19.7	+6.5	+20.0	+6.0	+11.8	0.0
	-30					-17.1	0.0	-19.7	-9.7	-18.0	-8.0	-15.7	0.0
B		+30				+5.1	+23.1	+12.9	+29.8	+12.0	+28.0	+11.8	+33.4
		-30				-4.3	-18.7	-16.1	-32.3	-14.0	-30.0	-13.7	-33.4
C			+30			+11.9	0.0	+3.2	+6.5	+4.0	+6.0	+9.8	+7.8
			-30			-17.9	-2.2	-6.5	-12.9	-6.0	-8.0	-11.8	-7.8
D				+20		+6.0	0.0	-29.0	-19.3	-10.0	-4.0	0.0	0.0
				-20		-6.8	0.0	+54.8	+29.0	+12.0	+4.0	0.0	0.0
E					+15	-2.6	-3.3	-6.5	-9.7	-8.0	-10.0	0.0	0.0
					-15	+3.3	+3.3	+9.7	+12.9	+8.0	+10.0	0.0	0.0
F	+30	+30			-30	+7.7	+20.9	+25.8	+22.6	+26.5	+22.0	+9.8	+19.6
	-30	-30			+30	-11.1	-17.6	-32.3	-35.5	-28.0	-32.0	-19.6	-25.5

Table 7.5

Selected Input Conditions

BOD	SS	Flow	R	T	Analysis	Remark
<u>+30%</u>	<u>+30%</u>	<u>+30%</u>	<u>+20%</u>	<u>+15%</u>	A B C D E	Individual Variation
<u>+30%</u>	<u>+30%</u>	<u>+30%</u>			F	Group Variation

Analysis A: Variations in the BOD concentration entering the primary settling tank, produces significant variations in the effluent BOD leaving both the primary settler and the trickling filter unit, as reported in Table 7.3. In fact, an increase in the BOD concentration at station No. 4, Figure 4.2, results in an increase BOD concentration throughout the entire plant. The effects appear, however, to be attenuated as the wastewaters progress through the units. The BOD of the effluent from the primary tank and trickling filter unit showed significant variations, while the overall BOD plant response can be termed as moderately sensitive. In all cases an increase in the input BOD concentration resulted in an increase BOD from the various units of the treatment plant, and vice versa.

As expected the suspended solids of the sewage throughout the treatment plant appeared only to suffer moderate variations with respect to BOD input changes. An increase in the BOD applied to the treatment plant, resulted in a moderate increase in the suspended solids of the wastewaters throughout, with a similar inverse effect.

As far as the individual units are concerned, most units showed a significant effluent BOD increase, resulting from the application of higher BOD concentrations to the units individually. The secondary settling tank unit, showed the least significant variations but the change is still considered to be significant. In all instances increased input

BOD concentrations resulted in increased effluent BOD concentrations, and vice versa.

The suspended solids concentration in the effluent of the individual unit was either non sensitive to BOD variations (as was observed for the primary settler and secondary settler units), or moderately affected (as observed with the trickling filter unit and the trickling filter system). In both instances, an increased BOD concentrations resulted in higher suspended solids concentrations in the effluent of the individual units, but the effect was not significant.

Analysis B: Variations in the concentration of suspended solids applied to the primary tank produced significant variations in the suspended solids concentration of the effluent throughout the treatment plant. The progressive percentage suspended solids changes, as reported in Table 7.3, were all significant. More specifically, the application of higher suspended solids concentrations produced equivalently higher concentrations of suspended solids in the effluent throughout, and vice versa. In fact, a 30% increase in the applied suspended solids concentration resulted in an overall increase of 19.7% in the effluent suspended solids concentration.

Because of the actual structure of the models and their dependency, high suspended solids resulted in

significantly higher trickling filter effluent and final effluent BOD. The opposite variation having, as expected, the inverse effect on the trickling filter and final effluent BOD.

The response of the individual treatment units to suspended solids variations, is reported in Table 7.4. The percentage change in the effluent suspended solids from all of the units revealed significant. In fact, all units showed higher effluent suspended solids concentration corresponding to the application of higher suspended solids concentration, and vice versa. Moreover, the trickling filter unit and the trickling filter system showed similar significant variation in their effluent BOD concentration. The trickling filter system responded by a 16.1% decrease in BOD concentration, corresponding to a 30% decrease in the applied suspended solids concentration. A similar percentage increase was observed with higher suspended solids concentrations.

Analysis C: The effects of flow variations onto the percentage change in the sewage characteristics (BOD and suspended solids) is reported in Table 7.3. As far as the progressive change in effluent BOD throughout the plant is concerned, the upper flow limit yielded higher BOD concentrations throughout, with an overall plant BOD increase of 10%. The behaviour of the plant to lower flow levels was, however, more sensitive, yielding a 16% decrease in the

effluent BOD concentration. The effluent suspended solids concentration, however, did not reveal sensitive to flow variations. In fact, the primary settling tank showed negligible responses to flow variations even of the order of $\pm 30\%$. This phenomena can most probably be accounted for by a fairly conservative hydraulic design, unaffected by flow variations. The overall percentage change in the suspended solids was found to be moderately sensitive, with higher effluent suspended solids accompanying higher flows, and vice versa.

Individually, the treatment units responded differently. Both settling tanks, showed moderate BOD and suspended solids variations accompanying flow variations with the exception of the primary settler suspended solids removal. High flows resulted in moderately higher BOD and suspended solids concentrations. However, the biological unit, i.e. the trickling filter unit, showed a different response. Higher flows in this case appeared to decrease both suspended solids and BOD concentrations, while lower flows produced the opposite effect. Such a response appears to corroborate the theories of increased BOD removal under turbulent flow regime.

Analysis D: The recirculation ratio effects onto the percentage change in the sewage characteristics for the treatment plant as a whole are presented in Table 7.3. Even

though high recirculation ratios do not appear to benefit primary tank BOD removal, the overall effect on the treatment plant is, however, different. In fact, higher recirculation ratios have a tendency to decrease plant effluent BOD, while lower recirculation hinders plant performance by increased effluent BOD concentration. The same general pattern is also observed with the suspended solids in the plant effluent, as shown in Table 7.3.

As far as the individual treatment units are concerned, both primary and secondary settling tanks, suspended solids removal, along with the BOD removal in the secondary settler, are not sensitive to recirculation ratio variations. The data in Table 7.4 indicates that high recirculation ratios produce higher primary effluent BOD, and vice versa. However, a considerable decrease in the trickling filter system BOD and suspended solids concentration accompanies higher recirculation ratios. A similar effect is also observed within the trickling filter unit under identical conditions, but to a much lesser extent. The explanation to such a response appears to lie in the fact that lower secondary tank overflow rates accompany high recirculation ratios, thus giving more opportunity for both BOD and suspended solids removal.

Analysis E: The data presented in Table 7.3 illustrates that a better effluent quality, both in BOD and suspended solids, is obtained with higher sewage temperatures. The progressive

sewage characteristics, indicate a moderate decrease in effluent BOD and suspended solids concentrations with a rise in sewage temperature of approximately 15%.

Individually, the subsystems follow a similar pattern, with the exception of the final settling tank, which has shown to be non-sensitive to temperature variations. All other units, under individual analysis, have shown a moderate increase in the BOD and suspended solids concentrations, accompanying lower temperatures, and vice versa.

Analysis F: The results of the sensitivity analysis, simulating dry and wet weather flow conditions reveal that overall significant increases in the waste characteristics, both BOD and suspended solids, accompany dry weather flow conditions. Wet weather flow conditions are characterized by a decrease in both BOD and suspended solids concentrations of the wastewaters throughout the treatment plant.

Individually, the subsystems responded in the same manner, showing significant BOD and suspended solids concentration increase when dry weather flow conditions prevailed, and vice versa.

7.3 Sensitivity Analysis: Sludge Production

7.3.1 General

The sludge to be disposed from the treatment plant originates from two different sources: the primary

and the secondary settling tanks. Because of the costs, capital and maintenance, associated with the handling and pumping of sludge, a sensitivity analysis examining the change in the total sludge production with corresponding variations in the input parameters was motivated. The input matrix presented in Table 7.1 was used to establish the desired responses. Table 7.6 shows the change in primary, secondary and total sludge production resulting from the different input situations. Moreover, the proportion of primary and secondary sludge is also reported in the Table. The conditions summarized in Table 7.5 have been selected to analyse more systematically the results.

7.3.2 Sludge Production Analysis

The following section summarizes the general sludge production responses to the specified input parameter variations.

Analysis A: The percentage change in the total sludge production, i.e. primary plus secondary, as illustrated in Table 7.6 revealed to be non-sensitive to BOD variations. The negligible trend observed in total sludge production indicated a very small increase in the pounds of sludge to be disposed with high BOD concentrations, and vice versa. The secondary sludge production appeared to be more sensitive. However, secondary sludge production generally accounts for only 10 to 15% of the total sludge production.

TABLE 7.6
Sludge Production

BOD	SS	FLOW	R	T	Primary Sludge			Secondary Sludge			Total Sludge	
					Pounds	%Change	% Comp	Pounds	%Change	%Comp	Pounds	%Change
A	A	A	A	A	17710	0.0	86.0	2883	0.0	14.0	20593	0.0
+30					17710	0.0	85.4	3026	+5.0	14.6	20737	+0.7
-30					17710	0.0	86.9	2665	-7.6	13.1	20375	-1.1
	+30				33714	+90.4	91.0	3343	+16.0	9.0	37057	+80.0
	-30				8906	-49.7	78.7	2408	-16.5	21.3	11314	-45.1
		+30			22469	+26.9	88.6	2891	+0.2	11.4	25360	+22.4
		-30			11603	-34.5	82.15	2521	-12.6	17.85	14124	-31.4
			+20		17710	0.0	86.6	2615	-5.2	13.4	20325	-0.7
			-20		17710	0.0	85.2	3073	6.3	14.8	20783	+0.9
				+15	18236	+3.0	87.6	2591	-10.1	12.4	20826	+1.1
				-15	17068	-3.6	84.6	3098	+7.5	15.4	20166	-2.1
+30	+30	-30			21984	+24.1	87.1	3253	+12.8	12.9	25237	+22.6
-30	-30	+30			11243	-36.5	82.9	2326	-19.3	17.1	13569	-34.1

Analysis B: The results of the sensitivity analysis simulating suspended solids variations, reported in Table 7.6, indicate that noticeable increases in sludge production result from the application of high suspended solids concentration, and vice versa. The sludge production from the primary settling tank is as expected the most affected. Secondary sludge production variation revealed less significant, but the same general pattern was still observed.

Analysis C: The results of the analysis simulating flow variations, indicated sludge production to be sensitive to changes in the rates of flow. In fact, higher rates of flow resulted in an increased total sludge production which can be accounted for by higher applied suspended solids loadings. As expected, lower rates of flow resulted in a decrease in sludge production.

Analysis D: Variations in the recirculation ratios, as reported in Table 7.6, have a negligible influence upon the production of sludge from the treatment plant as a whole. A moderate decrease in secondary sludge production was observed at high recirculation ratios. However, because of the relatively greater importance of primary sludge production the overall effect was observed to be negligible.

Analysis E: Temperature variations also showed a negligible influence upon the total sludge production. The general trend observed, indicated that with a 10% increase in temperature, total sludge production increased by only 1.1%.

Analysis F: Dry and wet weather flow conditions results, reported in Table 7.6, indicate significant sludge production variations. Under dry weather flow conditions sludge production was observed to increase by over 20% while under wet weather flow conditions, sludge handling decreased by nearly 35%.

CHAPTER 8

TRICKLING FILTER EFFICIENCY COMPARISON

8.1 Introduction

The purpose of this chapter is to evaluate the actual BOD performance and efficiency of the Alexandria sewage treatment plant, as described by the mathematical models developed earlier and to compare such performance with other well-known trickling filter models.

Because of the characteristics and features of most mathematical models used in designing trickling filter installations (Table 2.1), the efficiency comparison has been performed by using the trickling filter-final settling tank subsystem. A wide range of operating conditions has been selected to examine the response of the models, thus revealing the overall predictive efficiency. Table 8.1 summarizes the average, maximum and minimum expected input values to the trickling filter-final settling tank subsystem. Even though the predictive performance equations for the Alexandria sewage treatment plant have established BOD removal to be dependent upon the effluent suspended solids loading from the unit, none of the reported design equations have ever considered such a dependency.

Table 8.2 illustrates the thirteen different levels, or input vectors, used to compare the performance of the treatment plant.

TABLE 8.1

Input Parameter Levels

Input Parameter	Minimum	Average	Maximum
BOD (mg/l)	87	117	152
SS (mg/l)	63	90	117
Flow (MGD)	15.67	24.25	31.01
Recirculation	1.05	1.31	1.57
Sewage Temp °F	56	67	78

Six mathematical models, selected from Table 2.1, have been used for comparison with the Alexandria sewage treatment plant. These are the equations of: i) National Research Council (U.S.), ii) Ten State Standards, iii) Velz, iv) Rankin, v) Eckenfelder, and vi) Galler and Gotaas. The analysis therefore has, for the main purpose, the identification of those models that represent most closely actual plant performance, under varied input situations.

8.2 Analysis and Discussion

The response of the various mathematical models to the thirteen input vectors of Table 8.2 has been summarized in Table 8.3. By using the mean of the absolute value of the difference as a measure of the goodness of fit for each mathematical model tested a better appreciation of the

TABLE 8.2

Trickling Filter-Final Settling Tank Input Vectors

	BOD	SS	Flow	Recirculation	Sewage T °F
1	117	90	24.25	1.31	67
2	87	90	24.25	1.31	67
3	152	90	24.25	1.31	67
4	117	63	24.25	1.31	67
5	117	117	24.25	1.31	67
6	117	90	15.67	1.31	67
7	117	90	31.01	1.31	67
8	117	90	24.25	1.05	67
9	117	90	24.25	1.57	67
10	117	90	24.25	1.31	56
11	117	90	24.25	1.31	78
12	152	117	15.67	1.31	67
13	87	63	31.01	1.31	67

TABLE 8.3

Response of the Selected Mathematical Models

BOD	SS	FLOW	R	T	A.S.A.		NRC		VELZ		RANKIN		ECKENFELDER		GALLER AND GOTAAS		TEN STATES STANDARDS*	
					EFF.	DIF.	EFF.	DIF.	EFF.	DIF.	EFF.	DIF.	EFF.	DIF.	EFF.	DIF.	EFF.	DIF.
A	A	A	A	A	57.3	70.2	12.9	76.9	19.6	72.4	14.6	61.1	3.8	63.2	5.9	-	-	-
+					60.6	67.4	6.8	76.9	16.3	72.4	11.8	61.1	0.5	61.3	0.7	-	-	-
-					53.3	73.2	19.9	76.9	23.6	72.4	19.1	61.1	7.8	65.2	11.9	-	-	-
+					52.2	70.2	18.0	76.9	24.7	72.4	20.2	61.1	8.9	63.2	11.0	-	-	-
-					63.5	70.2	6.7	76.9	13.4	72.4	8.9	61.1	2.4	63.2	0.3	-	-	-
	+				55.9	67.6	11.7	76.9	21.0	72.4	16.5	58.2	2.3	62.1	6.2	-	-	-
	-				59.8	74.6	14.8	76.9	17.1	72.4	12.6	66.2	6.4	65.2	5.4	-	-	-
		+			61.2	73.4	12.2	76.9	15.7	75.8	14.6	61.1	0.1	61.3	0.1	-	-	-
		-			52.2	66.0	13.8	76.9	24.7	67.7	15.5	61.1	8.9	65.4	13.2	-	-	-
			+		60.4	70.2	9.8	76.9	16.5	72.4	12.0	61.1	0.7	64.7	4.3	-	-	-
			-		53.4	70.2	16.8	76.9	23.5	72.4	19.0	61.1	7.7	61.0	7.6	-	-	-
+	+	-			58.3	72.0	13.7	76.9	18.6	72.4	14.1	66.2	7.9	60.3	2.0	-	-	-
-	-	+			58.7	70.8	12.1	76.9	18.2	72.4	13.7	58.2	0.5	65.8	7.1	-	-	-
Σ =						169.2		252.9		192.6		57.9		75.7				
MEAN =						13.02		19.46		14.82		4.45		5.82				

* BOD loading is beyond allowable range, indicating an efficiency less than 68% in all instances.

models will have been gained. These statistics have also been reported in Table 8.3.

Thus, the information presented in Table 8.3 indicates fairly clearly that the models of Eckenfelder and of Galler and Gotaas represent more closely actual plant performance. Even though the models disagree in some particular instances with predicted plant performance, the results reported in Table 8.3 indicate that the mean of the absolute value of the difference for Eckenfelder's model is 4.45% while that for Galler and Gotaas' model is observed to be 5.82%.

The models of Rankin, Velz and the NRC do not agree very well with the estimated plant performance. The lack of agreement between these models and reported plant performance comes about from the actual concepts underlying their development. In fact, BOD removal efficiencies have been established on only a few parameters, some of which do not very clearly influence the BOD removal process.

As far as the Ten State Standards model is concerned, the BOD loadings calculated from the selected input conditions fell beyond the Ten State Standard limit of 50 pounds per thousand cubic feet indicating only that BOD removal must be less than 68% in all instances.

This straightforward analysis has illustrated that even though the process is physically straightforward, the removal (biological and physical) mechanisms are not well

defined. It would therefore be impossible to retain one specific mathematical model, to represent under all circumstances the BOD removal efficiency of the trickling filter system. In fact, it can be stated with much certainty that the models, whatever they be, can only approximate the BOD removal process within the trickling filter system, and that no universal model can at this stage be suggested.

CHAPTER 9

SUMMARY, CONCLUSIONS, AND RECOMMENDATIONS FOR FUTURE RESEARCH

9.1 Summary

Computer simulation of a high rate trickling filter installation was performed through the mathematical model building of eight representative BOD and suspended solids removal efficiency equations, summarized in Table 6.12. Mathematical expressions for all major units of the treatment plant were developed for both BOD and suspended solids removal evaluation. The simulation process is based on seven years of operational data, collected from the Alexandria sewage treatment plant, Virginia, covering a wide range in both sewage characteristics and flow conditions. The relevance of this investigation was further motivated by the fact that the plant, designed in the early 50's will soon (1973) reach ultimate design flow conditions.

Various forms of mathematical models were evaluated, ranging from first and second order polynomials to exponential and multiplicative non linear models. After examining the fitness of the models to the field data, the intrinsically linear multiplicative model was finally adopted to represent the removal efficiencies, for both BOD and suspended solids, of the units. Both linear and non linear

regression algorithms were evaluated in regressing the available data. The multiple linear regression technique was observed to best fit the data, and was selected for all of the mathematical formulations.

Because most trickling filter mathematical models evaluate the performance of both the trickling filter unit along with the secondary settling tank, and because the specific performance of the trickling filter unit is also of interest in the present investigation, both arrangements have been considered and analyzed in this study. Two subsystem configurations therefore resulted, each having their own specific characteristics and importance.

A total of eight mathematical models resulted from the analysis of both configurations. Multiple correlation coefficients ranging from 0.9184 to 0.9779 resulted. Parameters were tested for significance and the results reported. Rejection levels for the testing of statistical hypothesis were selected based on the work of other researchers, and on the characteristics of the available data.

The arrangement in series of the mathematical models from configuration No. 2, represented in Figure 6.2, subjected to a convergence criterion in the trickling filter effluent characteristics, essentially describes the computer simulation algorithm referred to as CSTF, and used in the sensitivity analysis of both the treatment plant as a whole and of the treatment units individually. Input matrices

describing variations in BOD, suspended solids, flow, recirculation, sewage temperature, and weather flow conditions, were used to examine the stability of the treatment plant as a whole and of the treatment units individually.

Sewage characteristics sensitivity analysis was performed on the treatment plant as a whole, and on the treatment units individually, so as to delineate parameters of significant importance in the performance of the process.

The sludge production within the treatment plant was analyzed so as to determine operation levels that cause additional sludge handling problems.

Lastly, the predictive equations for the trickling filter-final settling tank subsystem were examined and compared with the most common mathematical models available to practicing engineers. The models of Eckenfelder, and Galler and Gotaas appeared to be the most representative mathematical models compared. The formulations of Velz, Rankin, and NRC being functions of less variables did not appear to be very sensitive to input vector variations.

9.2 Conclusions

The following conclusions can be drawn from the studies reported in this dissertation:

1. Simulation and mathematical model building, based on readily available physical variables of an existing trickling filter installation, yielded very satisfactory results. High multiple correlation coefficients, up to 0.9795 were obtained.

2. In all cases BOD removal was observed to be a function of the effluent suspended solids of the particular treatment unit, corroborating the work of other researchers in this field.

3. Primary tank BOD removal was observed to be a function of the recirculation ratio, reflecting the importance of the BOD composition (solid BOD and dissolved BOD) entering the primary tank.

4. Recirculation ratio was observed to benefit BOD and suspended solids removal efficiencies of the trickling filter unit, while shown to hinder primary tank performance as far as BOD removal was concerned. The overall effect on the plant was an increase in both BOD and suspended solids removal efficiencies occurred with corresponding increases in the recirculation ratio.

5. As expected, temperature of the sewage was observed to be significant in all but the secondary tank models. BOD and suspended solids concentrations were observed to decrease by 8 and 11.8% respectively with a 15% increase in temperature.

6. Surprisingly, the suspended solids removal in the primary tank was observed to be almost non-sensitive to

variations in the primary tank overflow rate parameter. The other treatment units appeared, however, to be sensitive to the rate of flow applied to them. In fact, increased flow rates were shown to benefit both BOD and suspended solids removal efficiencies within the trickling filter unit, and trickling filter system. Explanations to such a response were accounted for by a more continuous and regular sloughing mechanism, and also by prevailing turbulent flow regime under higher rates of flow.

7. Variations in the BOD loadings resulted in only moderate changes in the sewage characteristics of the plant effluent. In fact, higher BOD loadings accounted for an increase in both BOD and suspended solids concentration in the effluent.

Variations in the suspended solids loading, however, resulted in a more definite change in the sewage characteristics of the plant effluent. Corresponding to higher suspended solids concentrations were noticeable increases in both the effluent BOD and suspended solids concentrations.

8. Weather flow variations as dictated in the sensitivity analysis of both the sewage characteristics and sludge production were observed to have a significant influence upon the plant's performance.

9. The sludge production was observed to be approximately 10 to 15% secondary sludge with primary sludge forming

about 85 to 90% of the total. Sludge production variations were found to be sensitive to suspended solids variations, flow variations, and weather variations.

10. Simulation, through multiple regression of readily available field data is justified and provides both an understanding of the actual removal mechanisms within a complete treatment process, along with supplying the structure for possible computer assisted trickling filter plant operation.

11. Comparison of predicted plant performance with other well known trickling filter design formulations indicated that the models presented by Eckenfelder and Galler and Gotaas represented more closely actual plant performance.

12. Lastly, the study revealed that no single mathematical model exists for the performance evaluation of trickling filter plants.

9.3 Recommendations for Future Research

Three recommendations for future research are hereafter presented. The first consists in performing an economic analysis of the described treatment plant so as to establish the optimum BOD and suspended solids removal efficiencies so as to minimize both operational and maintenance costs.

The second proposition for future research originates from the fact that the plant designed in the

early 50's has almost reached its ultimate designed flow conditions. Thus, by assuming the applicability in parallel or in series of the reported mathematical models, possible expansion of the actual treatment plant facilities could be planned with more understanding.

Thirdly, it is suggested that this type of analysis be extended to double filtration so as to appreciate the possible difference in the performance, and therefore analyze the economics between single and double stage filtration.

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APPENDIX A

LISTING OF MASBAL PROGRAM

&

OPERATION DATA FROM OCTOBER 64 TO SEPTEMBER 71

11/16/17

DATE = 72272

MAIN

FORTRAN IV G LEVEL 20

```

0022      PI=3.14159
0023      RTF=80.
0024      DTF=4.25
0025      NOTF=2
0026      AREATF=NOTF*PI*(RTF**2)
0027      VOLTF=AREATF*DTF
C
C.....SECONDARY SETTLING TANKS
C
0028      NOSST=5
0029      LSST=125
0030      WSST=2.*18.
0031      DSST=10.
0032      AREAST=NOSST*LSST*WSST
0033      VOLST=AREAST*DSST
C
C.....DIGESTERS
C
0034      NODIG=2
0035      RDIG=(95./2.)
0036      ODIG=28.
0037      VOLDIG=NODIG*PI*(RDIG**2)*ODIG
C
C.....DEWATERING: TWO STAINLESS STEEL COIL SPRING
C
0038      NODEW=2
0039      DDEW=10.
0040      LDEW=10
0041      AREADW=2*PI*DDEW*0.25*LDEW
C
C.....DATA CHARACTERISTICS
0042      NOYEAR=7
0043      YEAR(8)=71
0044      NOMONT=12
C
C.....READ DATA
C
DO 100 I=1,NOYEAR
READ(I,1) YEAR(I)
DO 100 J=1,NOMONT
READ(1,2) MONTH(J),DAYS(I,J),TEMPHA(I,J),TEMPMI(I,J),PPTN(I,J),SUN

```

```

FORTRAN IV G LEVEL 20          MAIN          DATE = 72272          11/16/17

1P(I,J),FLOW2(I,J),BYFLOW(I,J),FLOW14(I,J),FLOW3(I,J),DT1(I,J),DT2(
2I,J),BOD2(I,J),BOD5(I,J),BOD7(I,J),BOD8(I,J),BOD14(I,J),SS2(I,J),S
4S5(I,J),SS7(I,J),SS8(I,J),SS14(I,J),FLOW6(I,J),SL1TSP(I,J),SL1VSP(
5I,J),FLOW9(I,J),SL2TSP(I,J),SL2VSP(I,J),FLOW12(I,J),DSL1TSP(I,J),DS
6LVSP(I,J),FLOW13(I,J),SL1TSP(I,J),SL2VSP(I,J)

C      NET FLOW      BOD      SS
C
C      FLOW12(I,J)=FLOW12(I,J)/(DAYS(I,J)*1000,)
0049      FLOW14(I,J)=FLOW14(I,J)/DAYS(I,J)
0050      FLOW1(I,J)=FLOW2(I,J)-FLOW14(I,J)
0051      BOD1(I,J)=(BOD2(I,J)*FLOW2(I,J)-BOD14(I,J)*FLOW14(I,J))/FLOW1(I,J)
0052      SS1(I,J)=(SS2(I,J)*FLOW2(I,J)-SS14(I,J)*FLOW14(I,J))/FLOW1(I,J)
0053

C      FLOW      BOD      SS      ENTERING PRIMARY TANKS
C
C      FLOW4(I,J)=FLOW3(I,J)+FLOW2(I,J)
0054      BOD3(I,J)=BOD7(I,J)
0055      SS3(I,J)=SS7(I,J)
0056      BOD4(I,J)=(BOD3(I,J)*FLOW3(I,J)+BOD2(I,J)*FLOW2(I,J))/FLOW4(I,J)
0057      SS4(I,J)=(SS3(I,J)*FLOW3(I,J)+SS2(I,J)*FLOW2(I,J))/FLOW4(I,J)
0058

C      PRIMARY SLUDGE TO DIGESTER
C
C      FLOW6(I,J)=FLOW6(I,J)/(DAYS(I,J)*1000)
0059

C      FLOW ENTERING TRICKLING FILTER
C
C      FLOW5(I,J)=FLOW4(I,J)-FLOW6(I,J)
0060

C      FLOW ENTERING SECONDARY SETTLER
C
C      FLOW7(I,J)=FLOW5(I,J)-FLOW3(I,J)
0061

C      SECONDARY SLUDGE TO DIGESTER
C
C      FLOW9(I,J)=FLOW9(I,J)/(DAYS(I,J)*1000)
0062

C      FLOW LEAVING SECONDARY SETTLER
C
C      FLOW8(I,J)=FLOW7(I,J)-FLOW9(I,J)
0063

C      WATER FOR DEWATERING, BOD      SS      FLOW
C

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11/16/17

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FORTRAN IV G LEVEL 20                                MAIN          DATE = 72272
C
0064      FLOW13(I,J)=FLOW13(I,J)/(DAYS(I,J)*1000)
0065      FLOW11(I,J)=FLOW14(I,J)-FLOW13(I,J)
0066      BOD11(I,J)=BOD8(I,J)
0067      SS11(I,J)=SS8(I,J)
C
C      TRICKLING FILTER
C
0068      RATIO(I,J)=FLOW3(I,J)/FLOW2(I,J)
0069      TFL01(I,J)=(FLOW5(I,J)*(10**6))/AREATF
0070      TFL02(I,J)=(BOD5(I,J)*FLOW5(I,J)*8.34)/(VOLT/1000.)
C
C      BOD REMOVALS
C
C      GROSS
0071      BOD10(I,J)=BOD8(I,J)
0072      FLOW10(I,J)=FLOW7(I,J)-FLOW9(I,J)-FLOW11(I,J)
0073      WBOD2(I,J)=(FLOW2(I,J)*WBOD2(I,J)*DAYS(I,J)*8.34)
0074      WBOD10(I,J)=(FLOW10(I,J)*WBOD10(I,J)*DAYS(I,J)*8.34)
0075      WBODDR(I,J)=WBOD2(I,J)-WBOD10(I,J)
0076      RBODRP(I,J)=(WBODDR(I,J)/WBOD2(I,J))*100.
C      NET
0077      WBOD14(I,J)=(FLOW14(I,J)*WBOD14(I,J)*DAYS(I,J)*8.34)
0078      WBOD1(I,J)=WBOD2(I,J)-WBOD14(I,J)
0079      WNBODR(I,J)=WBOD1(I,J)-WBOD10(I,J)
0080      NBODRP(I,J)=(WNBODR(I,J)/WBOD1(I,J))*100
C
C      SS REMOVAL
C
C      GROSS
0081      SS10(I,J)=SS8(I,J)
0082      WSS2(I,J)=(FLOW2(I,J)*SS2(I,J)*DAYS(I,J)*8.34)
0083      WSS10(I,J)=(FLOW10(I,J)*SS10(I,J)*DAYS(I,J)*8.34)
0084      WSSSR(I,J)=WSS2(I,J)-WSS10(I,J)
0085      GSSRP(I,J)=(WSSSR(I,J)/WSS2(I,J))*100.
C      NET
0086      WSS14(I,J)=(FLOW14(I,J)*SS14(I,J)*DAYS(I,J)*8.34)
0087      WSS1(I,J)=WSS2(I,J)-WSS14(I,J)
0088      WNSSR(I,J)=WSS1(I,J)-WSS10(I,J)
0089      NSSRP(I,J)=(WNSSR(I,J)/WSS1(I,J))*100.
C
C      DIGESTER LOADING

```

11/16/17

DATE = 72272

FORTAN IV G LEVEL 20

MAIN

C

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0090 WTS1=(SL1TSP(I,J)*FLOW6(I,J)*8.34*HOURS(I,J)*1000.)
0091 WVS1=SL1VSP(I,J)*WTS1
0092 WTS2=SL2TSP(I,J)*FLOW9(I,J)*8.34*HOURS(I,J)*1000.
0093 WVS2=SL2VSP(I,J)*WTS2

```

C

LOADING OF DEWATERING UNIT

C

```

0094 WTS1=FLOW12(I,J)*DSL1TSP(I,J)*8.34
0095 WVS1=DSL1VSP(I,J)*WTS1
0096 WTS2=FLOW13(I,J)*SL1TSP(I,J)
0097 WVS2=SL1VSP(I,J)*WTS2
0098 DEWTS(I,J)=WTS1+WTS2
0099 DEWVS(I,J)=WVS1+WVS2
0100 ATEMP(I,J)=(TEMPMA(I,J)+TEMPMI(I,J))/2
0101 BOD5F(I,J)=BOD5(I,J)
0102 SSS(I,J)=SS3(I,J)
0103 FLOWSE(I,J)=FLOW5(I,J)

```

100 CONTINUE

```

0104 DO 654 I=1,NOYEAR
0105 DO 654 J=1,NOMONT
0106 IF(I.EQ.1.AND.J.EQ.1) TEMPA(1)=ATEMP(1,1)
0107 IF(I.EQ.1.AND.J.EQ.1) GO TO 654
0108 IF(I.EQ.1) K=J
0109 IF(I.GT.1) K=(I-1)*NOMONT +J
0110 TEMPA(K)=ATEMP(I,J)
0111 STEMP(I,J)=41.96217+0.18005*TEMPA(K)+0.26993*TEMPA(K-1)

```

654 CONTINUE

```

0112 1 FORMAT(I2)
0113 2 FORMAT(A8,2X,3I2,F4,2,I2,F5,2,F6,2,4F4,2,3I3,I2,I4,2I3,2I2,I5,/,F7
0114 1,2,F4,2,F5,2,F7,2,F4,2,F5,2,2(F7,2,F4,2,F5,2))
0115 JJ=NOYEAR-1

```

```

0116 DO 110 I=1,NOYEAR
0117 K=I+1
0118 WRITE(3,220) YEAR(I),YEAR(K)
0119 WRITE(1H1,58X,'TABLE NO. 1',/,49X,'FROM OCTOBER 1,12,1 TO SEPTEMB
0120 1ER 1,12)

```

```

220 FORMAT(1H1,58X,'TABLE NO. 1',/,49X,'FROM OCTOBER 1,12,1 TO SEPTEMB
1ER 1,12)

```

```

0121 WRITE(3,221)
0122 221 FORMAT(3X,'MONTH',3X,'DAYS',7X,'STATION # 1',12X,'STATION # 2',12X
1,'STATION # 3',12X,'STATION # 4',12X,'STATION # 5',/,17X,5('FLOW',
23X,'B.O.D.',4X,'S.S.',12X),/,17X,5('MGD',4X,'MG/L',6X,'MG/L',2X))

```

```

0123 DO 111 J=1,NOMONT
0124 111 WRITE(3,222) MONTH(J),DAYS(I,J),FLOW1(I,J),BOD1(I,J),SS1(I,J),FLOW

```



```

FORTRAN IV G LEVEL 20
                                MAIN                                DATE = 72272                                11/16/17

S'GROSS',6X,'X',5X,'NET',7X,'X',4X,'DIG,SLUDGE',7X,'AVG,TEMP',1,2X,'
SAVG,TEMP',1,/,20X,'WT. REM.',8X,'WT. REM.',8X,'WT. REM
SM.',8X,'TOTAL VOLATILE',7X,'AIR',6X,'SEWAGE',/,21X,'POUNDS',10X,'POUNDS',1
SOUNDS',10X,'POUNDS',10X,'POUNDS')
DO 114 J=1,NOMONT
WRITE(3,231) MONTH(J),DAYS(I,J),WG80DR(I,J),RBODRP(I,J),WNBODRP(I,J)
S),NBODRP(I,J),WGSSR(I,J),GSSRP(I,J),WNSSR(I,J),NSSRP(I,J),DSLTP(I,J),
S,J),DSLVP(I,J),ATEMP(I,J),STEMP(I,J)
231 FORMAT(1X,A8,5X,I2,4X,F8,0,2X,F4,1,2X,F8,0,2X,F4,1,2X,F8,0,2X,F4,1,
S,2X,F8,0,2X,F4,1,2X,F4,2,1X,2X,F5,2,1X,7X,I2,1 F,6X,F4,1,1 F)
114 CONTINUE
110 CONTINUE
110 RETURN
END
0147
0148
0149
0150
0151
0152
0153

```

TABLE NO. 1
FROM OCTOBER 64 TO SEPTEMBER 65

MONTH	DAYS	STATION # 1				STATION # 2				STATION # 3				STATION # 4				STATION # 5			
		FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L		
OCTOBER	31	9.60	213	255	9.75	213	260	5.95	72	75	15.70	159	189	15.64	105	89					
NOVEMBER	30	10.65	224	254	10.82	226	266	7.37	95	80	18.19	172	190	18.13	117	97					
DECEMBER	31	10.81	223	253	10.91	224	263	7.45	82	84	18.36	166	190	18.29	104	98					
JANUARY	31	14.34	211	219	14.45	212	226	5.75	97	80	20.20	179	184	20.15	113	90					
FEBRUARY	28	15.17	217	231	15.28	220	247	5.39	100	84	20.67	188	204	20.61	111	94					
MARCH	31	18.39	178	195	18.51	180	208	5.64	89	75	22.15	165	186	22.09	105	94					
APRIL	30	15.91	210	212	16.01	214	246	4.77	89	79	20.78	185	207	20.72	111	95					
MAY	31	13.59	199	220	13.70	201	242	6.45	95	79	20.15	167	189	20.09	112	92					
JUNE	30	13.04	189	198	13.17	191	209	7.49	68	59	20.66	146	154	20.61	103	81					
JULY	31	11.80	191	190	11.90	192	196	7.98	63	56	19.88	140	139	19.82	100	81					
AUGUST	31	11.48	178	194	11.57	179	198	7.98	56	54	19.55	128	139	19.49	94	78					
SEPTEMBER	30	11.82	183	219	11.94	186	260	8.23	60	61	20.17	134	178	20.10	90	88					

TABLE NO. 2
FROM OCTOBER 64 TO SEPTEMBER 65

MONTH	DAYS	STATION # 6				STATION # 7				STATION # 8				STATION # 9				STATION #10		STATION #11	
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	S.9. MG/L	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	FLOW MGD	FLOW MGD	FLOW MGD			
OCTOBER	31	0.06	3.72	81.27	9.69	72	75	9.68	47	43	0.01	2.20	72.07	9.58							
NOVEMBER	30	0.06	3.70	82.62	10.76	95	80	10.75	51	53	0.01	2.18	77.30	10.64							
DECEMBER	31	0.07	3.72	79.56	10.84	82	84	10.84	50	50	0.01	2.42	73.06	10.77							
JANUARY	31	0.05	3.75	83.01	14.40	97	80	14.39	52	49	0.01	2.82	75.47	14.30							
FEBRUARY	28	0.06	4.08	81.16	15.22	100	84	15.20	50	48	0.02	2.99	74.33	15.12							
MARCH	31	0.06	3.99	78.84	18.45	89	75	18.43	47	51	0.02	2.91	72.23	18.35							
APRIL	30	0.06	4.14	78.05	15.95	89	79	15.93	47	51	0.02	2.85	73.12	15.89							
MAY	31	0.06	4.19	77.66	13.64	95	79	13.62	45	37	0.02	2.57	74.67	13.57							
JUNE	30	0.05	3.78	77.83	13.12	68	59	13.10	37	37	0.02	2.53	74.83	13.05							
JULY	31	0.06	3.42	77.77	11.84	63	56	11.82	36	36	0.02	2.64	73.55	11.79							
AUGUST	31	0.06	4.23	75.94	11.51	56	54	11.49	32	35	0.02	2.37	74.29	11.45							
SEPTEMBER	30	0.07	4.25	76.18	11.87	60	61	11.85	34	39	0.02	2.64	69.86	11.80							

TABLE NO. 1
FROM OCTOBER 65 TO SEPTEMBER 66

MONTH	DAYS	STATION # 1				STATION # 2				STATION # 3				STATION # 4				STATION # 5			
		FLOW	B.O.D.	S.S.	MG/L	FLOW	B.O.D.	S.S.	MG/L	FLOW	B.O.D.	S.S.	MG/L	FLOW	B.O.D.	S.S.	MG/L	FLOW	B.O.D.	S.S.	MG/L
OCTOBER	31	12.39	189	219	12.49	190	190	226	8.20	60	60	57	20.69	138	138	159	20.63	100	100	159	20.63
NOVEMBER	30	11.85	210	250	11.95	216	216	281	7.50	70	70	70	19.45	159	159	199	19.38	114	114	199	19.38
DECEMBER	31	12.21	272	323	12.34	288	288	342	5.70	103	103	91	18.04	229	229	262	17.98	146	146	262	17.98
JANUARY	31	12.72	222	271	12.82	233	233	287	6.70	77	77	78	19.52	179	179	215	19.46	110	110	215	19.46
FEBRUARY	28	16.61	177	188	16.90	184	184	203	4.39	89	89	87	21.29	164	164	179	21.23	111	111	179	21.23
MARCH	31	15.72	184	201	15.82	193	193	210	5.25	95	95	81	21.07	168	168	177	21.01	118	118	177	21.01
APRIL	30	16.76	162	177	16.89	171	171	205	3.47	80	80	77	20.36	155	155	183	20.30	105	105	183	20.30
MAY	31	17.79	172	177	17.93	176	176	197	4.44	73	73	58	22.37	155	155	169	22.31	95	95	169	22.31
JUNE	30	15.50	179	194	15.63	186	186	252	7.42	60	60	59	23.05	145	145	189	22.98	95	95	189	22.98
JULY	31	14.01	168	218	14.13	172	172	234	8.20	55	55	60	22.36	129	129	173	22.26	106	106	173	22.26
AUGUST	31	13.61	185	219	13.74	189	189	260	8.62	57	57	55	22.36	138	138	180	22.28	106	106	180	22.28
SEPTEMBER	30	16.72	167	266	16.84	173	173	318	6.29	57	57	72	23.13	141	141	251	23.04	104	104	251	23.04

TABLE NO. 2
FROM OCTOBER 65 TO SEPTEMBER 66

MONTH	DAYS	STATION # 6				STATION # 7				STATION # 8				STATION # 9				STATION # 10				STATION # 11			
		FLOW	TOTAL	SOLIDS	VOL.	FLOW	B.O.D.	S.S.	MG/L	FLOW	B.O.D.	S.S.	MG/L	FLOW	TOTAL	SOLIDS	VOL.	FLOW	B.O.D.	S.S.	MG/L	FLOW	B.O.D.	S.S.	MG/L
OCTOBER	31	0.06	3.68	78.19	78.19	12.43	60	57	12.41	37	37	41	0.02	2.59	2.59	70.11	70.11	12.37	12.37	12.37	12.37	0.04	0.04	0.04	0.04
NOVEMBER	30	0.07	4.12	80.73	80.73	11.88	70	70	11.87	47	47	55	0.02	2.53	2.53	74.80	74.80	11.82	11.82	11.82	11.82	0.04	0.04	0.04	0.04
DECEMBER	31	0.06	4.02	82.82	82.82	12.28	103	91	12.26	61	61	62	0.02	2.56	2.56	76.32	76.32	12.20	12.20	12.20	12.20	0.06	0.06	0.06	0.06
JANUARY	31	0.06	3.98	81.92	81.92	12.76	77	78	12.74	49	49	57	0.02	3.15	3.15	74.81	74.81	12.68	12.68	12.68	12.68	0.06	0.06	0.06	0.06
FEBRUARY	28	0.06	3.91	80.58	80.58	16.84	89	87	16.82	56	56	64	0.02	3.06	3.06	73.86	73.86	16.77	16.77	16.77	16.77	0.05	0.05	0.05	0.05
MARCH	31	0.06	3.84	79.53	79.53	15.76	95	81	15.74	62	62	63	0.02	2.86	2.86	76.98	76.98	15.69	15.69	15.69	15.69	0.08	0.08	0.08	0.08
APRIL	30	0.06	4.05	78.70	78.70	16.83	80	77	16.81	50	50	56	0.02	2.96	2.96	72.90	72.90	16.73	16.73	16.73	16.73	0.07	0.07	0.07	0.07
MAY	31	0.06	4.12	79.43	79.43	17.87	73	58	17.85	44	44	46	0.02	2.93	2.93	69.04	69.04	15.47	15.47	15.47	15.47	0.06	0.06	0.06	0.06
JUNE	30	0.07	4.80	73.28	73.28	15.56	60	59	15.54	39	39	37	0.02	2.48	2.48	70.64	70.64	14.00	14.00	14.00	14.00	0.05	0.05	0.05	0.05
JULY	31	0.07	4.20	75.55	75.55	14.06	55	60	14.04	35	35	41	0.02	2.58	2.58	70.11	70.11	13.58	13.58	13.58	13.58	0.06	0.06	0.06	0.06
AUGUST	31	0.08	4.72	73.62	73.62	13.66	57	55	13.64	36	36	50	0.02	3.12	3.12	63.52	63.52	16.68	16.68	16.68	16.68	0.05	0.05	0.05	0.05
SEPTEMBER	30	0.09	5.85	68.40	68.40	16.75	57	72	16.73	36	36	50	0.02	3.12	3.12	63.52	63.52	16.68	16.68	16.68	16.68	0.05	0.05	0.05	0.05

TABLE NO. 3									
FROM OCTOBER 65 TO SEPTEMBER 66									
MONTH	DAYS	STATION #12			STATION #13			STATION #14	
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L
OCTOBER	31	0.01	6.13	51.49	0.05	0.68	57.09	0.10	310
NOVEMBER	30	0.00	5.48	53.25	0.06	1.31	65.10	0.10	830
DECEMBER	31	0.00	6.27	61.38	0.07	2.22	67.33	0.13	1810
JANUARY	31	0.01	5.42	64.34	0.04	1.05	69.55	0.10	1570
FEBRUARY	28	0.01	5.81	60.77	0.04	0.87	78.07	0.09	1310
MARCH	31	0.01	5.62	63.25	0.05	1.55	68.37	0.10	1550
APRIL	30	0.04	3.07	57.68	0.05	1.28	62.41	0.13	1240
MAY	31	0.01	5.34	54.51	0.07	1.20	61.51	0.14	570
JUNE	30	0.01	5.09	54.36	0.07	1.75	57.73	0.13	890
JULY	31	0.01	6.47	45.30	0.07	0.96	56.04	0.12	620
AUGUST	31	0.01	5.81	51.11	0.07	1.34	56.02	0.13	550
SEPTEMBER	30	0.00	5.80	57.05	0.07	3.46	55.49	0.12	970

TRICKLING FILTER LOADING
B.O.D./
1000FT³/DAY

GAL/FT²
PER DAY

S.S.
MG/L

342.01 67.11
321.34 71.89
298.11 85.41
322.65 69.65
351.94 76.66
348.32 80.66
336.51 69.34
369.85 74.03
380.90 71.01
369.06 68.80
369.31 76.82
382.01 77.96

TABLE NO. 4															
FROM OCTOBER 65 TO SEPTEMBER 66															
MONTH	DAYS	B.O.D.		REMOVAL		GROSS		S.S.		WT. REM. POUNDS	WT. REM. POUNDS	DIG. SLUDGE TOTAL VOLATILE	MISCELLANEOUS		AVG. TEMP. SEWAGE
		GROSS WT. REM. POUNDS	%	NET POUNDS	%	WT. REM. POUNDS	%	AVG. TEMP. AIR	%						
OCTOBER	31	495236.	80.7	487454.	80.5	598696.	82.0	571835.	81.4	6.13%	51.49%	54 F	71.1 F		
NOVEMBER	30	506785.	78.5	485049.	77.7	677463.	80.6	579783.	78.1	5.48%	53.25%	46 F	64.8 F		
DECEMBER	31	726355.	79.1	666577.	77.6	895479.	82.1	825794.	80.8	6.27%	61.38%	39 F	61.4 F		
JANUARY	31	611589.	79.2	571391.	78.1	764337.	80.4	706728.	79.1	5.42%	64.34%	31 F	58.1 F		
FEBRUARY	28	506829.	69.8	479406.	68.6	550480.	68.7	490402.	66.2	5.81%	60.77%	36 F	56.8 F		
MARCH	31	537823.	68.1	498266.	66.5	603297.	70.2	562209.	68.7	5.62%	63.25%	45 F	59.8 F		
APRIL	30	513361.	71.0	472305.	69.3	631929.	72.9	508429.	68.4	3.07%	57.68%	51 F	63.3 F		
MAY	31	613619.	75.2	593321.	74.6	729354.	79.9	634271.	77.5	5.34%	54.51%	63 F	67.1 F		
JUNE	30	576415.	79.2	546427.	78.4	807420.	81.9	575271.	76.4	5.09%	54.36%	76 F	72.7 F		
JULY	31	501692.	79.8	483129.	79.2	759217.	84.7	656581.	83.1	6.47%	45.30%	81 F	77.1 F		
AUGUST	31	548517.	81.7	530123.	81.2	779669.	84.4	629508.	81.4	5.81%	51.11%	78 F	77.9 F		
SEPTEMBER	30	578671.	79.4	549063.	78.5	1131181.	84.4	907438.	81.3	5.80%	57.05%	67 F	75.1 F		

TABLE NO. 1
FROM OCTOBER 66 TO SEPTEMBER 67

MONTH	DAYS	STATION # 1				STATION # 2				STATION # 3				STATION # 4				STATION # 5			
		FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L		
OCTOBER	31	18.04	147	177	18.13	149	194	5.92	52	50	24.05	125	158	23.97	89	158	23.97	89	75		
	30	15.48	180	232	15.58	182	246	6.02	66	62	21.60	149	194	21.52	111	194	21.52	111	87		
	31	17.78	195	199	17.85	197	204	4.98	70	63	22.83	169	173	22.75	119	173	22.75	119	90		
NOVEMBER	31	18.89	177	200	18.95	179	207	5.34	67	65	24.29	154	175	24.21	114	175	24.21	114	89		
DECEMBER	31	20.83	162	163	20.89	165	174	4.78	77	65	25.67	148	153	25.59	116	153	25.59	116	87		
JANUARY	28	22.33	178	183	22.41	181	193	3.28	78	64	25.69	167	176	25.61	122	176	25.61	122	85		
FEBRUARY	31	17.51	175	198	17.38	180	211	7.86	70	64	25.24	145	165	25.16	115	165	25.16	115	84		
MARCH	30	19.30	161	163	19.40	170	179	6.36	69	58	25.76	145	149	25.68	118	149	25.68	118	81		
APRIL	31	17.67	202	192	17.78	223	251	8.29	91	69	26.07	181	193	25.98	136	193	25.98	136	90		
MAY	30	17.70	231	212	17.76	250	251	8.94	79	60	26.70	192	187	26.62	124	187	26.62	124	87		
JUNE	31	20.58	244	211	20.69	273	346	6.70	91	69	27.39	228	278	27.29	139	278	27.29	139	101		
JULY	31	22.85	213	244	22.96	233	295	5.18	84	62	28.14	205	252	28.05	141	252	28.05	141	97		
AUGUST	30																				
SEPTEMBER	30																				

TABLE NO. 2
FROM OCTOBER 66 TO SEPTEMBER 67

MONTH	DAYS	STATION # 6				STATION # 7				STATION # 8				STATION # 9				STATION #10				STATION #11												
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L						
OCTOBER	31	0.08	4.09	79.42	18.05	52	50	18.03	34	39	0.02	2.84	69.04	18.00	2.84	69.04	18.00	39	34	39	0.02	2.84	69.04	18.00	2.84	69.04	18.00	39	34	39	0.02	2.84	69.04	18.00
NOVEMBER	30	0.08	3.95	82.44	15.50	66	62	15.48	40	45	0.02	2.29	75.80	15.47	2.29	75.80	15.47	45	40	45	0.02	2.29	75.80	15.47	2.29	75.80	15.47	45	40	45	0.02	2.29	75.80	15.47
DECEMBER	31	0.08	3.52	85.32	17.77	70	63	17.75	47	45	0.02	2.49	77.32	17.75	2.49	77.32	17.75	45	47	45	0.02	2.49	77.32	17.75	2.49	77.32	17.75	45	47	45	0.02	2.49	77.32	17.75
JANUARY	31	0.08	4.00	84.74	18.87	67	65	18.86	46	46	0.02	2.51	79.29	18.85	2.51	79.29	18.85	46	46	46	0.02	2.51	79.29	18.85	2.51	79.29	18.85	46	46	46	0.02	2.51	79.29	18.85
FEBRUARY	28	0.08	3.64	85.49	20.81	77	65	20.79	49	44	0.02	2.78	78.62	20.79	2.78	78.62	20.79	44	49	44	0.02	2.78	78.62	20.79	2.78	78.62	20.79	44	49	44	0.02	2.78	78.62	20.79
MARCH	31	0.08	3.79	82.91	22.33	78	64	22.31	52	51	0.02	2.95	78.27	22.30	2.95	78.27	22.30	51	52	51	0.02	2.95	78.27	22.30	2.95	78.27	22.30	51	52	51	0.02	2.95	78.27	22.30
APRIL	30	0.08	3.67	83.46	17.30	70	64	17.28	49	49	0.02	2.50	79.05	17.28	2.50	79.05	17.28	49	49	49	0.02	2.50	79.05	17.28	2.50	79.05	17.28	49	49	49	0.02	2.50	79.05	17.28
MAY	31	0.08	3.83	82.57	19.32	69	58	19.30	46	44	0.02	2.43	75.24	19.26	2.43	75.24	19.26	44	46	44	0.02	2.43	75.24	19.26	2.43	75.24	19.26	44	46	44	0.02	2.43	75.24	19.26
JUNE	30	0.09	4.32	82.03	17.69	91	69	17.67	55	51	0.02	2.85	72.90	17.57	2.85	72.90	17.57	51	55	51	0.02	2.85	72.90	17.57	2.85	72.90	17.57	51	55	51	0.02	2.85	72.90	17.57
JULY	31	0.08	4.21	78.36	17.68	79	60	17.66	43	40	0.02	2.69	68.40	17.60	2.69	68.40	17.60	40	43	40	0.02	2.69	68.40	17.60	2.69	68.40	17.60	40	43	40	0.02	2.69	68.40	17.60
AUGUST	31	0.10	5.25	74.42	20.59	91	69	20.56	45	51	0.03	3.72	65.92	20.45	3.72	65.92	20.45	51	45	51	0.03	3.72	65.92	20.45	3.72	65.92	20.45	51	45	51	0.03	3.72	65.92	20.45
SEPTEMBER	30	0.09	4.53	81.07	22.87	84	62	22.85	55	46	0.02	2.75	74.98	22.77	2.75	74.98	22.77	46	55	46	0.02	2.75	74.98	22.77	2.75	74.98	22.77	46	55	46	0.02	2.75	74.98	22.77

TABLE NO. 3											
FROM OCTOBER 66 TO SEPTEMBER 67											
MONTH	DAYS	STATION #12			STATION #13			STATION #14		S.S. MG/L	TRICKLING FILTER LOADING # 8.0.D./ 1000FT3/DAY
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L		
OCTOBER	31	0.02	5.59	49.57	0.06	0.51	56.66	0.09	470	3280	69.41
NOVEMBER	30	0.01	5.58	51.10	0.09	0.71	58.82	0.10	370	2280	77.72
DECEMBER	31	0.01	6.04	53.15	0.07	0.57	62.19	0.07	580	1280	68.09
JANUARY	31	0.01	5.30	57.57	0.05	0.57	62.19	0.06	602	2100	89.81
FEBRUARY	28	0.02	5.90	61.61	0.05	0.72	64.83	0.06	1035	3580	96.59
MARCH	31	0.01	5.22	57.86	0.08	0.65	66.98	0.08	978	2824	101.64
APRIL	30	0.01	5.83	59.07	0.07	1.10	68.43	0.07	1160	3310	94.14
MAY	31	0.01	5.08	65.55	0.07	1.11	69.18	0.10	1780	417.15	98.57
JUNE	30	0.02	3.88	72.47	0.01	0.91	60.83	0.11	3480	425.69	114.94
JULY	31	0.00	0.00	0.00	0.00	0.00	0.00	0.06	6125	9750	107.38
AUGUST	31	0.00	9.62	57.26	0.00	0.00	0.00	0.11	5490	12400	123.41
SEPTEMBER	30	0.03	5.96	72.18	0.02	1.14	72.23	0.11	4370	452.42	128.69
										465.08	11000

TABLE NO. 1
FROM OCTOBER 67 TO SEPTEMBER 68

MONTH	DAYS	STATION # 1				STATION # 2				STATION # 3				STATION # 4				STATION # 5			
		FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L		
OCTOBER	31	18.95	219	231	19.07	240	313	6.72	107	96	25.79	205	256	25.70	155	131	25.70	155	131		
NOVEMBER	30	17.49	266	285	17.60	288	348	6.46	112	87	24.06	240	277	23.97	164	108	23.97	164	108		
DECEMBER	31	22.63	190	178	22.75	206	222	4.88	97	85	27.63	186	197	27.53	133	101	27.53	133	101		
JANUARY	31	22.49	189	217	22.55	200	239	4.71	91	75	27.26	181	210	27.16	128	96	27.16	128	96		
FEBRUARY	29	19.66	174	161	19.72	182	185	6.46	91	76	26.18	159	158	26.08	124	100	26.08	124	100		
MARCH	31	24.14	200	168	24.23	208	185	3.37	106	72	27.60	195	171	27.50	133	86	27.50	133	86		
APRIL	30	20.59	155	157	20.67	162	161	4.63	71	57	25.30	145	141	25.21	114	81	25.21	114	81		
MAY	31	20.63	155	158	20.73	169	168	5.93	82	60	26.66	149	143	26.58	104	82	26.58	104	82		
JUNE	30	23.23	204	158	23.43	243	265	4.21	106	92	27.64	222	238	27.52	149	105	27.52	149	105		
JULY	31	19.86	203	112	20.01	221	229	6.24	89	75	26.25	189	192	26.15	125	90	26.15	125	90		
AUGUST	31	18.94	181	180	19.05	184	192	7.04	67	57	26.09	152	155	26.00	114	84	26.00	114	84		
SEPTEMBER	30	18.86	193	173	18.98	196	192	7.34	69	57	26.32	160	154	26.23	121	88	26.23	121	88		

TABLE NO. 2
FROM OCTOBER 67 TO SEPTEMBER 68

MONTH	DAYS	STATION # 6				STATION # 7				STATION # 8				STATION # 9				STATION # 10				STATION # 11			
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	FLOW MGD	FLOW MGD	FLOW MGD	FLOW MGD	FLOW MGD	FLOW MGD	FLOW MGD	FLOW MGD
OCTOBER	31	0.09	5.84	74.07	18.98	107	96	10.96	63	76	0.02	3.08	71.05	0.02	3.08	71.05	0.02	18.85	18.85	18.85	18.85	0.11	0.11	0.11	0.11
NOVEMBER	30	0.09	4.23	79.90	17.51	112	87	17.49	66	65	0.02	2.71	74.61	0.02	2.71	74.61	0.02	17.41	17.41	17.41	17.41	0.07	0.07	0.07	0.07
DECEMBER	31	0.10	4.84	79.85	22.65	97	85	22.62	57	70	0.03	2.71	73.22	0.03	2.71	73.22	0.03	22.56	22.56	22.56	22.56	0.06	0.06	0.06	0.06
JANUARY	31	0.10	4.10	83.61	22.45	91	75	22.43	54	61	0.02	3.00	78.41	0.02	3.00	78.41	0.02	22.39	22.39	22.39	22.39	0.04	0.04	0.04	0.04
FEBRUARY	29	0.10	3.84	83.46	19.62	91	76	19.60	53	61	0.02	3.08	76.64	0.02	3.08	76.64	0.02	19.57	19.57	19.57	19.57	0.03	0.03	0.03	0.03
MARCH	31	0.10	4.01	82.33	24.13	106	72	24.11	58	58	0.02	3.10	78.07	0.02	3.10	78.07	0.02	24.02	24.02	24.02	24.02	0.09	0.09	0.09	0.09
APRIL	30	0.09	3.39	84.95	20.58	71	57	20.56	41	42	0.02	2.30	80.82	0.02	2.30	80.82	0.02	20.48	20.48	20.48	20.48	0.08	0.08	0.08	0.08
MAY	31	0.08	3.81	76.24	20.65	82	60	20.62	53	49	0.03	2.52	71.32	0.03	2.52	71.32	0.03	20.65	20.65	20.65	20.65	-0.03	-0.03	-0.03	-0.03
JUNE	30	0.12	4.43	71.66	23.31	106	92	23.28	61	68	0.03	2.85	67.62	0.03	2.85	67.62	0.03	23.25	23.25	23.25	23.25	0.03	0.03	0.03	0.03
JULY	31	0.10	5.08	73.56	19.91	89	75	19.89	50	58	0.03	2.82	70.46	0.03	2.82	70.46	0.03	19.83	19.83	19.83	19.83	0.05	0.05	0.05	0.05
AUGUST	31	0.09	3.71	81.95	18.96	67	57	18.94	39	41	0.02	1.96	77.33	0.02	1.96	77.33	0.02	18.83	18.83	18.83	18.83	0.11	0.11	0.11	0.11
SEPTEMBER	30	0.09	4.18	76.84	18.89	69	57	18.86	53	41	0.02	2.53	72.82	0.02	2.53	72.82	0.02	18.88	18.88	18.88	18.88	-0.02	-0.02	-0.02	-0.02

MONTH	DAYS	TABLE NO. 3				FROM OCTOBER 67 TO SEPTEMBER 68				TRICKLING FILTER LOADING			
		STATION #12		STATION #13		STATION #14		STATION #15		STATION #16		STATION #17	
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	GAL/FT2 PER DAY	# B.O.D./ 1000FT3/DAY	
OCTOBER	31	0.09	5.63	65.35	0.02	2.59	63.31	0.12	3460	12890	426.12	129.61	
NOVEMBER	30	0.06	4.51	63.63	0.03	2.17	66.33	0.11	3780	10580	397.35	127.88	
DECEMBER	31	0.03	4.46	73.78	0.05	2.24	63.72	0.12	3165	8650	456.37	119.11	
JANUARY	31	0.04	5.89	72.21	0.02	3.21	61.39	0.06	3850	7900	450.35	113.12	
FEBRUARY	29	0.03	6.10	62.40	0.04	1.01	69.87	0.06	2680	7750	432.39	105.21	
MARCH	31	0.06	4.95	67.69	0.00	0.00	0.00	0.09	2320	4720	455.91	118.99	
APRIL	30	0.00	0.00	0.00	0.00	0.00	0.00	0.08	1710	1000	417.96	93.50	
MAY	31	0.00	6.75	55.72	0.14	1.38	56.26	0.10	2790	2090	440.58	89.92	
JUNE	30	0.00	0.00	0.00	0.17	2.36	58.04	0.20	4815	12860	456.22	133.39	
JULY	31	0.01	7.95	60.57	0.10	3.16	68.72	0.15	2430	15250	433.57	106.35	
AUGUST	31	0.00	0.00	0.00	0.00	0.00	0.00	0.11	520	2200	431.09	96.44	
SEPTEMBER	30	0.01	6.65	50.23	0.14	1.93	53.39	0.12	605	3090	434.80	103.24	

MONTH	DAYS	B.O.D. %		REMOVAL		%	FROM OCTOBER 67 TO SEPTEMBER 68		%	MISCELLANEOUS		AVG. TEMP. AIR	AVG. TEMP. SEWAGE
		GROSS WT. REM. POUNDS	NET POUNDS	GROSS WT. REM. POUNDS	NET POUNDS		DIG. SLUDGE TOTAL VOLATILE	AVG. TEMP. AIR					
OCTOBER	31	74.0	765980.	71.4	1172763.	76.0	762103.	67.3	5.63%	65.35%	57 F	70.3 F	
NOVEMBER	30	77.3	878879.	75.4	1249272.	81.5	964266.	77.3	4.51%	63.63%	41 F	64.7 F	
DECEMBER	31	72.6	784210.	70.2	897542.	68.7	637835.	61.0	4.46%	73.78%	42 F	60.6 F	
JANUARY	31	73.2	791141.	71.7	1040286.	74.7	912467.	72.1	5.89%	72.21%	30 F	58.7 F	
FEBRUARY	29	71.1	577131.	69.7	593576.	67.3	477879.	62.3	6.10%	62.40%	34 F	56.2 F	
MARCH	31	72.4	838674.	71.2	798770.	68.9	688548.	65.7	4.95%	67.69%	48 F	59.8 F	
APRIL	30	74.9	592198.	73.8	617413.	74.2	596646.	73.5	0.0%	0.0%	55 F	64.8 F	
MAY	31	68.8	547189.	65.9	638807.	70.9	582157.	69.0	6.75%	55.72%	61 F	67.8 F	
JUNE	30	75.1	833593.	70.1	1157971.	74.5	527328.	57.1	0.0%	0.0%	74 F	71.8 F	
JULY	31	77.6	790449.	75.5	887279.	74.9	281879.	48.7	7.95%	60.51%	80 F	76.3 F	
AUGUST	31	79.0	701196.	78.7	746040.	78.9	681822.	77.4	0.0%	0.0%	78 F	77.6 F	
SEPTEMBER	30	73.1	662063.	72.6	718075.	78.8	624528.	76.3	6.65%	50.23%	70 F	75.6 F	

MONTH	DAYS	B.O.D. %		REMOVAL		%	FROM OCTOBER 67 TO SEPTEMBER 68		%	MISCELLANEOUS		AVG. TEMP. AIR	AVG. TEMP. SEWAGE
		GROSS WT. REM. POUNDS	NET POUNDS	GROSS WT. REM. POUNDS	NET POUNDS		DIG. SLUDGE TOTAL VOLATILE	AVG. TEMP. AIR					
OCTOBER	31	74.0	765980.	71.4	1172763.	76.0	762103.	67.3	5.63%	65.35%	57 F	70.3 F	
NOVEMBER	30	77.3	878879.	75.4	1249272.	81.5	964266.	77.3	4.51%	63.63%	41 F	64.7 F	
DECEMBER	31	72.6	784210.	70.2	897542.	68.7	637835.	61.0	4.46%	73.78%	42 F	60.6 F	
JANUARY	31	73.2	791141.	71.7	1040286.	74.7	912467.	72.1	5.89%	72.21%	30 F	58.7 F	
FEBRUARY	29	71.1	577131.	69.7	593576.	67.3	477879.	62.3	6.10%	62.40%	34 F	56.2 F	
MARCH	31	72.4	838674.	71.2	798770.	68.9	688548.	65.7	4.95%	67.69%	48 F	59.8 F	
APRIL	30	74.9	592198.	73.8	617413.	74.2	596646.	73.5	0.0%	0.0%	55 F	64.8 F	
MAY	31	68.8	547189.	65.9	638807.	70.9	582157.	69.0	6.75%	55.72%	61 F	67.8 F	
JUNE	30	75.1	833593.	70.1	1157971.	74.5	527328.	57.1	0.0%	0.0%	74 F	71.8 F	
JULY	31	77.6	790449.	75.5	887279.	74.9	281879.	48.7	7.95%	60.51%	80 F	76.3 F	
AUGUST	31	79.0	701196.	78.7	746040.	78.9	681822.	77.4	0.0%	0.0%	78 F	77.6 F	
SEPTEMBER	30	73.1	662063.	72.6	718075.	78.8	624528.	76.3	6.65%	50.23%	70 F	75.6 F	

MONTH	DAYS	B.O.D. %		REMOVAL		%	FROM OCTOBER 67 TO SEPTEMBER 68		%	MISCELLANEOUS		AVG. TEMP. AIR	AVG. TEMP. SEWAGE
		GROSS WT. REM. POUNDS	NET POUNDS	GROSS WT. REM. POUNDS	NET POUNDS		DIG. SLUDGE TOTAL VOLATILE	AVG. TEMP. AIR					
OCTOBER	31	74.0	765980.	71.4	1172763.	76.0	762103.	67.3	5.63%	65.35%	57 F	70.3 F	
NOVEMBER	30	77.3	878879.	75.4	1249272.	81.5	964266.	77.3	4.51%	63.63%	41 F	64.7 F	
DECEMBER	31	72.6	784210.	70.2	897542.	68.7	637835.	61.0	4.46%	73.78%	42 F	60.6 F	
JANUARY	31	73.2	791141.	71.7	1040286.	74.7	912467.	72.1	5.89%	72.21%	30 F	58.7 F	
FEBRUARY	29	71.1	577131.	69.7	593576.	67.3	477879.	62.3	6.10%	62.40%	34 F	56.2 F	
MARCH	31	72.4	838674.	71.2	798770.	68.9	688548.	65.7	4.95%	67.69%	48 F	59.8 F	
APRIL	30	74.9	592198.	73.8	617413.	74.2	596646.	73.5	0.0%	0.0%	55 F	64.8 F	
MAY	31	68.8	547189.	65.9	638807.	70.9	582157.	69.0	6.75%	55.72%	61 F	67.8 F	
JUNE	30	75.1	833593.	70.1	1157971.	74.5	527328.	57.1	0.0%	0.0%	74 F	71.8 F	
JULY	31	77.6	790449.	75.5	887279.	74.9	281879.	48.7	7.95%	60.51%	80 F	76.3 F	
AUGUST	31	79.0	701196.	78.7	746040.	78.9	681822.	77.4	0.0%	0.0%	78 F	77.6 F	
SEPTEMBER	30	73.1	662063.	72.6	718075.	78.8	624528.	76.3	6.65%	50.23%	70 F	75.6 F	

MONTH	DAYS	B.O.D. %		REMOVAL		%	FROM OCTOBER 67 TO SEPTEMBER 68		%	MISCELLANEOUS		AVG. TEMP. AIR	AVG. TEMP. SEWAGE
		GROSS WT. REM. POUNDS	NET POUNDS	GROSS WT. REM. POUNDS	NET POUNDS		DIG. SLUDGE TOTAL VOLATILE	AVG. TEMP. AIR					
OCTOBER	31	74.0	765980.	71.4	1172763.	76.0	762103.	67.3	5.63%	65.35%	57 F	70.3 F	
NOVEMBER	30	77.3	878879.	75.4	1249272.	81.5	964266.	77.3	4.51%	63.63%	41 F	64.7 F	
DECEMBER	31	72.6	784210.	70.2	897542.	68.7	637835.	61.0	4.46%	73.78%	42 F	60.6 F	
JANUARY	31	73.2	791141.	71.7	1040286.	74.7	912467.	72.1	5.89%	72.21%	30 F	58.7 F	
FEBRUARY	29	71.1	577131.	69.7	593576.	67.3	477879.	62.3	6.10%	62.40%	34 F	56.2 F	
MARCH	31	72.4	838674.	71.2	798770.	68.9	688548.	65.7	4.95%	67.69%	48 F	59.8 F	
APRIL	30	74.9	592198.	73.8	617413.	74.2	596646.	73.5	0.0%	0.0%	55 F	64.8 F	
MAY	31	68.8	547189.	65.9	638807.	70.9	582157.	69.0	6.75%	55.72%	61 F	67.8 F	
JUNE	30	75.1	833593.	70.1	1157971.	74.5	527328.	57.1	0.0%	0.0%	74 F	71.8 F	
JULY	31	77.6	790449.	75.5	887279.	74.9	281879.	48.7	7.95%	60.51%	80 F	76.3 F	
AUGUST	31	79.0	701196.	78.7	746040.	78.9	681822.	77.4	0.0%	0.0%	78 F	77.6 F	
SEPTEMBER	30	73.1	662063.	72.6	718075.	78.8	624528.	76.3	6.65%	50.23%	70 F	75.6 F	

MONTH	DAYS	B.O.D. %		REMOVAL		%		FROM OCTOBER 67 TO SEPTEMBER 68		TABLE NO. 4		%	MISCELLANEOUS		AVG. TEMP. AIR	AVG. TEMP. SEWAGE
		GROSS WT. REM. POUNDS	NET WT. REM. POUNDS	GROSS WT. REM. POUNDS	NET WT. REM. POUNDS	S.S. %	REMOVAL	DIG. SLUDGE TOTAL VOLATILE	AVG. TEMP. AIR							
OCTOBER	31	74.0	765980.	71.4	1172763.	76.0	762103.	67.3	5.63%	65.35%	57 F	70.3 F				
NOVEMBER	30	77.3	878879.	75.4	1249272.	81.5	964266.	77.3	4.51%	63.63%	41 F	64.7 F				
DECEMBER	31	72.6	784210.	70.2	897542.	68.7	637835.	61.0	4.46%	73.78%	42 F	60.6 F				
JANUARY	31	73.2	791141.	71.7	1040286.	74.7	912467.	72.1	5.89%	72.21%	30 F	58.7 F				
FEBRUARY	29	71.1	577131.	69.7	593576.	67.3	477879.	62.3	6.10%	62.40%	34 F	56.2 F				
MARCH	31	72.4	838674.	71.2	798770.	68.9	688548.	65.7	4.95%	67.69%	48 F	59.8 F				
APRIL	30	74.9	592198.	73.8	617413.	74.2	596646.	73.5	0.0%	0.0%	55 F	64.8 F				
MAY	31	68.8	547189.	65.9	638807.	70.9	582157.	69.0	6.75%	55.72%	61 F	67.8 F				
JUNE	30	75.1	833593.	70.1	1157971.	74.5	527328.	57.1	0.0%	0.0%	74 F	71.8 F				
JULY	31	77.6	790449.	75.5	887279.	74.9	281879.	48.7	7.95%	60.51%	80 F	76.3 F				
AUGUST	31	79.0	701196.	78.7	746040.	78.9	681822.	77.4	0.0%	0.0%	78 F	77.6 F				
SEPTEMBER	30	73.1	662063.	72.6	718075.	78.8	624528.	76.3	6.65%	50.23%	70 F	75.6 F				

TABLE NO. 1										
FROM OCTOBER 68 TO SEPTEMBER 69										
MONTH	DAYS	STATION # 1			STATION # 2			STATION # 3		
		FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L
OCTOBER	31	18.34	180	175	18.47	182	184	7.09	66	60
NOVEMBER	30	21.05	163	167	21.16	164	172	5.47	72	59
DECEMBER	31	20.32	180	180	20.43	182	186	5.58	74	68
JANUARY	31	21.31	187	169	21.45	191	178	4.47	87	83
FEBRUARY	28	21.46	166	153	21.59	171	156	5.10	79	65
MARCH	31	22.15	175	170	22.27	180	177	4.34	86	73
APRIL	30	20.28	186	171	20.39	189	175	5.18	87	67
MAY	31	20.89	195	167	20.19	183	171	5.57	87	57
JUNE	30	20.56	221	154	20.99	208	185	5.00	98	68
JULY	31	23.54	236	145	20.68	237	216	4.80	104	84
AUGUST	31	22.00	225	201	23.66	254	260	3.68	81	71
SEPTEMBER	30			214	22.12	240	264	4.48	86	76

STATION # 4			STATION # 5		
FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L
25.56	149	149	25.49	118	87
26.63	145	148	26.56	121	88
26.01	158	160	25.93	117	93
25.92	173	161	25.84	133	102
26.69	153	138	26.61	114	82
26.61	164	160	26.54	128	81
25.57	168	153	25.49	131	92
25.76	162	146	25.67	130	81
25.99	186	162	25.91	137	83
25.08	211	191	25.38	152	96
27.34	230	234	27.24	134	98
26.60	214	232	26.51	128	107

TABLE NO. 2										
FROM OCTOBER 68 TO SEPTEMBER 69										
MONTH	DAYS	STATION # 6			STATION # 7			STATION # 8		
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L
OCTOBER	31	0.07	3.29	82.83	18.40	66	60	18.38	45	40
NOVEMBER	30	0.07	3.16	85.12	21.09	72	59	21.06	46	44
DECEMBER	31	0.08	3.40	86.33	20.35	74	68	20.33	55	48
JANUARY	31	0.08	3.03	85.28	21.37	87	83	21.35	60	58
FEBRUARY	28	0.08	3.06	85.10	21.51	79	65	21.49	56	51
MARCH	31	0.07	3.14	85.48	22.20	86	73	22.18	63	58
APRIL	30	0.08	3.27	84.96	20.31	87	67	20.29	61	47
MAY	31	0.09	3.66	85.01	20.10	87	57	20.09	51	39
JUNE	30	0.08	4.62	79.22	20.91	98	68	20.89	69	55
JULY	31	0.10	4.72	79.45	20.58	104	84	20.56	70	63
AUGUST	31	0.10	4.93	75.02	23.56	81	71	23.54	57	58
SEPTEMBER	30	0.09	5.04	77.82	22.03	86	76	22.01	66	67

STATION # 9			STATION # 10			STATION # 11		
FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L
0.02	2.31	75.82	18.37	40	40	18.37	40	40
0.02	2.43	79.04	21.06	44	44	21.06	44	44
0.02	2.47	79.61	20.32	48	48	20.32	48	48
0.02	2.46	81.08	21.34	58	58	21.34	58	58
0.02	2.52	81.63	21.46	51	51	21.46	51	51
0.02	2.55	81.54	22.16	58	58	22.16	58	58
0.02	2.66	82.18	20.28	47	47	20.28	47	47
0.02	2.16	81.08	20.04	39	39	20.04	39	39
0.02	2.41	75.65	20.79	55	55	20.79	55	55
0.02	2.74	72.77	20.44	63	63	20.44	63	63
0.02	3.01	70.93	23.42	58	58	23.42	58	58
0.02	2.91	74.49	21.90	67	67	21.90	67	67

MONTH	DAYS	STATION #12				STATION #13				STATION #14				TRICKLING FILTER LOADING	
		FLOW	TOTAL	VOL.	MGD	FLOW	TOTAL	SOLIDS	SOLID	MGD	8.O.D.	MG/L	S.S.	GAL/FT ²	# 8.O.D./1000FT ³ /DAY
		MGD	SOLIDS	SOLID		MGD	SOLIDS	SOLID		MGD	8.O.D.	MG/L	S.S.	PER DAY	
OCTOBER	31	0.02	6.45	50.37	0.12	0.12	0.43	65.58	0.13	0.13	398	1420	1420	422.56	97.85
NOVEMBER	30	0.01	5.86	54.06	0.11	0.11	0.53	65.08	0.11	0.11	299	920	920	440.26	104.54
DECEMBER	31	0.01	6.18	55.55	0.11	0.11	0.55	66.72	0.11	0.11	471	1200	1200	429.90	98.70
JANUARY	31	0.01	5.50	57.89	0.13	0.13	0.66	65.87	0.14	0.14	694	1555	1555	428.39	111.81
FEBRUARY	28	0.01	5.29	70.43	0.10	0.10	0.29	57.36	0.13	0.13	960	485	485	441.18	98.70
MARCH	31	0.02	2.94	68.71	0.10	0.10	0.40	66.88	0.12	0.12	1140	1360	1360	439.95	110.51
APRIL	30	0.02	3.04	66.48	0.10	0.10	0.30	67.67	0.11	0.11	645	870	870	422.59	108.64
MAY	31	0.01	7.69	77.37	0.04	0.04	0.24	61.47	0.09	0.09	1710	890	890	425.66	108.59
JUNE	30	0.01	9.73	75.25	0.00	0.00	0.00	0.00	0.10	0.10	3010	6830	6830	429.53	115.48
JULY	31	0.02	4.57	68.49	0.00	0.00	0.00	0.00	0.12	0.12	2920	12150	12150	420.75	125.50
AUGUST	31	0.01	5.98	71.11	0.00	0.00	0.00	0.00	0.12	0.12	3760	12210	12210	451.58	118.74
SEPTEMBER	30	0.07	3.43	71.55	0.00	0.00	0.00	0.00	0.11	0.11	2940	9810	9810	439.51	110.40

MONTH	DAYS	B.O.D.				REMOVAL				S.S.				STATION #14				TRICKLING FILTER LOADING	
		GROSS	WT. REM.	%	WT. REM.	GROSS	WT. REM.	%	WT. REM.	GROSS	WT. REM.	%	WT. REM.	DIG. SLUDGE	MISCELLANEOUS	AVG. TEMP.	AVG. TEMP.	GAL/FT ²	# 8.O.D./1000FT ³ /DAY
		POUNDS	POUNDS		POUNDS	POUNDS	POUNDS		POUNDS	POUNDS	POUNDS		POUNDS	TOTAL VOLATILE	AVG. TEMP.	AVG. TEMP.	AVG. TEMP.	PER DAY	
OCTOBER	31	655386.	75.4	75.0	642042.	688681.	78.4	77.1	641073.	6.45%	50.37%	77.1	60 F	60 F	60 F	71.7 F	71.7 F	422.56	97.85
NOVEMBER	30	625882.	72.1	71.8	617454.	678774.	74.5	73.8	652840.	5.86%	54.06%	73.8	46 F	46 F	46 F	66.4 F	66.4 F	440.26	104.54
DECEMBER	31	672331.	69.9	69.5	658700.	730239.	74.3	73.4	695511.	6.18%	55.35%	73.4	34 F	34 F	34 F	60.5 F	60.5 F	429.90	98.70
JANUARY	31	728167.	68.7	68.0	703915.	667109.	67.6	65.7	612770.	5.50%	57.89%	65.7	31 F	31 F	31 F	56.7 F	56.7 F	428.39	111.81
FEBRUARY	28	581512.	67.5	66.3	552048.	530941.	67.5	66.9	516056.	5.29%	70.43%	66.9	35 F	35 F	35 F	56.6 F	56.6 F	441.18	98.70
MARCH	31	675379.	65.2	64.0	641437.	686757.	67.4	66.0	646265.	3.84%	68.71%	66.0	42 F	42 F	42 F	59.0 F	59.0 F	439.95	110.51
APRIL	30	654704.	67.9	67.3	636253.	654313.	73.3	72.5	629426.	3.94%	66.48%	72.5	58 F	58 F	58 F	63.7 F	63.7 F	422.59	108.64
MAY	31	691060.	72.3	71.1	649845.	690582.	77.4	76.8	669131.	7.69%	77.37%	76.8	67 F	67 F	67 F	69.7 F	69.7 F	425.66	108.59
JUNE	30	733392.	67.1	64.8	661596.	685436.	70.5	64.6	522524.	9.73%	75.25%	64.6	76 F	76 F	76 F	73.7 F	73.7 F	429.53	115.48
JULY	31	897294.	70.8	68.5	805727.	822000.	71.2	57.0	440996.	4.57%	68.49%	57.0	79 F	79 F	79 F	76.7 F	76.7 F	420.75	125.50
AUGUST	31	1208528.	77.8	76.0	1095951.	1239174.	77.9	71.3	873600.	5.98%	71.11%	71.3	75 F	75 F	75 F	76.8 F	76.8 F	451.58	118.74
SEPTEMBER	30	966694.	72.8	70.9	882101.	1094041.	74.9	68.9	811778.	3.43%	71.55%	68.9	68 F	68 F	68 F	74.5 F	74.5 F	439.51	110.40

TABLE NO. 1
FROM OCTOBER 69 TO SEPTEMBER 70

MONTH	DAYS	STATION # 1				STATION # 2				STATION # 3				STATION # 4				STATION # 5			
		FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L
OCTOBER	31	18.91	223	183	19.01	246	241	6.00	112	86	25.01	213	203	24.94	147	120	24.94	147	120	24.94	147
NOVEMBER	30	18.35	212	159	18.45	224	213	5.90	113	92	24.35	197	183	24.28	137	105	24.28	137	105	24.28	137
DECEMBER	31	22.03	192	160	22.09	199	175	4.23	101	76	26.32	183	159	26.28	135	95	26.28	135	95	26.28	135
JANUARY	31	23.08	209	167	23.18	218	194	3.39	111	85	26.57	204	180	26.49	143	102	26.49	143	102	26.49	143
FEBRUARY	28	25.56	200	189	25.75	209	211	2.30	109	91	28.05	200	201	27.94	139	101	27.94	139	101	27.94	139
MARCH	31	25.74	214	217	25.99	223	241	2.19	111	92	28.18	214	229	28.07	129	102	28.07	129	102	28.07	129
APRIL	30	29.90	183	168	30.12	187	178	0.94	98	81	31.06	184	175	30.94	112	88	30.94	112	88	30.94	112
MAY	31	25.96	164	159	26.26	167	169	1.87	77	65	28.13	161	162	28.01	112	83	28.01	112	83	28.01	112
JUNE	30	23.58	189	161	23.89	193	179	2.41	81	61	26.30	182	168	26.20	124	85	26.20	124	85	26.20	124
JULY	31	23.66	166	145	23.90	169	158	2.14	77	63	26.04	161	150	25.95	107	86	25.95	107	86	25.95	107
AUGUST	31	21.94	166	173	22.16	167	180	3.00	69	59	25.16	155	165	25.08	104	86	25.08	104	86	25.08	104
SEPTEMBER	30	21.44	200	176	21.70	202	188	2.92	75	65	24.62	186	173	24.54	124	91	24.54	124	91	24.54	124

TABLE NO. 2
FROM OCTOBER 69 TO SEPTEMBER 70

MONTH	DAYS	STATION # 6				STATION # 7				STATION # 8				STATION # 9				STATION # 10				STATION # 11			
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	FLOW MGD	B.O.D. MG/L	FLOW MGD	B.O.D. MG/L
OCTOBER	31	0.07	4.45	80.01	18.94	112	86	18.91	75	69	0.02	2.87	73.96	18.82	2.87	73.96	18.82	2.87	73.96	18.82	2.87	73.96	18.82	2.87	73.96
NOVEMBER	30	0.07	4.46	79.74	18.38	113	92	18.35	67	67	0.02	3.09	76.33	18.26	3.09	76.33	18.26	3.09	76.33	18.26	3.09	76.33	18.26	3.09	76.33
DECEMBER	31	0.04	4.09	81.39	22.05	101	76	22.03	69	64	0.02	2.56	79.13	21.96	2.56	79.13	21.96	2.56	79.13	21.96	2.56	79.13	21.96	2.56	79.13
JANUARY	31	0.08	3.90	82.02	23.10	111	85	23.08	75	73	0.02	3.00	81.28	23.01	3.00	81.28	23.01	3.00	81.28	23.01	3.00	81.28	23.01	3.00	81.28
FEBRUARY	28	0.11	3.53	80.66	25.64	109	91	25.62	76	77	0.02	2.71	79.36	25.51	2.71	79.36	25.51	2.71	79.36	25.51	2.71	79.36	25.51	2.71	79.36
MARCH	31	0.11	4.51	83.90	25.80	111	92	25.86	76	74	0.02	2.66	81.79	25.69	2.66	81.79	25.69	2.66	81.79	25.69	2.66	81.79	25.69	2.66	81.79
APRIL	30	0.12	4.21	82.52	30.00	98	81	29.98	69	66	0.02	3.12	80.39	29.88	3.12	80.39	29.88	3.12	80.39	29.88	3.12	80.39	29.88	3.12	80.39
MAY	31	0.12	3.89	82.63	26.14	77	65	26.12	58	48	0.02	2.47	81.32	25.95	2.47	81.32	25.95	2.47	81.32	25.95	2.47	81.32	25.95	2.47	81.32
JUNE	30	0.10	3.85	80.78	23.79	81	61	23.76	59	50	0.02	2.41	77.95	23.58	2.41	77.95	23.58	2.41	77.95	23.58	2.41	77.95	23.58	2.41	77.95
JULY	31	0.09	3.81	80.76	23.81	77	63	23.79	49	44	0.02	2.79	75.15	23.66	2.79	75.15	23.66	2.79	75.15	23.66	2.79	75.15	23.66	2.79	75.15
AUGUST	31	0.08	3.90	81.39	22.08	69	59	22.05	42	41	0.03	2.38	77.48	21.95	2.38	77.48	21.95	2.38	77.48	21.95	2.38	77.48	21.95	2.38	77.48
SEPTEMBER	30	0.08	4.02	81.72	21.62	75	65	21.58	49	48	0.04	2.34	77.22	21.45	2.34	77.22	21.45	2.34	77.22	21.45	2.34	77.22	21.45	2.34	77.22

TABLE NO. 1
FROM OCTOBER 70 TO SEPTEMBER 71

MONTH	DAYS	STATION # 1			STATION # 2			STATION # 3			STATION # 4			STATION # 5		
		FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L
OCTOBER	31	19.38	194	171	19.64	197	188	4.21	79	64	23.85	176	166	23.77	122	86
NOVEMBER	30	22.55	168	168	22.70	170	177	2.87	61	60	25.57	157	163	25.49	102	88
DECEMBER	31	21.74	166	160	21.87	168	172	3.07	69	66	24.94	155	158	24.86	108	91
JANUARY	31	21.29	170	147	21.42	172	160	2.79	66	60	24.21	159	148	24.13	98	85
FEBRUARY	28	25.12	170	148	25.23	176	153	2.08	70	69	27.31	167	146	27.24	101	85
MARCH	31	24.53	185	161	24.47	194	175	1.53	77	70	26.00	187	168	25.93	113	89
APRIL	30	20.62	175	145	20.76	179	159	3.40	64	68	24.16	162	146	24.09	109	86
MAY	31	21.20	156	156	21.30	157	162	3.91	53	54	25.21	140	145	25.15	95	81
JUNE	30	17.08	178	149	18.11	180	160	4.82	51	51	22.93	152	137	22.87	96	80
JULY	31	15.75	181	156	15.87	182	168	7.30	52	50	23.17	141	130	23.11	99	78
AUGUST	31	16.77	161	145	16.88	162	149	6.67	46	45	23.55	129	119	23.49	89	70
SEPTEMBER	30	21.04	162	154	21.19	163	163	3.81	61	54	25.00	147	146	24.93	106	81

TABLE NO. 2
FROM OCTOBER 70 TO SEPTEMBER 71
STATION # 8

MONTH	DAYS	STATION # 6				STATION # 7				STATION # 8				STATION # 9				STATION #10		STATION #11	
		FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	B.O.D. MG/L	S.S. MG/L	FLOW MGD	TOTAL SOLIDS	VOL. SOLID	FLOW MGD	S.S. MG/L	FLOW MGD	S.S. MG/L	FLOW MGD	S.S. MG/L		
OCTOBER	31	0.08	3.98	83.66	19.56	79	64	19.53	51	44	2.13	79.10	19.39	0.14							
NOVEMBER	30	0.08	3.73	83.85	22.62	61	60	22.60	46	47	2.64	79.50	22.56	0.04							
DECEMBER	31	0.08	3.84	84.12	21.79	69	66	21.77	50	52	2.33	81.33	21.66	0.10							
JANUARY	31	0.08	3.67	85.76	21.34	66	60	21.52	52	51	3.34	83.33	21.25	0.08							
FEBRUARY	28	0.07	3.51	83.54	25.16	70	69	25.14	54	54	2.75	82.30	25.04	0.10							
MARCH	31	0.07	3.85	84.50	24.40	77	70	24.38	62	57	2.71	81.72	24.28	0.09							
APRIL	30	0.07	3.80	82.77	20.69	64	68	20.67	53	56	2.72	81.06	20.60	0.07							
MAY	31	0.06	3.77	81.36	21.24	53	54	21.22	39	37	2.50	79.67	21.17	0.05							
JUNE	30	0.06	3.57	81.77	18.05	51	51	18.03	38	33	2.59	76.82	17.98	0.05							
JULY	31	0.06	3.68	82.05	15.81	52	50	15.79	36	37	2.99	74.16	15.75	0.04							
AUGUST	31	0.06	3.54	81.40	16.82	46	45	16.80	37	34	2.69	72.80	16.78	0.02							
SEPTEMBER	30	0.07	3.75	83.46	21.12	61	54	21.09	50	39	2.68	77.20	21.04	0.05							

MONTH	DAYS	TABLE NO. 3				TABLE NO. 4				TABLE NO. 5				TABLE NO. 6			
		FROM OCTOBER 70 TO SEPTEMBER 71				FROM OCTOBER 70 TO SEPTEMBER 71				FROM OCTOBER 70 TO SEPTEMBER 71				FROM OCTOBER 70 TO SEPTEMBER 71			
		STATION #13				STATION #12				STATION #11				STATION #10			
		FLOW	TOTAL	SOLIDS	VOL. SOLID	FLOW	TOTAL	SOLIDS	VOL. SOLID	FLOW	TOTAL	SOLIDS	VOL. SOLID	FLOW	TOTAL	SOLIDS	VOL. SOLID
		MGD				MGD				MGD				MGD			
OCTOBER	31	0.01	5.28	5.28	53.99	0.11	0.61	0.61	66.69	0.26	0.26	0.26	0.26	0.26	0.26	0.26	0.26
NOVEMBER	30	0.01	5.72	5.72	54.88	0.11	0.41	0.41	64.98	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
DECEMBER	31	0.01	5.47	5.47	55.42	0.03	0.37	0.37	67.34	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.13
JANUARY	31	0.01	5.95	5.95	57.84	0.06	0.37	0.37	62.19	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.13
FEBRUARY	28	0.00	5.58	5.58	65.25	0.01	0.59	0.59	66.35	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
MARCH	31	0.01	5.11	5.11	68.98	0.05	1.50	1.50	71.75	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14
APRIL	30	0.05	3.93	3.93	63.94	0.07	0.82	0.82	69.62	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14
MAY	31	0.03	3.05	3.05	62.05	0.05	0.30	0.30	65.86	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10
JUNE	30	0.02	3.52	3.52	60.28	0.08	0.30	0.30	63.47	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.13
JULY	31	0.02	2.79	2.79	59.47	0.09	0.26	0.26	60.52	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12
AUGUST	31	0.01	4.07	4.07	55.03	0.08	0.30	0.30	63.84	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
SEPTEMBER	30	0.02	6.22	6.22	56.26	0.09	0.23	0.23	62.50	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15

TRICKLING FILTER LOADING		S.S.		B.O.D.		PER DAY		GAL/FT ²		# B.O.D./1000FT ³ /DAY	
		MG/L		MG/L							
		1440		372		394.06		94.34			
		1450		325		422.63		84.59			
		2110		382		412.11		87.34			
		2130		385		400.08		76.94			
		1200		1390		451.55		89.50			
		2500		1673		429.87		95.32			
		2100		629		399.36		85.42			
		1370		249		416.93		77.73			
		1640		394		379.18		71.43			
		1585		250		383.11		74.43			
		730		293		389.46		68.02			
		1410		287		413.24		85.96			

MONTH	DAYS	TABLE NO. 7				TABLE NO. 8				TABLE NO. 9				TABLE NO. 10			
		FROM OCTOBER 70 TO SEPTEMBER 71				FROM OCTOBER 70 TO SEPTEMBER 71				FROM OCTOBER 70 TO SEPTEMBER 71				FROM OCTOBER 70 TO SEPTEMBER 71			
		FLOW	TOTAL	SOLIDS	VOL. SOLID	FLOW	TOTAL	SOLIDS	VOL. SOLID	FLOW	TOTAL	SOLIDS	VOL. SOLID	FLOW	TOTAL	SOLIDS	VOL. SOLID
		MGD				MGD				MGD				MGD			
OCTOBER	31	0.01	5.28	5.28	53.99	0.11	0.61	0.61	66.69	0.26	0.26	0.26	0.26	0.26	0.26	0.26	0.26
NOVEMBER	30	0.01	5.72	5.72	54.88	0.11	0.41	0.41	64.98	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15
DECEMBER	31	0.01	5.47	5.47	55.42	0.03	0.37	0.37	67.34	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.13
JANUARY	31	0.01	5.95	5.95	57.84	0.06	0.37	0.37	62.19	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.13
FEBRUARY	28	0.00	5.58	5.58	65.25	0.01	0.59	0.59	66.35	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
MARCH	31	0.01	5.11	5.11	68.98	0.05	1.50	1.50	71.75	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14
APRIL	30	0.05	3.93	3.93	63.94	0.07	0.82	0.82	69.62	0.14	0.14	0.14	0.14	0.14	0.14	0.14	0.14
MAY	31	0.03	3.05	3.05	62.05	0.05	0.30	0.30	65.86	0.10	0.10	0.10	0.10	0.10	0.10	0.10	0.10
JUNE	30	0.02	3.52	3.52	60.28	0.08	0.30	0.30	63.47	0.13	0.13	0.13	0.13	0.13	0.13	0.13	0.13
JULY	31	0.02	2.79	2.79	59.47	0.09	0.26	0.26	60.52	0.12	0.12	0.12	0.12	0.12	0.12	0.12	0.12
AUGUST	31	0.01	4.07	4.07	55.03	0.08	0.30	0.30	63.84	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
SEPTEMBER	30	0.02	6.22	6.22	56.26	0.09	0.23	0.23	62.50	0.15	0.15	0.15	0.15	0.15	0.15	0.15	0.15

TRICKLING FILTER LOADING		S.S.		B.O.D.		PER DAY		GAL/FT ²		# B.O.D./1000FT ³ /DAY	
		MG/L		MG/L							
		1440		372		394.06		94.34			
		1450		325		422.63		84.59			
		2110		382		412.11		87.34			
		2130		385		400.08		76.94			
		1200		1390		451.55		89.50			
		2500		1673		429.87		95.32			
		2100		629		399.36		85.42			
		1370		249		416.93		77.73			
		1640		394		379.18		71.43			
		1585		250		383.11		74.43			
		730		293		389.46		68.02			
		1410		287		413.24		85.96			

TABLE NO. 4			
FROM OCTOBER 70 TO REMOVAL		S.S. %	
REMOVAL NET	%	GROSS WT. REM. POUNDS	WT. REM. POUNDS
720065.	73.8	734003.	76.9
693521.	72.8	739994.	73.6
656637.	70.1	681273.	70.1
653682.	69.6	605917.	68.4
686682.	68.5	585719.	65.0
777922.	66.6	749249.	67.7
634670.	69.9	531253.	65.1
644890.	75.1	689601.	77.3
642285.	78.7	576484.	79.5
592072.	80.2	538627.	78.1
538433.	77.0	502753.	77.3
590505.	69.2	658892.	76.2
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APPENDIX B

LINEAR VERSUS NON LINEAR REGRESSION

APPENDIX B

LINEAR VERSUS NON LINEAR REGRESSION

B.1 General

The intrinsically linear model defined as

$$Y = B_0 \prod_{i=1}^n X_i^{B_i} e_i \quad (B-1)$$

represents the adopted general form for the BOD and suspended solids removal models of the Alexandria sewage treatment plant. Using the original form (Equation B-1) data must be regressed by the use of non linear regression techniques such as defined in Chapter 5. However, through basic mathematical manipulations the intrinsically linear model can be reduced to one with linear parameters. Thus, Equation B-1 can be transformed to:

$$Y' = \ln Y = \ln B_0 + \sum_{i=1}^n B_i \ln X_i + \ln e_i \quad (B-2)$$

It must however be emphasized that the requirements for valid tests of significance and confidence interval estimate are now $\ln \epsilon \approx N(0, I\sigma^2)$, rather than $\epsilon \approx N(0, I\sigma^2)$

B.2 Results

Linear and non linear regression outputs for the TF2-BOD model have been included herein so as to illustrate actual test results. Such results will hereafter be

summarized so as to clearly point the characteristics of each regression.

B-2-1 Linear Regression

Model: TF2-BOD (linear)
No. of cases: 83
No. of independent variables: 4
No. of original variables: 5
Multiple correlation coefficient (R): 0.9546
Coefficient of determination (R^2): 0.9112
Standard error of the estimate(s): 0.08319
Variance of the estimate (s^2): 0.00692
Range of residuals: 0.465

B-2-2 Non Linear Regression

Model: TF2-BOD (non linear)
No. of cases: 83
No. of independent variables: 4
No. of original variables: 5
Multiple correlation coefficient (R): 0.9533
Coefficient of determination (R^2): 0.9088
Standard error of the estimate: 5.27928

B.3 Conclusions

Not only does the linearized form of the model yield slightly better regression results than the non linear form, probably due to the rounding-off errors within the convergence technique, but the use of linear regression algorithms makes it much easier to prepare the data for regression. Thus, because in all cases tested, the trend reported above was also observed, it was decided to adopt the linear regression algorithms to fit the equations to the data.

BMD03R - MULTIPLE REGRESSION WITH CASE COMBINATIONS - REVISED AUGUST 22, 1969
HEALTH SCIENCES COMPUTING FACILITY, UCLA

NO. OF SUBSAMPLES 1
NO. OF ORIGINAL VARIABLES ... 5
TOT. COMB. SAMP. SIZE ... 83
ALTERNATE TAPE NO. 5
NO. OF VAR. FORMAT CARDS ... 1
THE VARIABLE FORMAT IS (2E13,6,13X,E13,6,/,/,52X,2E13,6)

SELECTION NO, 1- 0
SUB-SAMPLES SELECTED 1

SUMS	374,92188	237,26555	348,57520	330,27905	341,76050
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SUM OF SQUARES	1697,44971	679,42505	1464,08330	1317,88965	1413,31030
----------------	------------	-----------	------------	------------	------------

CROSS PRODUCT SUMS					
ROW 1	1697,44971	1073,49902	1574,40015	1494,93213	1548,02856
ROW 2	1073,49902	679,42505	996,44580	945,32300	978,61304
ROW 3	1574,40015	996,44580	1464,88330	1386,33423	1434,52710
ROW 4	1494,93213	945,32300	1386,33423	1317,88965	1364,29004
ROW 5	1548,02856	978,61304	1434,52710	1364,29004	1413,31030

CROSS PRODUCTS OF DEVIATIONS

ROW 1	3,87915	1,73999	-0,15918	3,01880	4,25244
ROW 2	1,73999	1,17285	0,00171	1,18091	1,65015
ROW 3	-0,15918	0,00171	0,97217	-0,73877	-0,76416
ROW 4	3,01880	1,18091	-0,73877	3,62158	4,33447
ROW 5	4,25244	1,65015	-0,76416	4,33447	6,07935

SELECTION NO. 1- 1

CORRELATION COEFFICIENTS

ROW 1	1.000	0.8157	-.8197E-010	8054	0.8757
ROW 2	0.8157	1.000	0.1600E-020	5730	0.6180
ROW 3	-.8197E-010	0.1600E-02	1.000	-.3937	-.3143
ROW 4	8054	0.5730	-.3937	1.000	0.9238
ROW 5	0.8757	0.6180	-.3143	0.9238	1.000

INVERSE OF CORRELATION COEFFICIENT MATRIX

ROW 1	6.795	-3.239	-1.020	-4.019
ROW 2	-3.239	3.182	0.4586E-010	8037
ROW 3	-1.020	0.4586E-01	1.455	1.368
ROW 4	-4.019	8037	1.368	4.315

SELECTION NO. 1 1

SAMPLE SIZE 83 NO. OF VARIABLES DELETED 0 (FOR VARIABLES DELETED, SEE BELOW)

NO. OF VARIABLES 5
DEPENDENT VARIABLE IS NOW NO. 5

COEFFICIENT OF DETERMINATION 0.9112
MULTIPLE CORR. COEFFICIENT 0.9546

SUM OF SQUARES ATTRIBUTABLE TO REGRESSION 5.53953
SUM OF SQUARES OF DEVIATION FROM REGRESSION 0.53981

VARIANCE OF ESTIMATE 0.00692
STD. ERROR OF ESTIMATE 0.08319

INTERCEPT (A VALUE) -0.24521

ANALYSIS OF VARIANCE FOR THE MULTIPLE

SOURCE OF VARIATION	LINEAR	REGRESSION	O.F.	SUM OF SQUARES	MEAN SQUARES	F VALUE
DUE TO REGRESSION.....			4	5.53953	1.38488	200.1080
DEVIATION ABOUT REGRESSION...			78	0.53981	0.00692	
TOTAL...			82	6.07935		

VARIABLE NO.	MEAN	STD. DEVIATION	REG. COEFF.	STD. ERROR OF REG. COE.	COMPUTED T VALUE	PARTIAL CORR. COE.	SUM OF SQ. ADDED	PROP. VAR. CUM.
1	4.51713	0.21750	0.69747	0.11011	6.33457	0.58283	4.66165	0.76680
2	2.85862	0.11960	-0.32346	0.13702	-2.36075	-0.25824	0.16870	0.02775
3	4.19970	0.10888	-0.14605	0.10177	-1.43501	-0.16039	0.30826	0.05071
4	3.97927	0.21016	0.69114	0.09080	7.61150	0.65283	0.110093	0.06595
5	4.11760	0.27228						

COMP. CHECK ON FINAL COEFF. 0.69114

VARIABLES DELETED... 0

TABLE OF RESIDUALS

OBSERVATION	Y VALUE	Y ESTIMATE	RESIDUAL
1	4.02595	3.92956	0.09639
2	3.88758	3.89772	-0.01014
3	4.15242	4.03489	0.11753
4	4.20545	4.08088	0.12457
5	4.15827	4.03340	0.12487
6	4.09424	4.03192	0.06232
7	4.12860	3.99191	0.13669
8	3.81979	3.74810	0.07169
9	3.70433	3.64629	0.05804
10	3.56976	3.55839	0.01137
11	3.66957	3.64737	0.02220
12	3.69560	3.71083	-0.01523
13	3.78724	3.89118	-0.10394
14	4.09849	4.17312	-0.07463
15	3.88667	3.96151	-0.07484
16	4.11856	4.13922	-0.02066
17	4.17338	4.11395	0.05943
18	3.96715	3.95259	0.01456
19	3.97000	3.82862	0.14138
20	3.80347	3.81070	-0.00723
21	3.68463	3.77979	-0.09516
22	3.72125	3.79550	-0.07425
23	3.75479	4.00943	-0.25464
24	3.70255	3.70163	0.00092
25	3.83314	3.90027	-0.06713
26	3.94757	4.02828	-0.08071
27	3.96596	4.09147	-0.12551
28	4.16051	4.16287	-0.00236
29	4.17420	4.18704	-0.01285
30	4.04825	4.11540	-0.06715
31	4.05432	4.07834	0.02402
32	4.34269	4.30184	0.04085
33	4.22562	4.15767	0.06795
34	4.39188	4.35988	0.03200
35	4.33931	4.32952	0.00979
36	4.49382	4.61253	-0.11871
37	4.46981	4.52171	-0.05190
38	4.46449	4.51665	-0.05216
39	4.38711	4.39364	-0.00653
40	4.34653	4.34383	0.00270
41	4.55213	4.40273	0.14940
42	4.06443	4.02933	0.03510
43	4.26138	4.05057	0.21081
44	4.55286	4.62550	-0.07264
45	4.32699	4.29835	0.02864
46	4.33770	4.33770	0.00000

40	4.03124	4.03371	0.00120
47	4.07551	4.09063	-0.01513
48	4.00205	4.08595	-0.008350
49	4.13058	4.14668	-0.01610
50	4.13397	4.20947	-0.07550
51	4.29234	4.44235	-0.15001
52	4.22524	4.19739	0.02785
53	4.30750	4.34969	0.04219
54	4.27870	4.25220	0.02650
55	4.28574	4.12963	0.15611
56	4.41410	4.28982	0.12428
57	4.45286	4.48056	-0.02170
58	4.27364	4.35160	-0.07796
59	4.30637	4.34228	-0.03591
60	4.50908	4.46739	0.04209
61	4.49155	4.44755	0.04400
62	4.45843	4.40065	0.05778
63	4.56080	4.53607	0.02473
64	4.59591	4.62111	-0.02520
65	4.61874	4.57402	0.04472
66	4.59152	4.48169	0.10983
67	4.25087	4.21109	0.03978
68	4.23471	4.15743	0.07728
69	4.17448	4.06140	0.11308
70	4.03068	3.95870	0.07198
71	4.09230	4.12703	-0.03473
72	4.11230	4.07739	0.03499
73	3.92367	3.99438	-0.07071
74	4.02187	4.08533	-0.06346
75	3.94762	3.92918	0.01844
76	4.12769	4.17891	-0.05122
77	4.17371	4.20765	-0.03394
78	3.91518	4.07571	-0.16053
79	3.76965	3.85513	-0.08548
80	3.63616	3.71164	-0.07548
81	3.66601	3.72349	-0.05748
82	3.55972	3.59378	-0.03406
83	3.90145	3.90768	-0.00623

TEST OF EXTREME RESIDUALS

RANGE OF RESIDUALS.....
RANGE / STD, ERROR OF ESTIMATE....

0.465
5.595

0.45154933E 00	-0.21281338E 00	-0.11840910E 00	0.67507821E 00	0.66875702E 00
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OBS	PRED	DIFF	0.69009689E 02	0.13092899E 02	0.65270889E 02
0.56033493E 02	0.50215546E 02	0.58179474E 01	0.47186096E 02		
0.48792587E 02	0.48820984E 02	-0.2636606E-01	0.61883286E 02	0.13288400E 02	0.62120499E 02

0,6-765-02	0,5878979E 02	0,7598079E 01	0,5240795E 02	0,1455700E 02	0,55001190E 02
0,67050797E 02	0,58683334E 02	0,83674622E 01	0,52443497E 02	0,14883800E 02	0,57801590E 02
0,63960495E 02	0,56442963E 02	0,75175323E 01	0,56322693E 02	0,15952700E 02	0,59511597E 02
0,59993698E 02	0,56078690E 02	0,39150085E 01	0,53899292E 02	0,14963300E 02	0,62841797E 02
0,62091187E 02	0,53912262E 02	0,81789246E 01	0,53252899E 02	0,14508300E 02	0,68871887E 02
0,45594589E 02	0,42750610E 02	0,28439789E 01	0,51633698E 02	0,14883800E 02	0,74180893E 02
0,40622894E 02	0,38648880E 02	0,19740143E 01	0,69062393E 02	0,14313299E 02	0,75980896E 02
0,35507996E 02	0,35500824E 02	0,71716309E 02	0,36109192E 02	0,14075000E 02	0,76880493E 02
0,39234985E 02	0,38847717E 02	0,38726807E 00	0,59602692E 02	0,14515499E 02	0,75710297E 02
0,40269485E 02	0,41219482E 02	0,94999695E 00	0,34239899E 02	0,14898299E 02	0,71119797E 02
0,44134491E 02	0,48634796E 02	0,45003052E 01	0,58852493E 02	0,13995600E 02	0,64820587E 02
0,60249496E 02	0,63153152E 02	0,29036560E 01	0,44134491E 02	0,12984599E 02	0,61400894E 02
0,48748398E 02	0,52025040E 02	0,32766418E 01	0,53230087E 02	0,14053399E 02	0,58070999E 02
0,61470398E 02	0,62234055E 02	0,76365662E 00	0,69640488E 02	0,15331599E 02	0,56811798E 02
0,64934586E 02	0,60635147E 02	0,42994385E 01	0,49381133E 02	0,15172700E 02	0,59781891E 02
0,52833786E 02	0,51913605E 02	0,92018127E 00	0,60089996E 02	0,14660000E 02	0,63291489E 02
0,52984497E 02	0,46418488E 02	0,65660095E 01	0,50852585E 02	0,16111496E 02	0,67071686E 02
0,44856796E 02	0,45897354E 02	0,10405579E 01	0,74033096E 02	0,16595398E 02	0,72651489E 02
0,39830399E 02	0,44492462E 02	0,46620636E 01	0,42097290E 02	0,16075394E 02	0,77060898E 02
0,41315796E 02	0,45040210E 02	0,37324219E 01	0,71023193E 02	0,16089890E 02	0,77870392E 02
0,42725189E 02	0,55635635E 02	0,12910446E 02	0,44109085E 02	0,16638687E 02	0,75079987E 02
0,40550598E 02	0,41427124E 02	0,87652588E 00	0,68797897E 02	0,17310287E 02	0,69770096E 02
0,46207489E 02	0,49486542E 02	0,32790527E 01	0,76832993E 02	0,15540999E 02	0,64820587E 02
0,51800097E 02	0,56104935E 02	0,42958374E 01	0,39866196E 02	0,16429291E 02	0,60860687E 02
0,52771088E 02	0,59974014E 02	0,72029266E 01	0,77954697E 02	0,17483597E 02	0,58701599E 02
			0,53968597E 02		
			0,69403992E 02		
			0,38990997E 02		
			0,77712585E 02		
			0,43406998E 02		
			0,88075485E 02		
			0,46628189E 02		
			0,89789597E 02		
			0,51195786E 02		
			0,04577300E 02		

0,04104302C 02	0,04202C103L 02	-0,10040508E 01	0,5410090E 03	0,18494690E 02	0,59061996E 02
0,64987595E 02	0,65991653E 02	-0,10040508E 01	0,10164699E 03	0,18494690E 02	0,59061996E 02
0,57297394E 02	0,61677490E 02	-0,43800964E 01	0,53323196E 02	0,18169693E 02	0,63921890E 02
0,57646194E 02	0,59615417E 02	-0,19692230E 01	0,52386200E 02	0,18545197E 02	0,67881195E 02
0,76914291E 02	0,74008316E 02	0,29059753E 01	0,48456192E 02	0,18761887E 02	0,71661697E 02
0,68416595E 02	0,64676208E 02	0,37403870E 01	0,11494899E 03	0,19224091E 02	0,76250793E 02
0,80792587E 02	0,78682419E 02	0,21101605E 01	0,58319595E 02	0,19707886E 02	0,76520386E 02
0,76654507E 02	0,76450378E 02	0,20420837E 00	0,10738799E 03	0,20256790E 02	0,74270187E 02
0,89462997E 02	0,99734390E 02	-0,10271393E 02	0,51961990E 02	0,18559692E 02	0,70310287E 02
0,87339798E 02	0,90468109E 02	-0,31283112E 01	0,12340799E 03	0,17310287E 02	0,64730194E 02
0,86876892E 02	0,91363403E 02	-0,44865112E 01	0,61260300E 02	0,19801195E 02	0,60591385E 02
0,80407700E 02	0,80968887E 02	-0,56118774E 00	0,12867000E 03	0,19613998E 02	0,58700699E 02
0,77210297E 02	0,76885391E 02	0,32490540E 00	0,56578400E 02	0,18834091E 02	0,56181686E 02
0,94834198E 02	0,81709412E 02	0,13124706E 02	0,12959599E 03	0,19859589E 02	0,59782196E 02
0,50231491E 02	0,56746307E 02	0,14851837E 01	0,80265900E 02	0,18205795E 02	0,64821487E 02
0,70907990E 02	0,58383560E 02	0,12524429E 02	0,12789000E 03	0,19195190E 02	0,67791290E 02
0,94903198E 02	0,10168358E 03	-0,67803802E 01	0,67844299E 02	0,19873993E 02	0,71751587E 02
0,75716095E 02	0,74090500E 02	0,16255951E 01	0,11912000E 03	0,18884598E 02	0,76340988E 02
0,56672791E 02	0,57492477E 02	-0,81968689E 00	0,76129288E 02	0,18776291E 02	0,77600388E 02
0,58880798E 02	0,60543091E 02	-0,16622925E 01	0,1310100E 03	0,18942398E 02	0,75620193E 02
0,54731888E 02	0,60106216E 02	-0,53743286E 01	0,66270096E 02	0,18407990E 02	0,71660187E 02
0,62213791E 02	0,63808441E 02	-0,15946503E 01	0,10520999E 03	0,19180695E 02	0,66440186E 02
0,62425293E 02	0,67638107E 02	-0,52128143E 01	0,6483292E 02	0,18725800E 02	0,60500595E 02
0,73137192E 02	0,84438004E 02	-0,11300812E 02	0,11898999E 03	0,18660797E 02	0,56721298E 02
0,68390900E 02	0,66916489E 02	0,14744110E 01	0,64415695E 02	0,19216797E 02	0,56631699E 02
0,74254990E 02	0,77443466E 02	-0,31884766E 01	0,93498398E 02	0,19166290E 02	0,58971786E 02

0,7776500002	0,8500000002	0,9600000002	0,1000000002	0,1800000002	0,2600000002
0,7265599102	0,6253611802	0,1011987302	0,5556109602	0,1853799402	0,6968139602
0,8260759002	0,7316513102	0,9442459101	0,4760218802	0,1871128802	0,7373118602
0,8587199402	0,8782717902	-0,1955184901	0,1255049903	0,1832859802	0,7670079002
0,7178248602	0,7813032502	-0,6347839401	0,6935809302	0,1967179902	0,7679039002
0,7417109702	0,7731436202	-0,3143264801	0,1103940002	0,1914459202	0,7445028702
0,9087429802	0,8649881002	0,4375488301	0,6546585002	0,1801078802	0,7040019202
0,8925929302	0,8469059802	0,4568695101	0,6977839702	0,1753419502	0,6518049602
0,8635218802	0,8125640002	0,5095779401	0,7267129502	0,1697848502	0,6041069002
0,9566018702	0,9249852002	0,3161666901	0,1154209903	0,1913018802	0,5663108802
0,9907849102	0,1009743003	-0,1895813001	0,1232379903	0,2017729202	0,5627189602
0,1013660003	0,9680291702	0,4563079801	0,7325329602	0,2027119402	0,5924169902
0,9864448502	0,8960437002	0,9040115401	0,8271688802	0,2234379602	0,6320199602
0,7016659502	0,6843313602	0,1733459501	0,1178039903	0,2022789002	0,6887159702
0,6904188502	0,6452854902	0,4513336201	0,8401499902	0,1892079202	0,7355119302
0,6500619502	0,5901904302	0,5987152101	0,1127370002	0,1874018902	0,7625079302
0,5629928602	0,5330812102	0,2991165201	0,8153270002	0,1811189302	0,7688049302
0,5987728902	0,6238772602	-0,2510437001	0,5923149102	0,1772198502	0,7589039602
0,6109179702	0,5926159702	0,1830200201	0,5199492002	0,1716589402	0,7265008502
0,5058549502	0,5510972602	-0,4524231001	0,9033329002	0,1840799002	0,6689019802
0,5580538902	0,5989884902	-0,4093460101	0,5186890002	0,1795298802	0,6167079202
0,5181169102	0,5138237002	0,4293212900	0,4856888002	0,1742588802	0,5789090002
0,6203419502	0,6607176202	-0,4037567101	0,4813999902	0,1967179902	0,5672189302
0,6495608502	0,6756062302	-0,2604538001	0,9899708602	0,1872580002	0,5951159702
0,5015829502	0,5921658302	-0,9058288601	0,5189369202	0,1739698802	0,6302189602

APPENDIX C

MODEL BUILDING EQUATIONS (TABLE FORM)

TABLE C.1

PT1 - SS Model

<u>1. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	SS _{po}	#/D	0.46666	11.52473	R = 0.9191
2	OR1	GPD/FT ²	0.61539	6.44145	R ² = 0.8447
3	T	°F	-0.21988	-2.90769	s = 0.0660
4	R	--	0.07259	0.80612	RofR = 0.395
5	SS _{pe}	#/D			

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of the residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals indicate that 3.6% of the transformed residuals fall beyond the allowable range at 5% α -level.

b) Parameters: Since $t_c = \pm 1.45$, variable #4 has only a chance influence on the regression.

3. EQUATION

$$SS_{pe} = \exp(1.59103) SS_{po}^{.46666} OR1^{0.61539} T^{-.21988} R^{0.07259}$$

TABLE C.2

TF1 - SS Model

1. REGRESSION RESULTS

#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	SS _{to}	#/D/1000cf	1.02175	6.85495	R = 0.9647
2	HR	MGAD	-0.43305	-0.92743	R ² = 0.9307
3	R	--	-1.25568	-2.76700	s = 0.09829
4	OR2	GPD/sf	0.09772	0.22756	RofR = 0.393
5	T	°F	-0.55533	-4.77846	Intercept: 0.68404
6	L _{to}	#/D/1000cf	0.35640	2.83366	
7	SS _{se}				

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of the residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals reveal that only 3.6% of the transformed residuals fall beyond the allowable range at 5% α -level.

b) Parameters: Since $t_c = \pm 1.45$ variables #2, and #4, are thereby not significant into the regression.

3. EQUATION

$$SS_{te} = \exp(0.68404) SS_{to}^{1.02175} HR^{-0.43305} R^{-1.25568} OR2^{0.09772} T^{-0.55533} L_{to}^{0.35640}$$

TABLE C.3

TF1 - BOD Model

<u>I. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	L_{to}	#/D/1000cf	0.62734	5.99397	$R = 0.9772$ $R^2 = 0.9548$ $RofR = 0.392$ $s = 0.08555$ Intercept: 0.37351
2	HR	MGAD	-0.17092	-0.41828	
3	R	--	-1.00072	-2.49060	
4	OR2	GPD/sf	0.04968	0.13315	
5	T	$^{\circ}F$	-0.19942	-1.16272	
6	SS_{se}	#/D/1000cf	0.42843	5.45227	
7	L_{se}	#/D/1000cf			

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals indicate that only 4.8% of the transformed residuals fall beyond the allowable range at 5% α -level.

b) Parameters: Since $t_c = \pm 1.45$, variables #2, and #4, appear only to have a chance influence on the regression.

3. EQUATION

$$L_{se} = \exp(0.37351) L_{to}^{0.62734} HR^{-0.17092} R^{-1.00072} OR2^{0.04968} T^{-0.19942} SS_{se}^{0.42843}$$

TABLE C.4

TF2 - BOD Model

<u>I. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	L_{to}	#/D/1000cf	0.72372	6.38392	$R = 0.9551$ $R^2 = 0.9123$ $RofR = 0.472$ $s = 0.08322$ Intercept: -0.0952
2	HR	MGAD	-0.43194	-2.44933	
3	R	--	-0.11691	-0.97755	
4	T	°F	-0.11311	-1.05473	
5	SS_{te}	#/D/1000cf	0.67326	7.26619	
6	L_{to}				

2. TESTS

a) Model

i) Residuals: No definite patterns in the plot of residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals indicated that 1.2% of the transformed residuals fall beyond the allowable range at 5% α -level.

b) Parameters: Since $t_c = \pm 1.45$, variables #3, and #4, are not significant into the regression.

3. EQUATION

$$L_{te} = \exp(-0.09052) L_{to}^{0.72372} HR^{-0.43194} R^{-0.11691} T^{-0.11311} SS_{te}^{0.67326}$$

TABLE C. 5

ST2 - SS Model

<u>I. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	SS _{so}	#/D	1.02147	18.97592	R = 0.9599
2	OR2	GPD/sf	0.22673	3.47982	R ² = 0.9797
3	T	°F	-0.08032	-0.93589	RofR = 0.394
4	SS _{se}	#/D			s = 0.07349
					Intercept: -1.69498

2. TESTS

a) Model

i) Residuals: No definite patterns in the plot of residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals indicate that none of the transformed residuals fall beyond the allowable range at 5% α -level.

b) Parameters: Since $t_c = \pm 1.45$, variable #3, has only a chance influence in the regression.

3. EQUATION

$$SS_{se} = \exp(-1.69498) SS_{so}^{1.02147} OR2^{0.22673} T^{-0.08032}$$

TABLE C. 6

ST2 - BOD Model

<u>I. REGRESSION RESULTS</u>					
#	Symbol	Units	Regression Coefficient	't' value	Miscellaneous
1	L_{so}	#/D	0.45573	6.36815	$R = 0.9782$
2	OR1	GPD/sf	0.45153	5.71500	$R^2 = 0.9569$
3	T	$^{\circ}F$	-0.10172	-1.05373	$RofR = 0.388$
4	SS_{se}	#/D	0.35228	4.26469	$s = 0.08267$
5	L_{se}	#/D			Intercept: -1.08414

2. TESTS

a) Model

i) Residuals: No definite pattern in the plot of residuals was observed.

ii) U.N.D.: The Unit Normal Deviate form of the residuals indicate that none of the transformed residuals fall beyond the allowable range at 5% α -level.

b) Parameters: Since $t_c = \pm 1.45$, variable #3 has only a chance influence on the regression.

3. EQUATION

$$L_{se} = \exp(-1.08414) L_{so}^{0.45573} OR2^{6.36815} T^{-0.10172}$$

$$SS_{se}^{0.35228}$$

APPENDIX D

LISTING OF CSTF PROGRAM

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MAIN

FORTRAN IV G LEVEL 20

CMAIN PROGRAM FOR CSTF(COMPUTER SIMULATION OF TRICKLING FILTER)

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      INTEGER BOD,SS,T,BODIN,SSIN
      DIMENSION BOD(3),SS(3),FLOW(3),R(3),T(3)
      N=3
      READ(1,1) (BOD(I),SS(I),FLOW(I),R(I),T(I),I=1,N)
      ISET=1
      DO 100 I=1,N
      DO 100 J=1,N
      BODIN=BOD(I)
      SSIN=SS(J)
      CALL COTF(BODIN,SSIN,FLOW,R,T,N,ISET)
      ISET=ISET+1
100  CONTINUE
      1  FORMAT(I3,I3,F5,2,F4,2,I2)
      RETURN
      END

```

FORTRAN IV G LEVEL 20

COTF

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0001 SUBROUTINE COTF(BOD,SS,FLOW,R,T,N,ISET)
0002 INTEGER BOD,SS,T,BOD2,SS2,BOD3,SS3,BOD4,SS4,BOD5,SS5,BOD7,SS7,BOD8
0003 1,SSR,BOD9,SS9
      DIMENSION FLOW(3),R(3),T(3),FLOW2(27),BOD2(27),SS2(27),BOD3(27),SS3(27),
      13(27),BOD4(27),SS4(27),FLOW4(27),BOD5(27),SS5(27),FLOW6(27),
      22(27),FLOW7(27),BOD7(27),SS7(27),FLOW8(27),BOD8(27),SS8(27),FLOW9(27),
      33,BOD9(27),SS9(27),BOD2L(27),BOD3L(27),BOD4L(27),BOD5L(27),BOD7L(27),
      47),BOD8L(27),BOD9L(27),SS2L(27),SS3L(27),SS4L(27),SS5L(27),SS7L(27),
      5),SS8L(27),SS9L(27),BOD5R(27),SS5R(27),BOD5R(27),PSS5R(27),BOD9R(
      627),SS9R(27),BOD9R(27),PSS9R(27),CPBODR(27),CPSSR(27),FLOW10(27),
      70BODR(27),OSSR(27)
      DIMENSION BOD8R(27),SS8R(27),P80D8R(27),PSS8R(27)
      DIMENSION FLOW3(27)
      DIMENSION FLOW4(27)
      ITERMA=150
      AREAST=22500.
      AREATF=60310.51/43560.
      VOLTF=256.35363
      AREAPT=24912.
      25 II=1
      DO 100 K=1,N
      DO 100 L=1,N
      DO 100 M=1,N
      JJ=0
      BOD3(II)=80
      SS3(II)=76
      C.....STATION # 4
      500 JJ=JJ+1
      FLOW4(II)=FLOW(K)
      BOD4(II)=BOD
      SS4(II)=SS
      BOD4L(II)=BOD4(II)*8.34*FLOW4(II)
      SS4L(II)=SS4(II)*8.34*FLOW4(II)
      C.....STATION # 2
      FLOW2(II)=FLOW4(II)/R(L)
      FLOW3(II)=FLOW4(II)-FLOW2(II)
      BOD2(II)=(BOD4(II)+FLOW4(II)-BOD3(II)*FLOW3(II))/FLOW2(II)
      SS2(II)=(SS4(II)+FLOW4(II)-SS3(II)*FLOW3(II))/FLOW2(II)
      BOD2L(II)=BOD2(II)*8.34*FLOW2(II)
      SS2L(II)=SS2(II)*8.34*FLOW2(II)
      BOD3L(II)=BOD3(II)*8.34*FLOW3(II)
      SS3L(II)=SS3(II)*8.34*FLOW3(II)
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C..... PRIMARY TANK
OR1=(FLOW4(II)*(10**6))/AREAPT
B0=1.84233
B1=0.46764
B2=-0.19160
B3=0.56284
SS5L(II)=(EXP(B0))*(SS4L(II)**B1)*(T(M)**B2)*(OR1**B3)
FLOW6(II)=(SS4L(II)-SS5L(II))/(0.050)*(8.34)
FLOW5(II)=(FLOW4(II)-(FLOW6(II)/100000.))
SS5(II)=SS5L(II)/(8.34*FLOW5(II))
SS5R(II)=SS4L(II)-SS5L(II)
A0=-2.40023
A1=0.62517
A2=0.58027
A3=-0.11353
A4=0.34668
A5=0.23933
B0D5L(II)=(EXP(A0))*(B0D4L(II)**A1)*(OR1**A2)*(T(M)**A3)*(H(L)**A4)
S)*(SS5L(II)**A5)
B0D5(II)=B0D5L(II)/(8.34*FLOW5(II))
B0D5R(II)=B0D4L(II)-B0D5L(II)
PB0D5R(II)=(FLOAT(B0D4(II)-B0D5(II))/FLOAT(B0D4(II)))*100.
PSS5R(II)=(FLOAT(SS4(II)-SS5(II))/FLOAT(SS4(II)))*100.

C..... TRICKLING FILTER
SSTFL=SS5L(II)/VOLTf
HRTF=FLOW5(II)/AREATF
B0DTFL=B0D5L(II)/VOLTf
B0=2.11470
B1=0.94714
B2=-0.66828
B3=-0.26370
B4=-0.42106
B5=0.34868
SSTFE=(EXP(B0))*(SSTFL**B1)*(HRTF**B2)*(R(L)**B3)*(T(M)**B4)*(B0DT
1FL**B5)
SS9L(II)=SSTFE/VOLTf
SS9(II)=SS9L(II)/(8.34*FLOW5(II))
SS9R(II)=SS5L(II)-SS9L(II)
PSS9R(II)=(SS9R(II)/SS5L(II))*100.
CPSSR(II)=(FLOAT(SS2(II)-SS9(II))/FLOAT(SS2(II)))*100.
A0=-0.24521
A1=0.69747

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0070 A2=-0.32346
0071 A3=-0.14605
0072 A4=0.69114
0073 BODTFC=(EXP(A0))*(BODTFL**A1)*(HRTF**A2)*(T(M)**A3)*(SSTFE**A4)
0074 HOD9L(II)=BODTFC*VOLIF
0075 BOD9(II)=HOD9L(II)/(8.34*FLOW5(II))
0076 BOD9R(II)=BOD9L(II)-BOD9(II)
0077 PHOD9R(II)=(BOD9R(II)/HOD5L(II))*100.
0078 CPBODR(II)=(FLOAT(BOD2(II)-BOD9(II))/FLOAT(BOD2(II)))*100.
0079 LIMIT=2
0080 DIF1=IABS(SS9(II)-SS3(II))
0081 DIF2=IABS(BOD9(II)-BOD3(II))
0082 IF(DIF1.GE.LIMIT.AND.SS9(II).GT.SS3(II)) GO TO 600
0083 IF(DIF1.GE.LIMIT.AND.SS9(II).LT.SS3(II)) GO TO 601
0084 IF(DIF1.LT.LIMIT) GO TO 602
0085 SS3(II)=SS3(II)+0.50*(SS9(II)-SS3(II))
0086 GO TO 602
0087 SS3(II)=SS3(II)+0.50*(SS9(II)-SS3(II))
0088 GO TO 602
0089 IF(DIF2.GE.LIMIT.AND.BOD9(II).GT.BOD3(II)) GO TO 603
0090 IF(DIF2.GE.LIMIT.AND.BOD9(II).LT.BOD3(II)) GO TO 604
0091 IF(DIF2.LT.LIMIT) GO TO 605
0092 BOD3(II)=BOD3(II)+0.50*(BOD9(II)-BOD3(II))
0093 IF(JJ.GT.ITERMA) GO TO 700
0094 GO TO 500
0095 BOD3(II)=BOD3(II)+0.50*(BOD9(II)-BOD3(II))
0096 IF(JJ.GT.ITERMA) GO TO 700
0097 GO TO 500
0098 605 CONTINUE

C.....STATION # 7
0099 FLOW7(II)=FLOW5(II)-FLOW3(II)
0100 BOD7(II)=BOD3(II)
0101 SS7(II)=SS3(II)
0102 BOD7L(II)=BOD7(II)*8.34*FLOW7(II)
0103 SS7L(II)=SS7(II)*8.34*FLOW7(II)

C.....SECONDARY SETTLING TANKS
0104 OR2=(FLOW7(II)*(10**6))/AREAST
0105 B0=-2.12146
0106 B1=1.04553
0107 B2=0.20681
0108 SS8L(II)=(EXP(B0))*(SS7L(II)**B1)*(OR2**B2)

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0109  FLOW10(II)=(SS7L(II)-SS8L(II))/(0.032*8.34)
0110  FLOW8(II)=FLOW7(II)-(FLOW10(II)/1000000.)
0111  SS8R(II)=SS7L(II)-SS8L(II)
0112  PSS8R(II)=SS8R(II)/(SS7L(II))*100.
0113  SS8(II)=SS8L(II)/(8.34*FLOW8(II))
0114  QSSR(II)=(FLOW10(II)-SS8(II))/FLOW10(II)*100.
0115  A0=-1.56093
0116  A1=0.45429
0117  A2=0.42325
0118  A3=0.38054
0119  B008L(II)=(EXP(A0))*(B007L(II)**A1)*(OR2**A2)*(SS8L(II)**A3)
0120  B008R(II)=B007L(II)-B008L(II)
0121  PB008R(II)=(B008R(II)/B007L(II))*100.
0122  B008(II)=B008L(II)/(8.34*FLOW8(II))
0123  OB008R(II)=(FLOW10(II)-B008(II))/FLOW10(II)*100.
0124  II=II+1
0125  100 CONTINUE

C
C .....WRITE TABLE 1
C
      WRITE(3,400) ISET,SS,B0D
      II=1
      DO 101 K=1,N
      DO 101 L=1,N
      DO 101 M=1,N
      IF (FLOW4(II).LT.15.70.OR.FLOW4(II).GT.31.06) GO TO 680
      WRITE(3,401) II,SS,B0D,FLOW(K),R(L),T(M),FLOW2(II),B0D2(II),SS2(II),
      1),FLOW3(II),B0D3(II),SS3(II),FLOW4(II),B0D4(II),SS4(II),FLOW5(II),
      2B0D5(II),SS5(II),FLOW7(II),B0D7(II),SS7(II),FLOW8(II),B0D8(II),SS8
      3(II)
      GO TO 681
680  WRITE(3,404) II,SS,B0D,FLOW(K),R(L),T(M),FLOW2(II),B0D2(II),SS2(II),
      1),FLOW3(II),B0D3(II),SS3(II),FLOW4(II),B0D4(II),SS4(II),FLOW5(II),
      2B0D5(II),SS5(II),FLOW7(II),B0D7(II),SS7(II),FLOW8(II),B0D8(II),SS8
      3(II)
      681 II=II+1
      101 CONTINUE
      WRITE(3,403)

C
C .....WRITE TABLE 2
C

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0138 WRITE(3,405)
0139 II=1
0140 DO 102 K=1,N
0141 DO 102 L=1,N
0142 DO 102 M=1,N
0143 IF(FLOW4(II),LT,15.70,OR,FLOW4(II),GT,31.06) GO TO 682
0144 WRITE(3,406) II,SS,BOD,FLOW(K),R(L),T(M),BOD4L(II),BOD5L(II),BOD5R(II),P80
      1(II),PBOD5R(II),SS4L(II),SS5L(II),SS5R(II),PSS5R(II),FLOW6(II)
      GO TO 683
0145 WRITE(3,408) II,SS,BOD,FLOW(K),R(L),T(M),BOD4L(II),BOD5L(II),BOD5R
0146 1(II),PBOD5R(II),SS4L(II),SS5L(II),SS5R(II),PSS5R(II),FLOW6(II)
      683 II=II+1
0147 102 CONTINUE
0148 WRITE(3,407)
0149
C
C
C.....WRITE TABLE 3
C
0150 WRITE(3,410)
0151 II=1
0152 DO 103 K=1,N
0153 DO 103 L=1,N
0154 DO 103 M=1,N
0155 IF(FLOW4(II),LT,15.70,OR,FLOW4(II),GT,31.06) GO TO 684
0156 WRITE(3,411) II,SS,BOD,FLOW(K),R(L),T(M),BOD5L(II),BOD5R(II),P80
      1(II),PBOD5R(II),SS5L(II),SS5R(II),PSS5R(II),CPBODK(II),CP
      2SSR(II)
      GO TO 685
0157 WRITE(3,413) II,SS,BOD,FLOW(K),R(L),T(M),BOD5L(II),BOD5R(II),P80
0158 1(II),PBOD5R(II),SS5L(II),SS5R(II),PSS5R(II),CPBODK(II),CP
      2SSR(II)
      685 II=II+1
      103 CONTINUE
      WRITE(3,412)
0159
0160
0161
C
C
C.....WRITE TABLE 4
C
0162 WRITE(3,415)
0163 II=1
0164 DO 104 K=1,N
0165 DO 104 L=1,N
0166 DO 104 M=1,N

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1*****1,16X,1*****
2*****1,1X,1TRIAL*,1,44X,1H 0 D',20X,1SUSP
3ENDED SOLIDS',1,3X,1# *SS BOD FLOW RECIRC T * # IN',5
4N',4X,1# OUT',4X,1# REM',4X,1X',3X,1*,2X,
5
6X,1X',2X,1SLUDGE',2X,1PLANT EFF',1,6X,1*,105X,1BOD S.S.',2X,1REM
SARK')
416 FORMAT(2X,I2,2X,I*,1,3,2X,I3,2X,F5,2,3X,F4,2,3X,I2,1X,I*,1,F7,1,2X,
1F7,1,2X,F7,1,3X,F4,1,1X,I*,1X,
F7,1,2X,F7,1,2X,F7,1,3X,F4,1,1X,F7,0,1X,F4,
$
2,1,1X,F4,1)
417 FORMAT(1H0,1,51X,1NOTE: # IN =POUNDS ENTERING',1,58X,1# OUT =POU
NDS LEAVING',1,58X,1# REM =POUNDS REMOVED)',1,58X,1#
=PERCENTAG
SE REMOVED FOR THAT UNIT',1,58X,1SLUDGE=GALLONS OF SLUDGE PRODUCED
$ ASSUMING 3.2% SOLIDS',1,58X,1PLANT EFF= OVERALL PLANT EFFICIENCY'
$,1,58X,1O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE',1,65X,1RANGE
SGE OF THE ESTABLISHED OPERATION',1,65X,1MIN=15.70,MAX=31.061)
418 FORMAT(2X,I2,2X,I*,1,3,2X,I3,2X,F5,2,3X,F4,2,3X,I2,1X,I*,1,F7,1,2X,
1F7,1,2X,F7,1,3X,F4,1,1X,I*,1X,
F7,1,2X,F7,1,2X,F7,1,3X,F4,1,1X,F7,0,1X,F4,
$
2,1,1X,F4,1,2X,1O.O.R.',1)
C
C
800 FORMAT(1H0,1,10X,1SS=1,13,2X,1BOD=1,13,2X,1FLOW=1,F5,2,2X,1RECIRC=1,F4,2
1C=1,F4,2,2X,1TEMP=1,12,2X,1K=1,11,2X,1L=1,11,2X,1M=1,11,2X,1TRIAL=
2',12,2X,1BOD3=1,13,2X,1BOD9=1,13,2X,1SYSTEM UNSTABLE')
C
C
801 RETURN
0193 END
0194

```

APPENDIX E

SENSITIVITY ANALYSIS OUTPUT

COMPUTER SIMULATION OF TRICKLING FILTER OPERATION
ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 1 : SS =119 MG/L
BOD =125 MG/L

O U T P U T -- S T R E A M V E C T O R S

INPUT VECTORS

TRIAL#	*SS	BOD	FLOW	RECIRC	T	*STATION # 2	FLOW	BOD SS	*STATION # 3	FLOW	BOD SS	*STATION # 4	FLOW	BOD SS	*STATION # 5	FLOW	BOD SS	*STATION # 7	FLOW	BOD SS	*STATION # 8 *		
1	*119	125	15.70	1.05	56	*14.95	128	121	0.75	56	67	15.70	125	119	15.69	73	76	14.94	56	67	14.93	39	46*
2	*119	125	15.70	1.05	67	*14.95	128	121	0.75	49	59	15.70	125	119	15.69	71	74	14.94	49	59	14.93	35	40*
3	*119	125	15.70	1.05	78	*14.95	129	122	0.75	44	54	15.70	125	119	15.69	69	71	14.94	44	54	14.93	32	37*
4	*119	125	15.70	1.31	56	*11.98	146	135	3.72	57	65	15.70	125	119	15.69	78	76	11.97	57	65	11.96	36	42*
5	*119	125	15.70	1.31	67	*11.98	148	138	3.72	50	57	15.70	125	119	15.69	76	74	11.97	50	57	11.96	32	37*
6	*119	125	15.70	1.31	78	*11.98	149	139	3.72	45	52	15.70	125	119	15.69	74	71	11.97	45	52	11.96	30	33*
7	*119	125	15.70	1.57	56	*10.00	163	150	5.70	58	63	15.70	125	119	15.69	83	76	9.99	58	63	9.98	34	39*
8	*119	125	15.70	1.57	67	*10.00	167	154	5.70	51	56	15.70	125	119	15.69	80	74	9.99	51	56	9.98	30	34*
9	*119	125	15.70	1.57	78	*10.00	170	157	5.70	46	51	15.70	125	119	15.69	78	71	9.99	46	51	9.98	28	31*
10	*119	125	24.29	1.05	56	*23.13	128	121	1.16	62	62	24.29	125	119	24.27	89	77	23.11	62	62	23.10	47	47*
11	*119	125	24.29	1.05	67	*23.13	128	122	1.16	55	55	24.29	125	119	24.27	86	75	23.11	55	55	23.10	42	42*
12	*119	125	24.29	1.05	78	*23.13	128	122	1.16	49	50	24.29	125	119	24.27	84	72	23.11	49	50	23.10	38	38*
13	*119	125	24.29	1.31	56	*18.54	143	137	5.75	64	60	24.29	125	119	24.27	95	77	18.52	64	60	18.51	43	43*
14	*119	125	24.29	1.31	67	*18.54	146	139	5.75	56	53	24.29	125	119	24.27	92	75	18.52	56	53	18.51	39	38*
15	*119	125	24.29	1.31	78	*18.54	147	141	5.75	51	48	24.29	125	119	24.27	90	72	18.52	51	48	18.51	35	34*
16	*119	125	24.29	1.57	56	*15.47	159	153	8.82	65	58	24.29	125	119	24.27	101	77	15.45	65	58	15.44	40	40*
17	*119	125	24.29	1.57	67	*15.47	163	157	8.82	57	51	24.29	125	119	24.27	98	75	15.45	57	51	15.44	36	35*
18	*119	125	24.29	1.57	78	*15.47	167	160	8.82	51	47	24.29	125	119	24.27	95	72	15.45	51	47	15.44	33	32*
19	*119	125	31.06	1.05	56	*29.58	127	121	1.48	66	59	31.06	125	119	31.03	99	78	29.55	66	59	29.55	51	48*
20	*119	125	31.06	1.05	67	*29.58	128	122	1.48	58	52	31.06	125	119	31.03	97	75	29.55	58	52	29.55	46	42*
21	*119	125	31.06	1.05	78	*29.58	128	122	1.48	52	47	31.06	125	119	31.03	94	73	29.55	52	47	29.54	42	38*
22	*119	125	31.06	1.31	56	*23.71	142	137	7.35	68	57	31.06	125	119	31.03	106	78	23.68	68	57	23.68	47	44*
23	*119	125	31.06	1.31	67	*23.71	145	140	7.35	60	51	31.06	125	119	31.03	103	75	23.68	60	51	23.67	43	39*
24	*119	125	31.06	1.31	78	*23.71	147	141	7.35	54	46	31.06	125	119	31.03	101	73	23.68	54	46	23.67	39	35*
25	*119	125	31.06	1.57	56	*19.78	156	154	11.28	69	55	31.06	125	119	31.03	112	78	19.76	69	55	19.75	44	40*
26	*119	125	31.06	1.57	67	*19.78	161	158	11.28	61	49	31.06	125	119	31.03	109	75	19.76	61	49	19.75	40	35*
27	*119	125	31.06	1.57	78	*19.78	164	160	11.28	55	45	31.06	125	119	31.03	107	73	19.76	55	45	19.75	37	32*

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R.=MEANSFLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70, MAX=31.06

INPUT VECTORS ***** OUTPUT ***** PRIMARY TANK CHARACTERISTICS *****

TRIAL #	*SS	BOD	FLOW	RECIRC	T	#	IN	BOD	#	OUT	%	#	IN	SUSPENDED	#	OUT	#	REM	SOLIDS	%	SLUDGE	REMARK
1	*119	125	15.70	1.05	56	*16367.2	9598.4	9598.4	56	*15581.6	41.6	*15581.6	10033.7	5548.0	36.1	13304.						
2	*119	125	15.70	1.05	67	*16367.2	9328.0	9328.0	67	*15581.6	43.2	*15581.6	9694.7	5886.9	37.0	14117.						
3	*119	125	15.70	1.05	78	*16367.2	9104.7	9104.7	78	*15581.6	44.8	*15581.6	9416.4	6165.2	40.3	14785.						
4	*119	125	15.70	1.31	56	*16367.2	10272.3	10272.3	56	*15581.6	37.6	*15581.6	10033.7	5548.0	36.1	13304.						
5	*119	125	15.70	1.31	67	*16367.2	9982.8	9982.8	67	*15581.6	39.2	*15581.6	9694.7	5886.9	37.0	14117.						
6	*119	125	15.70	1.31	78	*16367.2	9743.8	9743.8	78	*15581.6	40.8	*15581.6	9416.4	6165.2	40.3	14785.						
7	*119	125	15.70	1.57	56	*16367.2	10858.8	10858.8	56	*15581.6	33.6	*15581.6	10033.7	5548.0	36.1	13304.						
8	*119	125	15.70	1.57	67	*16367.2	10552.8	10552.8	67	*15581.6	36.0	*15581.6	9694.7	5886.9	37.0	14117.						
9	*119	125	15.70	1.57	78	*16367.2	10300.2	10300.2	78	*15581.6	37.6	*15581.6	9416.4	6165.2	40.3	14785.						
10	*119	125	24.29	1.05	56	*25322.3	18088.7	18088.7	56	*24106.8	28.8	*24106.8	15731.3	8375.6	35.3	20085.						
11	*119	125	24.29	1.05	67	*25322.3	17579.0	17579.0	67	*24106.8	31.2	*24106.8	15199.9	8906.9	37.0	21360.						
12	*119	125	24.29	1.05	78	*25322.3	17158.2	17158.2	78	*24106.8	32.8	*24106.8	14763.6	9343.3	39.5	22406.						
13	*119	125	24.29	1.31	56	*25322.3	18813.1	18813.1	56	*24106.8	24.0	*24106.8	15731.3	8375.6	35.3	20085.						
14	*119	125	24.29	1.31	67	*25322.3	18362.7	18362.7	67	*24106.8	26.4	*24106.8	15199.9	8906.9	37.0	21360.						
15	*119	125	24.29	1.31	78	*25322.3	18463.8	18463.8	78	*24106.8	28.0	*24106.8	14763.6	9343.3	39.5	22406.						
16	*119	125	24.29	1.57	56	*25322.3	20463.8	20463.8	56	*24106.8	19.2	*24106.8	15731.3	8375.6	35.3	20085.						
17	*119	125	24.29	1.57	67	*25322.3	19887.2	19887.2	67	*24106.8	21.6	*24106.8	15199.9	8906.9	37.0	21360.						
18	*119	125	24.29	1.57	78	*25322.3	19411.1	19411.1	78	*24106.8	24.0	*24106.8	14763.6	9343.3	39.5	22406.						
19	*119	125	31.06	1.05	56	*32380.0	25849.9	25849.9	56	*30825.8	20.8	*30825.8	20267.1	10558.7	34.5	25321.						
20	*119	125	31.06	1.05	67	*32380.0	25120.9	25120.9	67	*30825.8	22.4	*30825.8	19582.5	11243.3	37.0	26962.						
21	*119	125	31.06	1.05	78	*32380.0	24519.6	24519.6	78	*30825.8	24.8	*30825.8	19020.4	11805.4	38.7	28310.						
22	*119	125	31.06	1.31	56	*32380.0	27664.0	27664.0	56	*30825.8	15.2	*30825.8	20267.1	10558.7	34.5	25321.						
23	*119	125	31.06	1.31	67	*32380.0	26884.5	26884.5	67	*30825.8	17.6	*30825.8	19582.5	11243.3	37.0	26962.						
24	*119	125	31.06	1.31	78	*32380.0	26240.9	26240.9	78	*30825.8	19.2	*30825.8	19020.4	11805.4	38.7	28310.						
25	*119	125	31.06	1.57	56	*32380.0	28243.5	28243.5	56	*30825.8	10.4	*30825.8	20267.1	10558.7	34.5	25321.						
26	*119	125	31.06	1.57	67	*32380.0	28419.4	28419.4	67	*30825.8	12.8	*30825.8	19582.5	11243.3	37.0	26962.						
27	*119	125	31.06	1.57	78	*32380.0	27739.1	27739.1	78	*30825.8	14.4	*30825.8	19020.4	11805.4	38.7	28310.						

NOTE:

IN = POUNDS ENTERING

OUT = POUNDS LEAVING

REM = POUNDS REMOVED

% = PERCENTAGE REMOVED FOR THAT UNIT

SLUDGE=GALLONS OF SLUDGE PRODUCED ASSUMING 5.0% SOLIDS

O.U.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE

RANGE OF THE ESTABLISHED PLANT OPERATION

MIN=15.70, MAX=31.06

INPUT VECTORS										TRICKLING FILTER CHARACTERISTICS										SOLIDS										OVERALL % REM										REMARK									
*****										*****										*****										*****										*****									
TRIAL#	#	SS	BOD	FLOW	RECIRC	T	#	IN	#	OUT	BOD	#	REM	%	#	IN	#	OUT	#	REM	%	BOD	S.S.	REMARK																									
1	*119	125	15.70	1.05	56	*	9598.4	7250.3	2348.1	24.5	*	10033.7	8671.9	1361.7	13.6	57.0	45.5																																
2	*119	125	15.70	1.05	67	*	9328.0	6381.4	2946.6	31.6	*	9694.7	7706.8	1988.0	20.5	62.5	52.1																																
3	*119	125	15.70	1.05	78	*	9104.7	5726.9	3377.8	37.1	*	9416.4	6973.3	2443.2	25.9	66.7	56.6																																
4	*119	125	15.70	1.31	56	*	10272.3	7421.6	2850.7	27.8	*	10033.7	8376.3	1657.4	16.5	61.6	52.6																																
5	*119	125	15.70	1.31	67	*	9982.8	6532.1	3450.7	34.6	*	9694.7	7444.0	2250.7	23.2	66.9	59.4																																
6	*119	125	15.70	1.31	78	*	9743.8	5862.2	3881.7	39.8	*	9416.4	6735.6	2680.9	28.5	70.5	63.3																																
7	*119	125	15.70	1.57	56	*	10858.8	7564.8	3294.0	30.3	*	10033.7	8141.9	1891.8	18.9	65.0	58.7																																
8	*119	125	15.70	1.57	67	*	10552.8	6658.2	3894.6	36.9	*	9694.7	7235.7	2459.0	25.4	70.1	64.3																																
9	*119	125	15.70	1.57	78	*	10300.2	5975.3	4324.9	42.0	*	9416.4	6547.1	2869.4	30.5	73.5	68.2																																
10	*119	125	24.29	1.05	56	*	18088.7	12520.6	5568.1	30.8	*	15731.3	12370.6	3360.6	21.4	52.3	49.6																																
11	*119	125	24.29	1.05	67	*	17579.0	11020.0	6559.0	37.3	*	15199.9	10993.8	4206.1	27.7	57.8	55.7																																
12	*119	125	24.29	1.05	78	*	17158.2	9889.8	7268.4	42.4	*	14763.6	9947.5	4816.1	32.6	62.5	59.8																																
13	*119	125	24.29	1.31	56	*	19358.6	12816.4	6542.2	33.8	*	15731.3	11948.9	3782.3	24.0	55.9	56.9																																
14	*119	125	24.29	1.31	67	*	18813.1	11280.4	7532.7	40.0	*	15199.9	10619.0	4580.8	30.1	62.3	62.6																																
15	*119	125	24.29	1.31	78	*	18362.7	10123.4	8239.3	44.9	*	14763.6	9608.4	5155.2	34.9	66.0	66.7																																
16	*119	125	24.29	1.57	56	*	20463.8	13063.7	7400.2	36.2	*	15731.3	11614.6	4116.7	26.2	59.7	62.7																																
17	*119	125	24.29	1.57	67	*	19887.2	11493.0	8389.2	42.2	*	15199.9	10321.9	4878.0	32.1	65.6	68.2																																
18	*119	125	24.29	1.57	78	*	19411.1	10310.8	9092.4	46.8	*	14763.6	9339.5	5424.0	36.7	70.1	71.2																																
19	*119	125	31.06	1.05	56	*	25049.3	17033.1	8816.3	34.1	*	20267.1	15111.4	5155.7	25.4	48.8	52.1																																
20	*119	125	31.06	1.05	67	*	25120.9	14991.7	10129.2	40.3	*	19582.5	13429.6	6152.9	31.4	55.5	58.2																																
21	*119	125	31.06	1.05	78	*	24519.6	13454.1	10228.6	45.1	*	19020.4	12151.4	6869.0	36.1	60.2	62.3																																
22	*119	125	31.06	1.31	56	*	27664.0	17435.5	11065.5	45.1	*	20267.1	14596.3	5670.8	28.0	52.8	59.1																																
23	*119	125	31.06	1.31	67	*	26884.5	15345.9	11538.6	42.9	*	19582.5	12971.8	6610.7	33.8	59.3	64.3																																
24	*119	125	31.06	1.31	78	*	26240.9	13772.0	12468.9	47.5	*	19020.4	11737.2	7283.2	38.3	63.9	68.1																																
25	*119	125	31.06	1.57	56	*	29243.5	17771.9	11471.6	39.2	*	20267.1	14187.8	6079.3	30.0	56.4	64.9																																
26	*119	125	31.06	1.57	67	*	28419.4	15642.0	12777.5	45.0	*	19582.5	12608.8	6973.7	35.6	62.7	69.6																																
27	*119	125	31.06	1.57	78	*	27739.1	14037.7	13701.4	49.4	*	19020.4	11408.8	7611.6	40.0	67.1	72.5																																

NOTE: # IN = POUNDS ENTERING
 # OUT = POUNDS LEAVING
 # REM = POUNDS REMOVED
 % = PERCENTAGE REMOVED FOR THAT UNIT
 OVER, % REM = OVERALL EFFICIENCY COMPARING THE OUTPUT FOR
 THIS UNIT INPUT AT STATION # 2
 O.O.R. = MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED OPERATION
 MIN=15.70, MAX=31.06

COMPUTER SIMULATION OF TRICKLING FILTER OPERATION
ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 2 : SS =178 MG/L
BOD =125 MG/L

OUTPUT -- STREAM VECTORS

TRIAL #	*SS	BOD	FLOW	RECIRC	T	*FLOW	STATION # 2 BOD SS	STATION # 3 FLOW BOD SS	STATION # 4 FLOW BOD SS	STATION # 5 FLOW BOD SS	STATION # 7 FLOW BOD SS	STATION # 8 FLOW BOD SS	REMARK
1	*178	125	15.70	1.05	56	*14.95	127 182	0.75 66	79 15.70 125 178 15.67	76	92 14.93	66	79 14.91 45
2	*178	125	15.70	1.05	67	*14.95	128 183	0.75 58	72 15.70 125 178 15.67	74	89 14.92	58	72 14.91 41
3	*178	125	15.70	1.05	78	*14.95	128 183	0.75 52	65 15.70 125 178 15.67	72	86 14.92	52	65 14.91 37
4	*178	125	15.70	1.31	56	*11.98	142 209	3.72 68	76 15.70 125 178 15.67	82	92 11.96	68	76 11.95 42
5	*178	125	15.70	1.31	67	*11.98	145 211	3.72 60	70 15.70 125 178 15.67	79	89 11.96	60	70 11.95 38
6	*178	125	15.70	1.31	78	*11.98	147 213	3.72 53	63 15.70 125 178 15.67	77	86 11.96	53	63 11.95 34
7	*178	125	15.70	1.57	56	*10.00	156 236	5.70 69	76 15.70 125 178 15.67	86	92 9.97	69	76 9.96 36
8	*178	125	15.70	1.57	67	*10.00	161 240	5.70 61	68 15.70 125 178 15.67	84	89 9.97	61	68 9.96 32
9	*178	125	15.70	1.57	78	*10.00	165 244	5.70 54	61 15.70 125 178 15.67	82	86 9.97	54	61 9.96 32
10	*178	125	24.29	1.05	56	*23.13	127 183	1.16 74	75 24.29 125 178 24.25	93	93 23.09	74	75 23.08 49
11	*178	125	24.29	1.05	67	*23.13	128 183	1.16 65	67 24.29 125 178 24.25	90	90 23.09	65	67 23.08 45
12	*178	125	24.29	1.05	78	*23.13	128 183	1.16 58	60 24.29 125 178 24.25	88	88 23.09	58	60 23.08 45
13	*178	125	24.29	1.31	56	*18.54	140 210	5.75 75	72 24.29 125 178 24.25	100	93 18.50	75	72 18.49 45
14	*178	125	24.29	1.31	67	*18.54	143 213	5.75 66	64 24.29 125 178 24.25	97	90 18.50	66	64 18.49 41
15	*178	125	24.29	1.31	78	*18.54	145 215	5.75 60	58 24.29 125 178 24.25	94	88 18.50	60	58 18.49 41
16	*178	125	24.29	1.57	56	*15.47	152 239	8.82 77	70 24.29 125 178 24.25	105	93 15.43	77	70 15.42 47
17	*178	125	24.29	1.57	67	*15.47	157 243	8.82 68	63 24.29 125 178 24.25	102	90 15.43	68	63 15.42 42
18	*178	125	24.29	1.57	78	*15.47	161 246	8.82 61	57 24.29 125 178 24.25	100	88 15.43	61	57 15.42 39
19	*178	125	31.06	1.05	56	*29.58	127 183	1.48 78	71 31.06 125 178 31.01	101	94 29.53	78	71 29.52 60
20	*178	125	31.06	1.05	67	*29.58	127 183	1.48 69	64 31.06 125 178 31.01	99	91 29.53	69	64 29.52 54
21	*178	125	31.06	1.05	78	*29.58	128 183	1.48 62	58 31.06 125 178 31.01	111	88 29.53	62	58 29.52 50
22	*178	125	31.06	1.31	56	*23.71	138 209	7.35 80	72 31.06 125 178 31.01	108	94 23.66	80	72 23.65 56
23	*178	125	31.06	1.31	67	*23.71	141 213	7.35 71	61 31.06 125 178 31.01	106	91 23.66	71	61 23.65 50
24	*178	125	31.06	1.31	78	*23.71	144 215	7.35 63	56 31.06 125 178 31.01	106	88 23.65	63	56 23.64 45
25	*178	125	31.06	1.57	56	*19.78	150 236	11.28 80	71 31.06 125 178 31.01	118	94 19.73	80	71 19.72 52
26	*178	125	31.06	1.57	67	*19.78	155 244	11.28 72	60 31.06 125 178 31.01	114	91 19.73	72	60 19.72 47
27	*178	125	31.06	1.57	78	*19.78	159 248	11.28 65	54 31.06 125 178 31.01	112	88 19.73	65	54 19.72 43

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R.=MEANSFLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70,MAX=31.06

***** INPUT VECTORS ***** O U T P U T ***** P R I M A R Y T A N K C H A R A C T E R I S T I C S ***** S L U D G E R E M O V E D F O R T H A T U N I T A S S U M I N G 5.0% S O L I D S O . O . R . = M E A N S F L O W T H R U T H E P L A N T I S O U T O F T H E R A N G E O F T H E E S T A B L I S H E D P L A N T O P E R A T I O N M I N = 15.70, M A X = 31.06

TRIAL #	*SS	BOD	FLOW	RECIRC	T	# IN	B O D # OUT	# REM	%	* # IN	SUSPENDED # OUT	# REM	SOLIDS %	SLUDGE REMARK
1	*178	125	15.70	1.05	56	*16367.2	10040.9	6326.3	39.2	*23307.0	12112.6	11194.4	48.3	26845.
2	*178	125	15.70	1.05	67	*16367.2	9757.9	6609.3	40.8	*23307.0	11703.4	11603.5	50.0	27826.
3	*178	125	15.70	1.05	78	*16367.2	9524.4	6842.9	42.4	*23307.0	11367.5	11939.5	51.7	28632.
4	*178	125	15.70	1.31	56	*16367.2	10745.8	5621.4	34.4	*23307.0	12112.6	11194.4	48.3	26845.
5	*178	125	15.70	1.31	67	*16367.2	10443.0	5924.3	36.8	*23307.0	11703.4	11603.5	50.0	27826.
6	*178	125	15.70	1.31	78	*16367.2	10193.0	6174.2	38.4	*23307.0	11367.5	11939.5	51.7	28632.
7	*178	125	15.70	1.57	56	*16367.2	11359.3	5007.9	31.2	*23307.0	12112.6	11194.4	48.3	26845.
8	*178	125	15.70	1.57	67	*16367.2	11039.2	5328.0	32.0	*23307.0	11703.4	11603.5	50.0	27826.
9	*178	125	15.70	1.57	78	*16367.2	10775.0	5592.3	34.4	*23307.0	11367.5	11939.5	51.7	28632.
10	*178	125	24.29	1.05	56	*25322.3	18922.5	6399.8	25.6	*36059.0	18990.7	17068.3	47.8	40931.
11	*178	125	24.29	1.05	67	*25322.3	18389.3	6933.0	28.0	*36059.0	18349.2	18236.5	50.6	42469.
12	*178	125	24.29	1.05	78	*25322.3	17949.1	7373.2	29.6	*36059.0	18990.7	17068.3	47.8	40931.
13	*178	125	24.29	1.31	56	*25322.3	20250.9	5071.4	20.4	*36059.0	18349.2	17709.7	49.4	42469.
14	*178	125	24.29	1.31	67	*25322.3	19680.3	5642.0	22.4	*36059.0	17822.5	18236.5	50.6	43733.
15	*178	125	24.29	1.31	78	*25322.3	19209.2	6113.1	24.8	*36059.0	18990.7	17068.3	47.8	40931.
16	*178	125	24.29	1.57	56	*25322.3	21407.2	3915.1	16.0	*36059.0	18349.2	17709.7	49.4	42469.
17	*178	125	24.29	1.57	67	*25322.3	20803.9	4518.4	18.4	*36059.0	17822.5	18236.5	50.6	43733.
18	*178	125	24.29	1.57	78	*25322.3	20305.9	5016.4	20.0	*36059.0	18349.2	18236.5	50.6	43733.
19	*178	125	31.06	1.05	56	*32380.0	27040.9	5339.1	16.8	*46109.2	24466.4	21642.8	47.2	51901.
20	*178	125	31.06	1.05	67	*32380.0	26278.9	6101.1	19.2	*46109.2	23639.9	22469.2	48.9	53883.
21	*178	125	31.06	1.05	78	*32380.0	25649.9	6730.2	20.8	*46109.2	22961.3	23147.9	50.6	55510.
22	*178	125	31.06	1.31	56	*32380.0	28939.3	3440.8	11.2	*46109.2	24466.4	21642.8	47.2	51901.
23	*178	125	31.06	1.31	67	*32380.0	28123.8	4256.3	13.6	*46109.2	23639.9	22469.2	48.9	53883.
24	*178	125	31.06	1.31	78	*32380.0	27450.5	4929.5	15.2	*46109.2	22961.3	23147.9	50.6	55510.
25	*178	125	31.06	1.57	56	*32380.0	30591.5	1788.5	5.6	*46109.2	24466.4	21642.8	47.2	51901.
26	*178	125	31.06	1.57	67	*32380.0	29729.5	2650.6	8.8	*46109.2	23639.9	22469.2	48.9	53883.
27	*178	125	31.06	1.57	78	*32380.0	29017.8	3362.2	10.4	*46109.2	22961.3	23147.9	50.6	55510.

NOTE:

IN = POUNDS ENTERING
 # OUT = POUNDS LEAVING
 # REM = POUNDS REMOVED
 % = PERCENTAGE OF SLUDGE PRODUCED FOR THAT UNIT
 SLUDGE = GALLONS OF SLUDGE PRODUCED ASSUMING 5.0% SOLIDS
 O.O.R. = MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED PLANT OPERATION
 MIN=15.70, MAX=31.06

TRIAL #	SS	INPUT VECTORS		BOD		O U T P U T		T R I C K L I N G		F I L T E R		C H A R A C T E R I S T I C S	
		#	RECIRC	T	#	IN	#	OUT	BOD	#	IN	#	OUT
1	*178	125	15.70	1.05	56	*10040.9	8561.5	1479.4	14.7	*	12112.6	10535.2	1577.4
2	*178	125	15.70	1.05	67	*9757.9	7535.5	2222.4	22.8	*	11703.4	9362.8	2340.6
3	*178	125	15.70	1.05	78	*9524.4	6762.7	2761.7	29.0	*	11367.5	8471.7	2895.8
4	*178	125	15.70	1.31	56	*10745.8	8763.8	1982.0	18.4	*	12112.6	10176.1	1936.5
5	*178	125	15.70	1.31	67	*10443.0	7713.5	2729.5	26.1	*	11703.4	9043.6	2659.8
6	*178	125	15.70	1.31	78	*10193.0	6922.5	3270.5	32.1	*	11367.5	8182.9	3184.5
7	*178	125	15.70	1.57	56	*11359.3	8932.9	2426.5	21.4	*	12112.6	9891.3	2221.2
8	*178	125	15.70	1.57	67	*11039.2	7862.4	3176.9	28.8	*	11703.4	8790.6	2912.9
9	*178	125	15.70	1.57	78	*10775.0	7056.0	3718.9	34.5	*	11367.5	7953.9	3413.5
10	*178	125	24.29	1.05	56	*18922.5	14784.9	4137.7	21.9	*	18990.7	15028.7	3962.0
11	*178	125	24.29	1.05	67	*18389.3	13013.0	5376.3	29.2	*	18349.2	13356.1	4993.1
12	*178	125	24.29	1.05	78	*17949.1	11678.5	6270.6	34.9	*	17822.5	12085.0	5737.5
13	*178	125	24.29	1.31	56	*20250.9	15134.2	5116.8	25.3	*	18990.7	14516.4	4474.4
14	*178	125	24.29	1.31	67	*19680.3	13320.5	6359.8	32.3	*	18349.2	12900.8	5448.4
15	*178	125	24.29	1.31	78	*19209.2	11954.4	7254.8	37.8	*	17822.5	11673.1	6149.4
16	*178	125	24.29	1.57	56	*21407.2	15426.1	5981.0	27.9	*	18990.7	14110.1	4880.6
17	*178	125	24.29	1.57	67	*20803.9	13577.5	7226.4	34.7	*	18349.2	12539.8	5809.4
18	*178	125	24.29	1.57	78	*20305.9	12185.0	8120.9	40.0	*	17822.5	11346.4	6476.1
19	*178	125	31.06	1.05	56	*27040.9	20113.3	6927.6	25.6	*	24466.4	18358.3	6108.0
20	*178	125	31.06	1.05	67	*26278.9	17703.0	8575.9	32.6	*	23639.9	16315.3	7324.7
21	*178	125	31.06	1.05	78	*25649.9	15887.4	9762.4	38.1	*	22961.3	14762.5	8198.8
22	*178	125	31.06	1.31	56	*28939.3	20588.6	8350.7	28.9	*	24466.4	17732.5	6733.9
23	*178	125	31.06	1.31	67	*28123.8	18121.2	10002.5	35.6	*	23639.9	15759.1	7880.8
24	*178	125	31.06	1.31	78	*27450.5	16262.8	11187.8	40.8	*	22961.3	14259.3	8702.0
25	*178	125	31.06	1.57	56	*30591.5	20985.8	9605.8	31.4	*	24466.4	17236.3	7230.1
26	*178	125	31.06	1.57	67	*29729.5	18470.9	11258.6	37.9	*	23639.9	15310.1	8321.8
27	*178	125	31.06	1.57	78	*29017.8	16576.6	12441.2	42.9	*	22961.3	13860.3	9101.0
												OVERALL	800
													48.8
													55.5
													60.2
													52.0
													59.3
													64.6
													56.4
													62.7
													67.9
													59.6
													49.6
													55.5
													47.1
													54.5
													59.3
													50.0
													57.3
													62.7
													39.4
													61.7
													65.6
													31.0
													52.3
													42.8
													33.3
													50.4
													56.9
													46.0
													54.2
													39.6
													59.7

NOTE: # IN = POUNDS ENTERING
 # OUT = POUNDS LEAVING
 # REM = POUNDS REMOVED
 % = PERCENTAGE REMOVED FOR THAT UNIT
 OVER, % REM = OVERALL EFFICIENCY COMPARING THE OUTPUT FOR
 THIS UNIT INPUT AT STATION # 2
 O.O.R. = MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED OPERATION
 MIN=15.70, MAX=31.06

COMPUTER SIMULATION OF TRICKLING FILTER OPERATION ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 3 : SS =278 MG/L
BOD =125 MG/L

OUTPUT -- STREAM VECTORS *****

INPUT VECTORS *****

TRIAL #	*SS	BOD	FLOW	RECIRC	T	*FLOW	STATION # 2 BOD SS	STATION # 3 FLOW BOD SS	STATION # 4 FLOW BOD SS	STATION # 5 FLOW BOD SS	STATION # 7 FLOW BOD SS	STATION # 8 FLOW BOD SS	REMARK
1	*278	125	15.70	1.05	56	*14.95	127 287	0.75 79	94 15.70 125 278 15.65	80 114 14.90	79 94 14.89	53 66*	
2	*278	125	15.70	1.05	67	*14.95	127 287	0.75 70	88 15.70 125 278 15.65	78 110 14.90	70 88 14.89	48 61*	
3	*278	125	15.70	1.05	78	*14.95	128 287	0.75 63	79 15.70 125 278 15.65	76 107 14.90	79 14.89	44 55*	
4	*278	125	15.70	1.31	56	*11.98	138 340	3.72 80	86 15.70 125 278 15.65	86 114 11.93	86 11.92	47 57*	
5	*278	125	15.70	1.31	67	*11.98	141 338	3.72 72	85 15.70 125 278 15.65	84 110 11.93	85 11.92	45 56*	
6	*278	125	15.70	1.31	78	*11.98	143 340	3.72 64	76 15.70 125 278 15.65	82 107 11.93	76 11.92	41 50*	
7	*278	125	15.70	1.57	56	*10.00	150 388	5.70 81	89 15.70 125 278 15.65	91 114 9.95	89 9.94	45 56*	
8	*278	125	15.70	1.57	67	*10.00	154 389	5.70 73	82 15.70 125 278 15.65	88 110 9.95	82 9.94	42 51*	
9	*278	125	15.70	1.57	78	*10.00	158 393	5.70 66	76 15.70 125 278 15.65	86 107 9.95	76 9.94	38 47*	
10	*278	125	24.29	1.05	56	*23.13	126 287	1.16 87	91 24.29 125 278 24.21	98 115 23.05	91 23.04	63 71*	
11	*278	125	24.29	1.05	67	*23.13	127 287	1.16 78	80 24.29 125 278 24.21	95 111 23.05	80 23.04	57 62*	
12	*278	125	24.29	1.05	78	*23.13	127 288	1.16 70	75 24.29 125 278 24.21	93 108 23.05	75 23.04	53 58*	
13	*278	125	24.29	1.31	56	*18.54	136 336	5.75 89	88 24.29 125 278 24.21	105 115 18.46	88 18.45	58 65*	
14	*278	125	24.29	1.31	67	*18.54	138 340	5.75 80	77 24.29 125 278 24.21	102 111 18.46	77 18.45	53 56*	
15	*278	125	24.29	1.31	78	*18.54	141 341	5.75 72	72 24.29 125 278 24.21	100 108 18.46	72 18.45	49 52*	
16	*278	125	24.29	1.57	56	*15.47	144 388	8.82 90	85 24.29 125 278 24.21	111 115 15.39	85 15.38	54 60*	
17	*278	125	24.29	1.57	67	*15.47	150 393	8.82 80	76 24.29 125 278 24.21	108 111 15.39	76 15.38	49 53*	
18	*278	125	24.29	1.57	78	*15.47	154 396	8.82 73	70 24.29 125 278 24.21	105 108 15.39	70 15.38	46 49*	
19	*278	125	31.06	1.05	56	*29.58	126 287	1.48 92	87 31.06 125 278 30.96	116 116 29.48	87 29.47	70 72*	
20	*278	125	31.06	1.05	67	*29.58	127 288	1.48 81	77 31.06 125 278 30.96	106 112 29.48	77 29.47	63 63*	
21	*278	125	31.06	1.05	78	*29.58	127 288	1.48 75	71 31.06 125 278 30.96	104 109 29.48	75 29.47	59 58*	
22	*278	125	31.06	1.31	56	*23.71	134 338	7.35 94	84 31.06 125 278 30.96	117 116 23.61	84 23.60	64 66*	
23	*278	125	31.06	1.31	67	*23.71	138 340	7.35 83	76 31.06 125 278 30.96	114 112 23.61	76 23.59	58 59*	
24	*278	125	31.06	1.31	78	*23.71	140 342	7.35 76	69 31.06 125 278 30.96	111 109 23.61	69 23.59	54 53*	
25	*278	125	31.06	1.57	56	*19.78	141 390	11.28 96	81 31.06 125 278 30.96	124 116 19.68	81 19.67	60 60*	
26	*278	125	31.06	1.57	67	*19.78	147 394	11.28 85	74 31.06 125 278 30.96	121 112 19.68	74 19.67	55 55*	
27	*278	125	31.06	1.57	78	*19.78	151 395	11.28 78	68 31.06 125 278 30.96	118 109 19.68	68 19.67	51 50*	

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R. MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70, MAX=31.06

***** INPUT VECTORS ***** OUTPUT PRIMARY TANK CHARACTERISTICS *****

TRIAL#	#SS	BOD	FLOW	RECIRC	T	#	IN	#	OUT	BOD	#	REM	%	#	IN	SUSPENDED	#	REM	SOLIDS	%	SLUDGE	REMARK
1	*278	125	15.70	1.05	56	*	16367.2		10554.6	5812.6		36.0		*	36400.8	14920.5	21480.3	59.0	51511.			
2	*278	125	15.70	1.05	67	*	16367.2		10257.2	6110.0		37.6		*	36400.8	14416.5	21984.2	60.4	52720.			
3	*278	125	15.70	1.05	70	*	16367.2		10011.7	6355.6		39.2		*	36400.8	14002.7	22398.1	61.5	53712.			
4	*278	125	15.70	1.31	56	*	16367.2		11295.6	5071.6		31.2		*	36400.8	14920.5	21480.3	59.0	51511.			
5	*278	125	15.70	1.31	67	*	16367.2		10977.3	5389.9		32.8		*	36400.8	14416.5	21984.2	60.4	52720.			
6	*278	125	15.70	1.31	78	*	16367.2		10714.5	5652.7		34.4		*	36400.8	14002.7	22398.1	61.5	53712.			
7	*278	125	15.70	1.57	56	*	16367.2		11940.5	4426.7		27.2		*	36400.8	14920.5	21480.3	59.0	51511.			
8	*278	125	15.70	1.57	67	*	16367.2		11604.0	4763.2		29.6		*	36400.8	14416.5	21984.2	60.4	52720.			
9	*278	125	15.70	1.57	78	*	16367.2		11326.3	5041.0		31.2		*	36400.8	14002.7	22398.1	61.5	53712.			
10	*278	125	24.29	1.05	56	*	25322.3		19890.7	5431.6		21.6		*	56316.8	23393.1	32923.7	58.6	78954.			
11	*278	125	24.29	1.05	67	*	25322.3		19330.2	5992.1		24.0		*	56316.8	22603.0	33713.9	60.1	80849.			
12	*278	125	24.29	1.05	78	*	25322.3		18867.5	6454.9		25.6		*	56316.8	21954.1	34362.7	61.2	82405.			
13	*278	125	24.29	1.31	56	*	25322.3		21287.1	4035.2		16.0		*	56316.8	23393.1	32923.7	58.6	78954.			
14	*278	125	24.29	1.31	67	*	25322.3		20687.2	4635.1		18.4		*	56316.8	22603.0	33713.9	60.1	80849.			
15	*278	125	24.29	1.31	78	*	25322.3		20192.0	5130.3		20.0		*	56316.8	21954.1	34362.7	61.2	82405.			
16	*278	125	24.29	1.57	56	*	25322.3		22502.4	2819.9		11.2		*	56316.8	23393.1	32923.7	58.6	78954.			
17	*278	125	24.29	1.57	67	*	25322.3		21868.3	3454.0		13.6		*	56316.8	22603.0	33713.9	60.1	80849.			
18	*278	125	24.29	1.57	78	*	25322.3		21344.9	3977.4		16.0		*	56316.8	21954.1	34362.7	61.2	82405.			
19	*278	125	31.06	1.05	56	*	32380.0		20424.4	3955.6		12.0		*	72013.2	30138.1	41875.1	58.3	100420.			
20	*278	125	31.06	1.05	67	*	32380.0		2623.4	4756.6		15.2		*	72013.2	29120.1	42893.1	59.7	102861.			
21	*278	125	31.06	1.05	78	*	32380.0		26962.2	5417.8		16.8		*	72013.2	28284.2	43729.0	60.8	104866.			
22	*278	125	31.06	1.31	56	*	32380.0		30419.9	1960.1		6.4		*	72013.2	30138.1	41875.1	58.3	100420.			
23	*278	125	31.06	1.31	67	*	32380.0		29562.7	2817.4		8.8		*	72013.2	29120.1	42893.1	59.7	102861.			
24	*278	125	31.06	1.31	78	*	32380.0		28855.0	3525.0		11.2		*	72013.2	28284.2	43729.0	60.8	104866.			
25	*278	125	31.06	1.57	56	*	32380.0		32156.7	223.3		0.8		*	72013.2	30138.1	41875.1	58.3	100420.			
26	*278	125	31.06	1.57	67	*	32380.0		31250.6	1129.5		3.2		*	72013.2	29120.1	42893.1	59.7	102861.			
27	*278	125	31.06	1.57	78	*	32380.0		30502.5	1877.5		5.6		*	72013.2	28284.2	43729.0	60.8	104866.			

NOTE: # IN =POUNDS ENTERING
 # OUT =POUNDS LEAVING
 # REM =POUNDS REMOVED
 % =PERCENTAGE REMOVED FOR THAT UNIT
 SLUDGE=GALLONS OF SLUDGE PRODUCED
 O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED PLANT OPERATION
 MIN=15.70,MAX=31.06

INPUT VECTORS

TRIAL#	*SS	BOD	FLOW	RECIRC	T	* IN	* OUT	# REM	%	* IN	* OUT	SUSPENDED	# REM	%	OVERALL % REM	REMARK
1	*278	125	15.70	1.05	56	* 10554.6	10296.7	257.9	2.4	* 14920.5	13074.3	1846.2	12.4	38.6	65.2	
2	*278	125	15.70	1.05	67	* 10257.2	9062.8	1194.4	11.6	* 14416.5	11619.4	2797.1	19.4	45.7	69.0	
3	*278	125	15.70	1.05	78	* 10011.7	8133.5	1878.2	18.8	* 14002.7	10513.7	3489.0	24.9	51.6	72.1	
4	*278	125	15.70	1.31	56	* 11295.6	10539.9	755.6	6.7	* 14920.5	12628.6	2291.9	15.4	42.0	71.8	
5	*278	125	15.70	1.31	67	* 10977.3	9276.9	1700.4	15.5	* 14416.5	11233.3	3193.2	22.1	49.6	74.6	
6	*278	125	15.70	1.31	78	* 10714.5	8325.6	2388.9	22.3	* 14002.7	10155.3	3847.4	27.5	55.9	77.4	
7	*278	125	15.70	1.57	56	* 11940.5	10743.3	1197.2	10.0	* 14920.5	12275.2	2645.3	17.7	45.3	75.8	
8	*278	125	15.70	1.57	67	* 11604.0	9455.9	2140.1	18.5	* 14416.5	10989.2	3507.3	24.3	53.2	78.7	
9	*278	125	15.70	1.57	78	* 11326.3	8486.3	2840.0	25.1	* 14002.7	9871.1	4131.6	29.5	58.9	80.9	
10	*278	125	24.29	1.05	56	* 19890.7	17781.2	2109.4	10.6	* 23393.1	18650.6	4742.5	20.3	30.2	67.9	
11	*278	125	24.29	1.05	67	* 19330.2	15650.5	3679.7	19.0	* 22603.0	16575.1	6027.8	26.7	39.4	71.4	
12	*278	125	24.29	1.05	78	* 18867.5	14045.6	4821.8	25.6	* 21954.1	14997.8	6956.3	31.7	45.7	74.3	
13	*278	125	24.29	1.31	56	* 21287.1	18201.4	3085.7	14.5	* 23393.1	18014.9	5378.3	23.0	33.8	73.5	
14	*278	125	24.29	1.31	67	* 20687.2	16020.2	4667.0	22.6	* 22603.0	16010.1	6592.8	29.2	42.8	76.8	
15	*278	125	24.29	1.31	78	* 20192.0	14377.4	5814.6	28.8	* 21954.1	14486.6	7467.6	34.0	49.6	79.2	
16	*278	125	24.29	1.57	56	* 22502.4	18552.6	3949.9	17.6	* 23393.1	17510.7	5882.4	25.1	36.8	77.8	
17	*278	125	24.29	1.57	67	* 21868.3	16329.3	5539.0	25.3	* 22603.0	15562.1	7040.9	31.2	46.7	80.4	
18	*278	125	24.29	1.57	78	* 21344.9	14654.8	6690.8	31.3	* 21954.1	14081.2	7872.9	35.9	53.2	82.6	
19	*278	125	31.06	1.05	56	* 28424.4	24189.6	4234.9	14.9	* 30138.1	22782.6	7355.5	24.4	26.2	69.3	
20	*278	125	31.06	1.05	67	* 27623.4	21290.9	6332.5	22.9	* 29120.1	20247.4	8872.7	30.5	35.4	72.9	
21	*278	125	31.06	1.05	78	* 26962.2	19107.6	7854.6	29.1	* 28284.2	18320.6	9963.6	35.2	41.7	75.7	
22	*278	125	31.06	1.31	56	* 30419.9	24761.1	5658.9	18.6	* 30138.1	22006.0	8132.1	27.0	29.1	74.9	
23	*278	125	31.06	1.31	67	* 29562.7	21793.9	7768.8	26.3	* 29120.1	19557.2	9562.9	32.8	39.1	77.9	
24	*278	125	31.06	1.31	78	* 28855.0	19559.1	9296.0	32.2	* 28284.2	17696.1	10588.1	37.4	46.4	80.1	
25	*278	125	31.06	1.57	56	* 32156.7	25238.8	6917.9	21.5	* 30138.1	21390.2	8747.9	29.0	31.2	79.0	
26	*278	125	31.06	1.57	67	* 31250.6	22214.5	9036.1	28.9	* 29120.1	19009.9	10110.2	34.7	41.5	81.5	
27	*278	125	31.06	1.57	78	* 30502.5	19336.5	10566.0	34.6	* 28284.2	17200.9	11083.3	39.2	49.0	83.3	

NOTE: # IN =POUNDS ENTERING
 # OUT =POUNDS LEAVING
 # REM =POUNDS REMOVED
 % =PERCENTAGE REMOVED FOR THAT UNIT
 OVER.% REM=OVERALL EFFICIENCY COMPARING THE OUTPUT FOR
 THIS UNIT INPUT AT STATION # 2
 O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED OPERATION
 MIN=15.70, MAX=31.06

TRIAL#	#	*SS	INPUT VECTORS			O U T P U T			SECONDARY TANK			C H A R A C T E R I S T I C S			REMARK			
			BOD	FLOW	RECIRC	T	#	IN	#	OUT	B O D	#	REM	SUSPENDED		SOLIDS	PLANT EFF	
***** B O D *****																		
1	*	278	125	15.70	1.05	56	*9817.6	6597.1	3220.5	32.8	*11681.7	8218.2	3463.5	29.6	12978.	58.3	77.0	
2	*	278	125	15.70	1.05	67	*8698.4	6082.0	2616.4	30.1	*10935.2	7669.8	3265.4	29.9	12236.	62.2	78.7	
3	*	278	125	15.70	1.05	78	*7828.1	5553.6	2274.4	29.1	*9816.1	6851.0	2965.1	30.2	11110.	65.6	80.8	
4	*	278	125	15.70	1.31	56	*7961.8	4740.8	3221.0	40.5	*8559.0	5670.1	2888.8	33.8	10824.	65.9	83.2	
5	*	278	125	15.70	1.31	67	*7164.9	4497.7	2667.2	37.2	*8458.6	5600.5	2858.1	33.8	10709.	68.1	83.4	
6	*	278	125	15.70	1.31	78	*6368.3	4077.2	2291.1	36.0	*7562.4	4981.5	2580.8	34.1	9670.	71.3	85.3	
7	*	278	125	15.70	1.57	56	*6720.6	3778.0	2942.6	43.8	*7384.4	4679.8	2704.6	36.6	10134.	70.0	85.6	
8	*	278	125	15.70	1.57	67	*6056.1	3487.5	2568.6	42.4	*6802.7	4295.0	2507.7	36.9	9396.	72.7	86.9	
9	*	278	125	15.70	1.57	78	*5474.8	3231.7	2243.1	41.0	*6304.4	3966.5	2337.8	37.1	8760.	75.9	88.0	
10	*	278	125	24.29	1.05	56	*16727.8	12286.8	4441.0	26.5	*17496.9	13722.0	3774.9	21.6	14145.	50.0	75.3	
11	*	278	125	24.29	1.05	67	*14996.1	11106.7	3889.4	25.9	*15380.6	11991.5	3389.1	22.0	12699.	55.1	78.4	
12	*	278	125	24.29	1.05	78	*13457.1	10304.9	3152.2	23.4	*14418.3	11208.1	3210.2	22.3	12029.	58.3	79.9	
13	*	278	125	24.29	1.31	56	*13704.3	9068.3	4636.0	33.8	*13550.4	10032.4	3517.9	26.0	13182.	57.4	80.7	
14	*	278	125	24.29	1.31	67	*12317.3	8191.4	4125.8	33.5	*11855.4	8724.1	3131.3	26.4	11733.	61.6	83.5	
15	*	278	125	24.29	1.31	78	*11084.6	7601.9	3482.7	31.4	*11084.6	8131.8	2952.8	26.6	11064.	65.2	84.8	
16	*	278	125	24.29	1.57	56	*11553.5	7027.0	4526.6	39.2	*10911.6	7704.2	3207.5	29.4	12018.	62.5	84.5	
17	*	278	125	24.29	1.57	67	*10268.5	6369.7	3898.8	38.0	*9755.1	6852.4	2902.7	29.8	10877.	67.3	86.5	
18	*	278	125	24.29	1.57	78	*9369.1	5912.7	3456.4	36.9	*8984.0	6287.0	2697.0	30.0	10106.	70.1	87.6	
19	*	278	125	31.06	1.05	56	*22619.8	17269.9	5349.9	23.7	*21390.5	17812.9	3577.6	16.7	13405.	44.4	74.9	
20	*	278	125	31.06	1.05	67	*19913.6	15524.6	4389.0	22.0	*18930.2	15676.4	3253.8	17.2	12192.	50.4	78.1	
21	*	278	125	31.06	1.05	78	*18437.3	14513.7	3923.6	21.3	*17454.0	14400.3	3053.6	17.5	11442.	53.5	79.9	
22	*	278	125	31.06	1.31	56	*18508.9	12731.2	5777.7	31.2	*16539.9	13002.1	3537.8	21.4	13256.	52.2	80.5	
23	*	278	125	31.06	1.31	67	*16341.3	11560.1	4781.2	29.3	*14963.1	11708.8	3254.3	21.7	12194.	58.0	82.6	
24	*	278	125	31.06	1.31	78	*14961.8	10686.4	4275.5	28.6	*13583.8	10582.6	3001.2	22.1	11245.	61.4	84.5	
25	*	278	125	31.06	1.57	56	*15759.0	9903.1	5855.9	37.2	*13296.6	9967.2	3329.5	25.0	12076.	57.4	84.6	
26	*	278	125	31.06	1.57	67	*13951.5	9037.9	4913.6	35.2	*12146.1	9067.0	3079.1	25.4	11537.	62.6	86.0	
27	*	278	125	31.06	1.57	78	*12801.3	8403.2	4398.1	34.4	*11160.1	8298.8	2861.3	25.6	10721.	66.2	87.3	

NOTE: # IN = POUNDS ENTERING
 # OUT = POUNDS LEAVING
 # REM = POUNDS REMOVED
 % = PERCENTAGE REMOVED FOR THAT UNIT
 SLUDGE = GALLONS OF SLUDGE PRODUCED ASSUMING 3.2% SOLIDS
 PLANT EFF = OVERALL PLANT EFFICIENCY
 O.O.R. = MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED OPERATION
 MIN=15.70, MAX=31.06

COMPUTER SIMULATION OF TRICKLING FILTER OPERATION
ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 4 : SS =119 MG/L
BOD =169 MG/L

O U T P U T " " S T R E A M V E C T O R S

INPUT VECTORS

TRIAL#	*SS	BOD	FLOW	RECIRC	* T	* FLOW	STATION # 2	STATION # 3	STATION # 4	STATION # 5	STATION # 7	STATION # 8	REMARK
#							FLOW BOD SS	FLOW BOD SS	FLOW BOD SS	FLOW BOD SS	FLOW BOD SS	FLOW BOD SS	
1	*119	169	15.70	1.05	56	*14.95	174 121	0.75 67	71 15.70 169	119 15.69 88	76 14.94 67	71 14.93 44	49*
2	*119	169	15.70	1.05	67	*14.95	174 121	0.75 59	63 15.70 169	119 15.69 86	74 14.94 59	63 14.93 39	43*
3	*119	169	15.70	1.05	78	*14.95	174 122	0.75 53	57 15.70 169	119 15.69 84	71 14.94 53	57 14.93 36	39*
4	*119	169	15.70	1.31	56	*11.98	200 134	3.72 68	69 15.70 169	119 15.69 94	76 11.97 68	69 11.96 40	45*
5	*119	169	15.70	1.31	67	*11.98	202 136	3.72 60	61 15.70 169	119 15.69 92	74 11.97 60	61 11.96 36	39*
6	*119	169	15.70	1.31	78	*11.98	204 138	3.72 54	55 15.70 169	119 15.69 89	71 11.97 54	55 11.96 33	35*
7	*119	169	15.70	1.57	56	*10.00	225 152	5.70 70	67 15.70 169	119 15.69 100	76 9.99 70	67 9.98 38	41*
8	*119	169	15.70	1.57	67	*10.00	230 156	5.70 61	60 15.70 169	119 15.69 97	74 9.99 61	60 9.98 34	37*
9	*119	169	15.70	1.57	78	*10.00	233 156	5.70 55	54 15.70 169	119 15.69 95	71 9.99 55	54 9.98 31	33*
10	*119	169	24.29	1.05	56	*23.13	173 121	1.16 74	66 24.29 169	119 24.27 107	77 23.11 74	66 23.10 52	51*
11	*119	169	24.29	1.05	67	*23.13	174 121	1.16 65	59 24.29 169	119 24.27 104	75 23.11 65	59 23.10 47	45*
12	*119	169	24.29	1.05	78	*23.13	174 122	1.16 59	53 24.29 169	119 24.27 102	72 23.11 59	53 23.10 43	40*
13	*119	169	24.29	1.31	56	*18.54	197 135	5.75 76	64 24.29 169	119 24.27 115	77 18.52 76	64 18.51 48	46*
14	*119	169	24.29	1.31	67	*18.54	200 137	5.75 67	57 24.29 169	119 24.27 112	75 18.52 67	57 18.51 43	41*
15	*119	169	24.29	1.57	78	*18.54	202 140	5.75 60	51 24.29 169	119 24.27 109	72 18.52 60	51 18.51 39	36*
16	*119	169	24.29	1.57	56	*15.47	220 148	8.82 78	65 24.29 169	119 24.27 118	75 15.45 68	64 15.44 45	44*
17	*119	169	24.29	1.57	67	*15.47	226 154	8.82 68	55 24.29 169	119 24.27 115	72 15.45 61	55 15.44 40	38*
18	*119	169	24.29	1.57	78	*15.47	230 158	8.82 61	50 24.29 169	119 24.27 115	72 15.45 61	50 15.44 37	34*
19	*119	169	31.06	1.05	56	*29.58	173 121	1.48 79	65 31.06 169	119 31.03 120	78 29.56 79	65 29.54 58	53*
20	*119	169	31.06	1.05	67	*29.58	173 122	1.48 70	56 31.06 169	119 31.03 117	75 29.55 70	56 29.54 52	45*
21	*119	169	31.06	1.05	78	*29.58	174 122	1.48 63	51 31.06 169	119 31.03 114	73 29.55 63	51 29.54 47	41*
22	*119	169	31.06	1.31	56	*23.71	196 132	7.35 80	68 31.06 169	119 31.03 129	78 23.68 80	68 23.67 55	53*
23	*119	169	31.06	1.31	67	*23.71	199 138	7.35 71	54 31.06 169	119 31.03 125	75 23.68 71	54 23.67 47	41*
24	*119	169	31.06	1.31	78	*23.71	201 140	7.35 64	49 31.06 169	119 31.03 122	73 23.68 64	49 23.67 43	37*
25	*119	169	31.06	1.57	56	*19.78	219 143	11.28 80	67 31.06 169	119 31.03 136	78 19.76 80	67 19.75 51	49*
26	*119	169	31.06	1.57	67	*19.78	223 155	11.28 73	53 31.06 169	119 31.03 132	75 19.76 73	53 19.75 45	39*
27	*119	169	31.06	1.57	78	*19.78	228 159	11.28 65	48 31.06 169	119 31.03 129	73 19.76 65	48 19.75 41	35*

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R.=MEANSFLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70,MAX=31.06

PRIMARY TANK CHARACTERISTICS

O U T P U T										*****											
INPUT VECTORS										*****											
TRIAL*	#	SS	HOD	FLOW	RECIRC	T	#	IN	#	B O D	#	REM	%	#	IN	SUSPENDED	#	REM	%	SLUDGE	REMARK
1	119	169	15.70	1.05	56	22128.5	11590.0	10538.5	47.9	15581.6	10033.7	5548.0	36.1	13304.0							
2	119	169	15.70	1.05	67	22128.5	11263.4	10865.1	49.1	15581.6	9694.7	5886.9	37.8	14117.0							
3	119	169	15.70	1.05	78	22128.5	10993.8	11134.8	50.3	15581.6	9416.4	6165.2	40.3	14785.0							
4	119	169	15.70	1.31	56	22128.5	12403.6	9724.9	44.4	15581.6	10033.7	5548.0	36.1	13304.0							
5	119	169	15.70	1.31	67	22128.5	12054.1	10074.4	45.6	15581.6	9694.7	5886.9	37.8	14117.0							
6	119	169	15.70	1.31	78	22128.5	11765.6	10363.0	47.3	15581.6	9416.4	6165.2	40.3	14785.0							
7	119	169	15.70	1.57	56	22128.5	13111.8	9016.7	40.8	15581.6	10033.7	5548.0	36.1	13304.0							
8	119	169	15.70	1.57	67	22128.5	12742.3	9386.2	42.6	15581.6	9694.7	5886.9	37.8	14117.0							
9	119	169	15.70	1.57	78	22128.5	12437.3	9691.2	43.8	15581.6	9416.4	6165.2	40.3	14785.0							
10	119	169	24.29	1.05	56	34235.8	21841.9	12393.9	36.7	24106.8	15731.3	8375.6	35.3	20885.0							
11	119	169	24.29	1.05	67	34235.8	21226.4	13009.4	38.5	24106.8	14763.6	8906.9	37.0	21360.0							
12	119	169	24.29	1.05	78	34235.8	20718.3	13517.5	39.6	24106.8	15199.9	9343.3	39.5	22406.0							
13	119	169	24.29	1.31	56	34235.8	23375.2	10860.5	32.0	24106.8	15731.3	8375.6	35.3	20885.0							
14	119	169	24.29	1.31	67	34235.8	22716.5	11519.2	33.7	24106.8	14763.6	8906.9	37.0	21360.0							
15	119	169	24.29	1.31	78	34235.8	22172.7	12063.0	35.5	24106.8	15731.3	8375.6	35.3	20885.0							
16	119	169	24.29	1.57	56	34235.8	24709.9	9525.9	27.8	24106.8	15199.9	8906.9	37.0	21360.0							
17	119	169	24.29	1.57	67	34235.8	24013.5	10222.2	30.2	24106.8	14763.6	8375.6	35.3	20885.0							
18	119	169	24.29	1.57	78	34235.8	23438.7	10797.1	32.0	24106.8	15199.9	8906.9	37.0	21360.0							
19	119	169	31.06	1.05	56	43777.8	31212.7	12565.1	29.0	30825.8	20267.1	10558.7	34.5	25321.0							
20	119	169	31.06	1.05	67	43777.8	30333.2	13444.6	30.8	30825.8	19582.5	11243.3	37.0	26962.0							
21	119	169	31.06	1.05	78	43777.8	29607.1	14170.7	32.5	30825.8	19020.4	11805.4	38.7	28310.0							
22	119	169	31.06	1.31	56	43777.8	33404.0	10373.8	23.7	30825.8	20267.1	10558.7	34.5	25321.0							
23	119	169	31.06	1.31	67	43777.8	32462.7	11315.1	26.0	30825.8	19582.5	11243.3	37.0	26962.0							
24	119	169	31.06	1.31	78	43777.8	31685.6	12092.2	27.8	30825.8	19020.4	11805.4	38.7	28310.0							
25	119	169	31.06	1.57	56	43777.8	35311.2	8466.7	19.5	30825.8	20267.1	10558.7	34.5	25321.0							
26	119	169	31.06	1.57	67	43777.8	34316.1	9461.7	21.9	30825.8	19582.5	11243.3	37.0	26962.0							
27	119	169	31.06	1.57	78	43777.8	33494.7	10283.1	23.7	30825.8	19020.4	11805.4	38.7	28310.0							

NOTE: # IN = POUNDS ENTERING
 # OUT = POUNDS LEAVING
 # REM = POUNDS REMOVED
 % = PERCENTAGE OF SLUDGE PRODUCED FOR THAT UNIT
 SLUDGE=GALLONS OF SLUDGE PRODUCED ASSUMING 5.0% SOLIDS
 O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED PLANT OPERATION
 MIN=15.70, MAX=31.06

TRIAL# INPUT VECTORS OUTPUT TRICKLING FILTER CHARACTERISTICS

TRIAL*		BOD		RECIRC		FLOW		SOLIDS		SUSPENDED		OVERALL		REMARK						
#	*SS	BOD	FLOW	RECIRC	T	#	IN	#	OUT	#	REM	%	#	IN	#	OUT	%	BOD	%	REMARK
1	*119	169	15.70	1.05	56	*	11590.0	8653.7	2936.3	25.3	*	10033.7	9261.2	772.5	7.7	62.1	9.9	42.1		
2	*119	169	15.70	1.05	67	*	11263.4	7616.5	3646.9	32.4	*	9694.7	8230.4	1464.3	15.1	66.7	48.8			
3	*119	169	15.70	1.05	78	*	10993.8	6835.4	4158.4	37.8	*	9416.4	7447.1	1969.3	20.9	70.1	54.1			
4	*119	169	15.70	1.31	50	*	12403.6	8858.1	3545.5	28.6	*	10033.7	8945.5	1088.2	10.8	66.5	49.3			
5	*119	169	15.70	1.31	67	*	12054.1	7796.5	4257.6	35.3	*	9694.7	7949.9	1744.9	18.0	70.8	55.9			
6	*119	169	15.70	1.31	78	*	11765.6	6996.9	4768.7	40.5	*	9416.4	7193.2	2223.2	23.6	74.0	60.9			
7	*119	169	15.70	1.57	56	*	13111.8	9029.0	4082.8	31.1	*	10033.7	8695.1	1338.5	13.3	69.3	55.4			
8	*119	169	15.70	1.57	67	*	12742.3	7946.9	4795.4	37.6	*	9694.7	7727.4	1967.3	20.3	73.9	61.2			
9	*119	169	15.70	1.57	78	*	12437.3	7131.9	5305.5	42.7	*	9416.4	6991.9	2424.5	25.7	76.8	66.0			
10	*119	169	24.29	1.05	56	*	21841.9	14944.0	6897.9	31.6	*	15731.3	13211.2	2520.1	16.0	57.8	46.3			
11	*119	169	24.29	1.05	67	*	21226.4	13153.0	8073.4	38.0	*	15199.9	11740.8	3459.1	22.8	63.2	52.1			
12	*119	169	24.29	1.05	78	*	20718.3	11804.0	8914.2	43.0	*	14763.6	10623.4	4140.2	28.0	66.7	57.4			
13	*119	169	24.29	1.31	56	*	23375.2	15297.1	8078.1	34.6	*	15731.3	12760.9	2970.4	18.9	61.9	53.3			
14	*119	169	24.29	1.31	67	*	22716.5	13463.8	9252.8	40.7	*	15199.9	11340.6	3859.3	25.4	67.0	59.1			
15	*119	169	24.29	1.31	78	*	22172.7	12082.9	10089.8	45.5	*	14763.6	10261.3	4502.3	30.5	70.8	64.3			
16	*119	169	24.29	1.57	56	*	24709.9	15592.2	9117.6	36.9	*	15731.3	12403.8	3327.5	21.2	65.0	58.8			
17	*119	169	24.29	1.57	67	*	24013.5	13723.5	10290.0	42.9	*	15199.9	11023.3	4176.6	27.5	70.4	64.9			
18	*119	169	24.29	1.57	78	*	23438.7	12316.0	11122.7	47.5	*	14763.6	9974.1	4789.4	32.4	73.9	69.0			
19	*119	169	31.06	1.05	56	*	31212.7	20329.9	10882.8	34.9	*	20267.1	16138.2	4128.9	20.4	54.9	48.8			
20	*119	169	31.06	1.05	67	*	30333.2	17893.4	12439.8	41.0	*	19582.5	14342.1	5240.4	26.8	60.1	54.9			
21	*119	169	31.06	1.05	78	*	29607.1	16058.3	13548.8	45.8	*	19020.4	12977.1	6043.3	31.1	64.4	59.0			
22	*119	169	31.06	1.31	56	*	32462.7	20810.2	12593.7	37.7	*	20267.1	15588.1	4679.0	23.1	59.2	54.5			
23	*119	169	31.06	1.31	67	*	32462.7	18316.2	14146.5	43.6	*	19582.5	13853.2	5729.3	29.3	64.8	61.6			
24	*119	169	31.06	1.31	78	*	31685.6	16437.7	15247.9	48.1	*	19020.4	12534.7	6485.6	34.1	68.7	65.7			
25	*119	169	31.06	1.57	56	*	35311.2	21211.8	14099.4	39.9	*	20267.1	15151.9	5115.2	25.2	63.0	59.4			
26	*119	169	31.06	1.57	67	*	34316.1	18669.6	15646.5	45.6	*	19582.5	13465.6	6117.0	31.2	67.7	66.5			
27	*119	169	31.06	1.57	78	*	33494.7	16754.8	16739.9	50.0	*	19020.4	12184.0	6836.4	35.9	71.9	70.4			

NOTE: # IN = POUNDS ENTERING
OUT = POUNDS LEAVING
REM = POUNDS REMOVED
% OVER = PERCENTAGE REMOVED FOR THAT UNIT
% REM = OVERALL EFFICIENCY COMPARING THE OUTPUT FOR THIS UNIT INPUT AT STATION # 2
O.O.R. = MEANS FLOW THRU THE PLANT IS OUT OF THE RANGE OF THE ESTABLISHED OPERATION
MIN=15.70, MAX=31.06

INPUT VECTORS

TRIAL*	SS	BOD	FLOW	RECIRC	T	#	IN	#	OUT	BOD	#	REM	%	#	IN	#	OUT	SUSPENDED	#	REM	%	SLUDGE	PLANT EFF	REMARK
1	*119	169	15.70	1.05	56	*8347.7	5493.7	2853.9	34.2	*8846.0	6148.2	2697.8	30.5	10109.	74.7	59.5								
2	*119	169	15.70	1.05	67	*7350.5	4944.1	2406.4	32.7	*7848.9	5425.5	2423.4	30.9	9080.	77.6	64.5								
3	*119	169	15.70	1.05	78	*6602.7	4524.9	2077.8	31.5	*7101.0	4886.2	2214.9	31.2	8299.	79.3	68.0								
4	*119	169	15.70	1.31	56	*6789.2	4051.5	2737.7	40.3	*6889.1	4522.0	2367.1	34.4	8870.	80.0	66.4								
5	*119	169	15.70	1.31	67	*5990.1	3644.1	2346.0	39.2	*6089.9	3975.0	2114.9	34.7	7925.	82.2	71.3								
6	*119	169	15.70	1.31	78	*5390.8	3333.3	2057.5	38.2	*5490.6	3566.9	1923.7	35.0	7208.	83.8	74.6								
7	*119	169	15.70	1.57	56	*5830.2	3174.3	2055.9	45.6	*5580.4	3494.5	2085.9	37.4	7816.	83.1	72.3								
8	*119	169	15.70	1.57	67	*5080.2	2853.5	2226.7	43.8	*4996.9	3113.4	1883.5	37.7	7058.	85.2	75.7								
9	*119	169	15.70	1.57	78	*4580.2	2610.4	1969.8	43.0	*4496.9	2788.4	1708.5	38.0	6402.	86.7	78.8								
10	*119	169	24.29	1.05	56	*14264.6	10081.1	4183.5	29.3	*12722.4	9839.1	2683.3	22.7	10804.	69.9	57.9								
11	*119	169	24.29	1.05	67	*12529.0	9089.0	3440.0	27.5	*11372.5	8750.2	2622.3	23.1	9826.	73.0	62.8								
12	*119	169	24.29	1.05	78	*11371.9	8333.9	3038.0	26.7	*10215.5	7821.6	2393.9	23.4	8970.	75.3	67.2								
13	*119	169	24.29	1.31	56	*11739.9	7468.2	4271.7	36.4	*9886.2	7220.0	2666.2	27.0	9990.	75.6	65.9								
14	*119	169	24.29	1.31	67	*10348.9	6734.4	3614.6	34.9	*8804.3	6396.0	2408.4	27.4	9024.	78.5	70.1								
15	*119	169	24.29	1.31	78	*9267.2	6127.4	3139.8	33.9	*7877.1	5693.4	2183.7	27.7	8182.	80.7	74.3								
16	*119	169	24.29	1.57	56	*10051.3	5912.1	4139.2	41.2	*8247.3	5753.8	2493.5	30.2	9343.	79.5	70.3								
17	*119	169	24.29	1.57	67	*8762.0	5229.3	3532.7	40.3	*7086.9	4910.1	2176.8	30.7	8156.	82.3	75.3								
18	*119	169	24.29	1.57	78	*7859.5	4791.8	3067.7	39.0	*6442.2	4444.1	1998.1	31.0	7487.	83.9	78.5								
19	*119	169	31.06	1.05	56	*19473.0	14399.9	5073.1	26.1	*16022.1	13174.9	2847.2	17.8	10668.	66.5	56.2								
20	*119	169	31.06	1.05	67	*17253.6	12804.3	4409.3	25.6	*13802.9	11273.1	2529.8	18.3	9479.	69.9	63.1								
21	*119	169	31.06	1.05	78	*15527.5	11796.0	3731.5	24.0	*12569.9	10222.4	2347.5	18.7	8796.	73.0	66.4								
22	*119	169	31.06	1.31	56	*15802.4	10924.6	4877.7	30.9	*13432.0	10466.3	2965.7	22.1	11113.	71.9	59.8								
23	*119	169	31.06	1.31	67	*14023.6	9440.3	4583.3	32.7	*10665.9	8224.0	2441.9	22.9	9150.	76.4	70.3								
24	*119	169	31.06	1.31	78	*12640.3	8663.3	3976.9	31.5	*9677.7	7429.0	2248.7	23.2	8426.	78.6	73.6								
25	*119	169	31.06	1.57	56	*13182.6	8496.7	4685.9	35.5	*11040.4	8212.6	2827.8	25.6	10596.	76.7	65.7								
26	*119	169	31.06	1.57	67	*12028.1	7423.9	4604.2	38.3	*8732.8	6426.9	2305.9	26.4	8640.	79.8	74.8								
27	*119	169	31.06	1.57	78	*10709.2	6769.7	3939.5	36.8	*7908.4	5793.9	2114.5	26.7	7923.	82.0	78.0								

NOTE: # IN = POUNDS ENTERING
 # OUT = POUNDS LEAVING
 % REM = POUNDS REMOVED
 % = PERCENTAGE REMOVED FOR THAT UNIT
 SLUDGE=GALLONS OF SLUDGE PRODUCED ASSUMING 3.2% SOLIDS
 PLANT EFF= OVERALL PLANT EFFICIENCY
 O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED OPERATION
 MIN=15.70, MAX=31.06

COMPUTER SIMULATION OF TRICKLING FILTER OPERATION
ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 5 : SS #178 MG/L
BOD #169 MG/L

O U T P U T S T R E A M V E C T O R S

TRIAL#	*SS	BOD	FLOW	RECIRC	T	*FLOW	STATION # 2 BOD SS	STATION # 3 FLOW BOD SS	STATION # 4 FLOW BOD SS	STATION # 5 FLOW BOD SS	STATION # 7 FLOW BOD SS	STATION # 8 FLOW BOD SS	REMARK
1	*178	169	15.70	1.05	56	*14.95	173 182	0.75 79	83 15.70 169 178 15.67 92	92 14.93	79 83	14.91 50	58*
2	*178	169	15.70	1.05	67	*14.95	173 183	0.75 69	76 15.70 169 178 15.67 90	89 14.92	69 76	14.91 45	53*
3	*178	169	15.70	1.05	78	*14.95	174 183	0.75 62	70 15.70 169 178 15.67 87	86 14.92	62 70	14.91 42	48*
4	*178	169	15.70	1.31	56	*11.98	196 209	3.72 80	79 15.70 169 178 15.67 99	92 11.96	80 79	11.95 46	52*
5	*178	169	15.70	1.31	67	*11.98	199 210	3.72 71	74 15.70 169 178 15.67 96	89 11.96	71 74	11.95 42	48*
6	*178	169	15.70	1.31	78	*11.98	201 212	3.72 64	67 15.70 169 178 15.67 94	86 11.96	64 67	11.95 39	43*
7	*178	169	15.70	1.57	56	*10.00	219 236	5.70 80	78 15.70 169 178 15.67 104	92 9.97	80 78	9.96 43	49*
8	*178	169	15.70	1.57	67	*10.00	224 238	5.70 72	72 15.70 169 178 15.67 101	89 9.97	72 72	9.96 39	45*
9	*178	169	15.70	1.57	78	*10.00	228 242	5.70 65	65 15.70 169 178 15.67 99	86 9.97	65 65	9.96 36	40*
10	*178	169	24.29	1.05	56	*23.13	173 182	1.16 86	78 24.29 169 178 24.25 112	93 23.09	86 78	23.08 59	60*
11	*178	169	24.29	1.05	67	*23.13	173 183	1.16 77	71 24.29 169 178 24.25 109	90 23.09	77 71	23.08 54	55*
12	*178	169	24.29	1.05	78	*23.13	173 183	1.16 69	64 24.29 169 178 24.25 107	88 23.09	69 64	23.08 50	49*
13	*178	169	24.29	1.31	56	*18.54	194 209	5.75 88	76 24.29 169 178 24.25 120	93 18.50	88 76	18.49 55	55*
14	*178	169	24.29	1.31	67	*18.54	196 210	5.75 79	70 24.29 169 178 24.25 117	90 18.50	79 70	18.49 50	51*
15	*178	169	24.29	1.31	78	*18.54	199 213	5.75 71	62 24.29 169 178 24.25 114	88 18.50	71 62	18.49 46	45*
16	*178	169	24.29	1.57	56	*15.47	214 236	8.82 90	75 24.29 169 178 24.25 127	93 15.43	90 75	15.42 52	52*
17	*178	169	24.29	1.57	67	*15.47	219 236	8.82 80	71 24.29 169 178 24.25 124	90 15.43	80 71	15.42 48	49*
18	*178	169	24.29	1.57	78	*15.47	224 244	8.82 72	60 24.29 169 178 24.25 121	88 15.43	72 60	15.42 43	41*
19	*178	169	31.06	1.05	56	*29.58	172 183	1.48 91	76 31.06 169 178 31.01 126	94 29.53	91 76	29.52 66	62*
20	*178	169	31.06	1.05	67	*29.58	173 183	1.48 80	71 31.06 169 178 31.01 122	91 29.53	80 71	29.52 60	58*
21	*178	169	31.06	1.05	78	*29.58	173 183	1.48 74	62 31.06 169 178 31.01 119	88 29.53	74 62	29.52 55	50*
22	*178	169	31.06	1.31	56	*23.71	192 210	7.35 94	74 31.06 169 178 31.01 135	91 23.66	94 74	23.65 61	57*
23	*178	169	31.06	1.31	67	*23.71	195 212	7.35 82	66 31.06 169 178 31.01 131	91 23.66	82 66	23.65 55	51*
24	*178	169	31.06	1.31	78	*23.71	197 213	7.35 76	60 31.06 169 178 31.01 128	88 23.65	76 60	23.64 51	46*
25	*178	169	31.06	1.57	56	*19.78	211 238	11.28 95	72 31.06 169 178 31.01 142	94 19.73	95 72	19.72 57	53*
26	*178	169	31.06	1.57	67	*19.78	217 242	11.28 84	64 31.06 169 178 31.01 138	91 19.73	84 64	19.72 51	47*
27	*178	169	31.06	1.57	78	*19.78	221 244	11.28 77	59 31.06 169 178 31.01 135	88 19.73	77 59	19.72 48	43*

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R.=MEANSFLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70,MAX=31.06

PRIMARY TANK CHARACTERISTICS

INPUT VECTORS

OUTPUT

CHARACTERISTICS

TRIAL #	SS	BOD	FLOW	RECIRC	T	#	IN	#	OUT	BOD	#	OUT	REM	%	#	IN	SUSPENDED	#	OUT	REM	%	SLUDGE	REMARK
1	*178	169	15.70	1.05	56	*22128.5	12124.3	10004.3	45.6	*23307.0	12112.6	11194.4	48.3	26845.									
2	*178	169	15.70	1.05	67	*22128.5	11782.6	10345.9	46.7	*23307.0	11703.4	11603.5	50.0	27826.									
3	*178	169	15.70	1.05	78	*22128.5	11500.5	10628.0	48.5	*23307.0	11367.5	11939.5	51.7	28632.									
4	*178	169	15.70	1.31	56	*22128.5	12975.4	9153.1	41.4	*23307.0	12112.6	11194.4	48.3	26845.									
5	*178	169	15.70	1.31	67	*22128.5	12609.8	9518.7	43.2	*23307.0	11703.4	11603.5	50.0	27826.									
6	*178	169	15.70	1.31	78	*22128.5	12307.9	9820.6	44.4	*23307.0	11367.5	11939.5	51.7	28632.									
7	*178	169	15.70	1.57	56	*22128.5	13716.2	8412.3	38.5	*23307.0	12112.6	11194.4	48.3	26845.									
8	*178	169	15.70	1.57	67	*22128.5	13329.7	8798.8	40.2	*23307.0	11703.4	11603.5	50.0	27826.									
9	*178	169	15.70	1.57	78	*22128.5	13010.6	9117.9	41.4	*23307.0	11367.5	11939.5	51.7	28632.									
10	*178	169	24.29	1.05	56	*34235.8	22848.7	11387.0	33.7	*36059.0	18990.7	17709.7	49.4	42469.									
11	*178	169	24.29	1.05	67	*34235.8	22204.9	12030.9	35.5	*36059.0	18349.2	18236.5	50.6	43733.									
12	*178	169	24.29	1.05	78	*34235.8	21673.3	12562.5	36.7	*36059.0	17822.5	17709.7	49.4	42469.									
13	*178	169	24.29	1.31	56	*34235.8	24452.0	9783.0	29.0	*36059.0	18990.7	18236.5	50.6	43733.									
14	*178	169	24.29	1.31	67	*34235.8	23763.7	10472.1	30.8	*36059.0	18349.2	17709.7	49.4	42469.									
15	*178	169	24.29	1.31	78	*34235.8	23194.8	11040.9	32.5	*36059.0	17822.5	18236.5	50.6	43733.									
16	*178	169	24.29	1.57	56	*34235.8	25848.9	8386.9	24.9	*36059.0	18990.7	18236.5	47.8	40931.									
17	*178	169	24.29	1.57	67	*34235.8	25120.5	9115.3	26.6	*36059.0	18349.2	17709.7	49.4	42469.									
18	*178	169	24.29	1.57	78	*34235.8	24519.1	9716.6	28.4	*36059.0	17822.5	18236.5	50.6	43733.									
19	*178	169	31.06	1.05	56	*43777.8	32651.5	11126.3	25.4	*46109.2	24466.4	21642.8	47.2	51901.									
20	*178	169	31.06	1.05	67	*43777.8	31731.4	12006.4	27.8	*46109.2	23639.9	22469.2	48.9	53883.									
21	*178	169	31.06	1.05	78	*43777.8	30971.9	12805.9	29.6	*46109.2	22961.3	23147.9	50.6	55510.									
22	*178	169	31.06	1.31	56	*43777.8	34943.8	8834.0	20.1	*46109.2	24466.4	21642.8	47.2	51901.									
23	*178	169	31.06	1.31	67	*43777.8	33959.1	9818.7	22.5	*46109.2	23639.9	22469.2	48.9	53883.									
24	*178	169	31.06	1.31	78	*43777.8	33146.2	10631.6	24.3	*46109.2	22961.3	23147.9	50.6	55510.									
25	*178	169	31.06	1.57	56	*43777.8	36938.9	6838.9	16.0	*46109.2	24466.4	21642.8	47.2	51901.									
26	*178	169	31.06	1.57	67	*43777.8	35808.0	7879.8	18.3	*46109.2	23639.9	22469.2	48.9	53883.									
27	*178	169	31.06	1.57	78	*43777.8	35038.7	8739.1	20.1	*46109.2	22961.3	23147.9	50.6	55510.									

NOTE: # IN =POUNDS ENTERING
OUT =POUNDS LEAVING
REM =POUNDS REMOVED
% =PERCENTAGE OF SLUDGE PRODUCED FOR THAT UNIT
SLUDGE=GALLONS OF SLUDGE PRODUCED ASSUMING 5.0% SOLIDS
O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED PLANT OPERATION
MIN=15.70,MAX=31.06

COMPUTER SIMULATION OF TRICKLING FILTER OPERATION
ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 6 : SS =278 MG/L
BOD =169 MG/L

INPUT VECTORS

O U T P U T S T R E A M V E C T O R S

TRIAL #	*SS	BOD	FLOW	RECIRC	T	*STATION # 2	FLOW	BOD SS	*STATION # 3	FLOW	BOD SS	*STATION # 4	FLOW	BOD SS	*STATION # 5	FLOW	BOD SS	*STATION # 7	FLOW	BOD SS	*STATION # 8	SS	REMARK
1	*278	169	15.70	1.05	56	*14.95	172	286	0.75	93	105	15.70	169	278	15.65	97	114	14.90	93	105	14.89	59	74*
2	*278	169	15.70	1.05	67	*14.95	173	287	0.75	81	90	15.70	169	278	15.65	94	110	14.90	81	90	14.89	52	63*
3	*278	169	15.70	1.05	78	*14.95	173	287	0.75	75	84	15.70	169	278	15.65	92	107	14.90	75	84	14.89	49	58*
4	*278	169	15.70	1.31	56	*11.98	191	332	3.72	95	102	15.70	169	278	15.65	104	114	11.93	95	102	11.92	55	68*
5	*278	169	15.70	1.31	67	*11.98	195	337	3.72	83	89	15.70	169	278	15.65	101	110	11.93	83	89	11.92	49	59*
6	*278	169	15.70	1.31	78	*11.98	197	339	3.72	77	82	15.70	169	278	15.65	99	107	11.93	77	82	11.92	45	54*
7	*278	169	15.70	1.57	56	*10.00	210	380	5.70	97	99	15.70	169	278	15.65	110	114	9.95	97	99	9.94	51	63*
8	*278	169	15.70	1.57	67	*10.00	216	386	5.70	85	88	15.70	169	278	15.65	107	110	9.95	85	88	9.94	46	55*
9	*278	169	15.70	1.57	78	*10.00	220	391	5.70	78	79	15.70	169	278	15.65	104	107	9.95	78	79	9.94	42	49*
10	*278	169	24.29	1.05	56	*23.13	172	287	1.16	104	97	24.29	169	278	24.21	118	115	23.05	104	97	23.04	71	76*
11	*278	169	24.29	1.05	67	*23.13	173	287	1.16	91	86	24.29	169	278	24.21	115	111	23.05	91	86	23.04	63	67*
12	*278	169	24.29	1.05	78	*23.13	188	335	1.16	82	78	24.29	169	278	24.21	112	108	23.05	82	78	23.04	58	60*
13	*278	169	24.29	1.31	56	*18.54	192	338	5.75	106	94	24.29	169	278	24.21	127	115	18.46	106	94	18.45	65	69*
14	*278	169	24.29	1.31	67	*18.54	193	340	5.75	93	83	24.29	169	278	24.21	123	111	18.46	93	83	18.45	58	61*
15	*278	169	24.29	1.31	78	*18.54	195	340	5.75	83	76	24.29	169	278	24.21	120	108	18.46	83	76	18.45	53	55*
16	*278	169	24.29	1.57	56	*15.47	203	384	8.82	108	91	24.29	169	278	24.21	134	115	15.39	108	91	15.38	61	64*
17	*278	169	24.29	1.57	67	*15.47	211	390	8.82	95	81	24.29	169	278	24.21	130	111	15.39	95	81	15.38	55	57*
18	*278	169	24.29	1.57	78	*15.47	216	393	8.82	85	75	24.29	169	278	24.21	127	108	15.39	85	75	15.38	50	52*
19	*278	169	31.06	1.05	56	*29.58	171	287	1.48	110	93	31.06	169	278	30.96	132	116	29.48	110	93	29.47	78	77*
20	*278	169	31.06	1.05	67	*29.58	172	287	1.48	97	82	31.06	169	278	30.96	129	112	29.48	97	82	29.47	70	68*
21	*278	169	31.06	1.05	78	*29.58	173	288	1.48	87	76	31.06	169	278	30.96	126	109	29.48	87	76	29.46	64	62*
22	*278	169	31.06	1.31	56	*23.71	186	336	7.35	113	90	31.06	169	278	30.96	142	116	23.61	113	90	23.60	72	71*
23	*278	169	31.06	1.31	67	*23.71	190	339	7.35	99	79	31.06	169	278	30.96	138	112	23.61	99	79	23.59	64	61*
24	*278	169	31.06	1.31	78	*23.71	193	341	7.35	89	74	31.06	169	278	30.96	134	109	23.61	89	74	23.59	59	57*
25	*278	169	31.06	1.57	56	*19.78	199	386	11.28	115	87	31.06	169	278	30.96	150	116	19.68	115	87	19.67	67	65*
26	*278	169	31.06	1.57	67	*19.78	207	392	11.28	101	77	31.06	169	278	30.96	146	112	19.68	101	77	19.67	60	57*
27	*278	169	31.06	1.57	78	*19.78	213	395	11.28	91	72	31.06	169	278	30.96	142	109	19.68	91	72	19.67	56	53*

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R.=MEANSFLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70,MAX=31.06

***** PRIMARY TANK CHARACTERISTICS *****

INPUT VECTORS

OUTPUT *****

REMARK

SLUDGE

SOLIDS %

REM

SUSPENDED # OUT

IN

%

REM

%

OUT

IN

T

RECIRC

FLOW

BOD

SS

TRIAL #

1	*278	169	15.70	1.05	56	*22128.5	12744.6	9383.9	42.6	*	36400.8	14920.5	21480.3	59.0	51511.
2	*278	169	15.70	1.05	67	*22128.5	12385.4	9743.1	44.4	*	36400.8	14416.5	21984.2	60.4	52720.
3	*278	169	15.70	1.05	78	*22128.5	12089.0	10039.6	45.6	*	36400.8	14002.7	22398.1	61.5	53712.
4	*278	169	15.70	1.31	56	*22128.5	13639.3	8489.2	38.5	*	36400.8	14920.5	21480.3	59.0	51511.
5	*278	169	15.70	1.31	67	*22128.5	13254.9	8873.6	40.2	*	36400.8	14416.5	21984.2	60.4	52720.
6	*278	169	15.70	1.31	78	*22128.5	12937.6	9190.9	41.4	*	36400.8	14002.7	22398.1	61.5	53712.
7	*278	169	15.70	1.57	56	*22128.5	14418.0	7710.5	34.9	*	36400.8	14920.5	21480.3	59.0	51511.
8	*278	169	15.70	1.57	67	*22128.5	14011.7	8116.8	36.7	*	36400.8	14416.5	21984.2	60.4	52720.
9	*278	169	15.70	1.57	78	*22128.5	13676.3	8452.2	38.5	*	36400.8	14002.7	22398.1	61.5	53712.
10	*278	169	24.29	1.05	56	*34235.8	24017.8	10218.0	30.2	*	56316.8	23393.1	32923.7	58.6	78954.
11	*278	169	24.29	1.05	67	*34235.8	23340.9	10894.8	32.0	*	56316.8	22603.0	33713.9	60.1	80849.
12	*278	169	24.29	1.05	78	*34235.8	22782.2	11453.5	33.7	*	56316.8	21954.1	34362.7	61.2	82405.
13	*278	169	24.29	1.31	56	*34235.8	25703.9	8531.9	24.9	*	56316.8	23393.1	32923.7	58.6	78954.
14	*278	169	24.29	1.31	67	*34235.8	24979.6	9256.2	27.2	*	56316.8	22603.0	33713.9	60.1	80849.
15	*278	169	24.29	1.31	78	*34235.8	24381.6	9854.2	29.0	*	56316.8	21954.1	34362.7	61.2	82405.
16	*278	169	24.29	1.57	56	*34235.8	27171.4	7064.3	20.7	*	56316.8	23393.1	32923.7	58.6	78954.
17	*278	169	24.29	1.57	67	*34235.8	26405.8	7830.0	23.1	*	56316.8	22603.0	33713.9	60.1	80849.
18	*278	169	24.29	1.57	78	*34235.8	25773.7	8462.1	24.9	*	56316.8	21954.1	34362.7	61.2	82405.
19	*278	169	31.06	1.05	56	*43777.8	34322.2	9455.6	21.9	*	72013.2	30138.1	41875.1	58.3	100420.
20	*278	169	31.06	1.05	67	*43777.8	33355.0	10422.9	23.7	*	72013.2	29120.1	42893.1	59.7	102861.
21	*278	169	31.06	1.05	78	*43777.8	32556.5	11221.3	25.4	*	72013.2	28284.2	43729.0	60.8	104866.
22	*278	169	31.06	1.31	56	*43777.8	36731.7	7046.1	16.0	*	72013.2	30138.1	41875.1	58.3	100420.
23	*278	169	31.06	1.31	67	*43777.8	35696.6	8081.2	18.3	*	72013.2	29120.1	42893.1	59.7	102861.
24	*278	169	31.06	1.31	78	*43777.8	34842.1	8935.7	20.7	*	72013.2	28284.2	43729.0	60.8	104866.
25	*278	169	31.06	1.57	56	*43777.8	38828.9	4948.9	11.2	*	72013.2	30138.1	41875.1	58.3	100420.
26	*278	169	31.06	1.57	67	*43777.8	37734.7	6043.2	13.6	*	72013.2	29120.1	42893.1	59.7	102861.
27	*278	169	31.06	1.57	78	*43777.8	36831.4	6946.4	16.0	*	72013.2	28284.2	43729.0	60.8	104866.

NOTE: # IN =POUNDS ENTERING
OUT =POUNDS LEAVING
REM =POUNDS REMOVED
% =PERCENTAGE REMOVED FOR THAT UNIT
SLUDGE=GALLONS OF SLUDGE PRODUCED ASSUMING 5.0% SOLIDS
O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED PLANT OPERATION
MIN=15.70,MAX=31.06

CHARACTERISTICS

TRICKLING FILTER

OUTPUT

INPUT VECTORS

TRIAL#

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NOTE: # IN = POUNDS ENTERING
OUT = POUNDS LEAVING
REM = POUNDS REMOVED
% = PERCENTAGE REMOVED FOR THAT UNIT
OVER, X REM = OVERALL EFFICIENCY COMPARING THE OUTPUT FOR
THIS UNIT INPUT AT STATION # 2
O.O.R. = MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70, MAX=31.06

INPUT VECTORS

TRIAL#	#	ASS	BOD	FLOW	RECIRC	T	#	IN	#	OUT	BOD	#	REM	%	#	IN	#	OUT	SUSPENDED	%	REM	%	SOLIDS	PLANT EFF	REMARK
1	1	*278	169	15.70	1.05	56	*11557.4	7424.4	4133.0	35.0	*13048.7	9226.2	3822.5	29.3	14323.0	65.7	74.1								
2	2	*278	169	15.70	1.05	67	*10065.3	6557.3	3508.0	36.9	*11183.7	7852.1	3331.6	29.8	12483.0	69.9	78.0								
3	3	*278	169	15.70	1.05	78	*9319.1	6160.0	3159.1	33.9	*10437.4	7305.0	3132.4	30.0	11737.0	71.7	79.8								
4	4	*278	169	15.70	1.31	56	*9454.7	5485.9	3968.8	42.0	*10151.4	6777.5	3373.9	33.2	12642.0	71.2	79.5								
5	5	*278	169	15.70	1.31	67	*8259.6	4886.4	3373.2	40.8	*8856.7	5876.4	2980.3	33.7	11167.0	74.9	82.5								
6	6	*278	169	15.70	1.31	78	*7661.9	4570.6	3091.2	40.3	*8159.4	5393.5	2765.9	33.9	10364.0	77.2	84.1								
7	7	*278	169	15.70	1.57	56	*8048.1	4277.8	3770.3	46.8	*8214.1	5230.9	2983.1	36.3	11178.0	75.7	83.4								
8	8	*278	169	15.70	1.57	67	*7051.6	3843.6	3208.0	45.5	*7300.5	4624.1	2676.4	36.7	10028.0	78.7	85.8								
9	9	*278	169	15.70	1.57	78	*6470.3	3540.7	2929.6	45.3	*6553.2	4130.4	2422.8	37.0	9078.0	80.9	87.5								
10	10	*278	169	24.29	1.05	56	*19996.4	13667.4	6329.0	31.7	*18650.5	14669.3	3981.2	21.3	14918.0	58.7	73.5								
11	11	*278	169	24.29	1.05	67	*17495.4	12260.1	5235.4	29.9	*16534.1	12933.4	3600.8	21.8	13492.0	63.4	76.7								
12	12	*278	169	24.29	1.05	78	*15764.1	11247.0	4517.0	28.7	*14995.1	11677.3	3317.8	22.1	12432.0	66.5	79.1								
13	13	*278	169	24.29	1.31	56	*16322.0	10078.8	6243.2	38.3	*14474.3	10748.7	3725.6	25.7	13960.0	65.4	79.4								
14	14	*278	169	24.29	1.31	67	*14318.8	9037.1	5281.7	36.9	*12779.1	9436.0	3343.1	26.2	12527.0	69.8	82.0								
15	15	*278	169	24.29	1.31	78	*12778.1	8285.4	4492.7	35.2	*11700.4	8604.7	3095.7	26.5	11599.0	72.8	83.8								
16	16	*278	169	24.29	1.57	56	*13864.2	7843.7	6020.5	43.4	*11681.9	8273.6	3408.2	29.2	12771.0	70.0	83.3								
17	17	*278	169	24.29	1.57	67	*12193.9	7063.7	5130.2	42.1	*10396.9	7324.4	3072.5	29.6	11513.0	73.9	85.4								
18	18	*278	169	24.29	1.57	78	*10909.2	6512.2	4397.0	40.3	*9625.8	6757.3	2868.5	29.8	10748.0	76.9	86.8								
19	19	*278	169	31.06	1.05	56	*27045.4	19234.0	7811.4	28.9	*22865.7	19099.2	3766.4	16.5	14113.0	54.4	73.2								
20	20	*278	169	31.06	1.05	67	*23847.2	17276.5	6570.7	27.6	*20159.5	16742.2	3417.3	17.0	12805.0	59.3	76.3								
21	21	*278	169	31.06	1.05	78	*21387.3	15952.2	5435.1	25.4	*18683.1	15462.3	3220.8	17.2	12069.0	63.0	78.5								
22	22	*278	169	31.06	1.31	56	*22350.1	14226.9	8023.1	36.1	*17721.3	13974.7	3746.6	21.1	14039.0	61.3	78.9								
23	23	*278	169	31.06	1.31	67	*19091.4	12718.3	6773.1	34.7	*15553.7	12192.5	3361.3	21.6	12595.0	66.3	82.0								
24	24	*278	169	31.06	1.31	78	*18821.0	11805.8	7818.2	32.6	*14568.1	11305.6	3182.4	21.8	11925.0	69.4	83.3								
25	25	*278	169	31.06	1.57	56	*16577.7	9930.3	6647.4	40.1	*12638.5	9451.7	3541.2	24.8	13269.0	66.3	83.2								
26	26	*278	169	31.06	1.57	67	*14934.8	9220.0	5714.8	38.3	*11816.6	8809.8	3006.8	25.4	11266.0	73.7	86.6								
27	27	*278	169	31.06	1.57	78																			

NOTE: # IN = POUNDS ENTERING
 # OUT = POUNDS LEAVING
 # REM = POUNDS REMOVED
 % = PERCENTAGE OF SLUDGE PRODUCED FOR THAT UNIT
 SLUDGE = GALLONS OF SLUDGE PRODUCED ASSUMING 3.2% SOLIDS
 PLANT EFF = OVERALL PLANT EFFICIENCY
 O.D.R. = MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED OPERATION
 MIN=15.70, MAX=31.06

COMPUTER SIMULATION OF TRICKLING FILTER OPERATION
ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 7 : SS =119 MG/L
BOD =240 MG/L

OUTPUT -- STREAM VECTORS

TRIAL#	*SS	BOD	FLOW	RECIRC	T	*FLOW	STATION # 2	STATION # 3	STATION # 4	STATION # 5	STATION # 7	STATION # 8	REMARK
#							FLOW BOD SS	FLOW BOD SS	FLOW BOD SS	FLOW BOD SS	FLOW BOD SS	FLOW BOD SS	
1	*119	240	15.70	1.05	56	*14.95	247 121	0.75 80	76 15.70 240	119 15.69 110	76 14.94	80 76 14.93	53*
2	*119	240	15.70	1.05	67	*14.95	248 121	0.75 72	68 15.70 240	119 15.69 107	74 14.94	72 68 14.93	47*
3	*119	240	15.70	1.05	78	*14.95	248 121	0.75 65	62 15.70 240	119 15.69 104	71 14.94	65 62 14.93	42*
4	*119	240	15.70	1.31	56	*11.98	288 132	3.72 82	74 15.70 240	119 15.69 118	76 11.97	82 74 11.96	48*
5	*119	240	15.70	1.31	67	*11.98	291 135	3.72 74	66 15.70 240	119 15.69 114	74 11.97	74 66 11.96	43*
6	*119	240	15.70	1.31	78	*11.98	293 136	3.72 66	60 15.70 240	119 15.69 111	71 11.97	66 60 11.96	37 39*
7	*119	240	15.70	1.57	56	*10.00	329 145	5.70 83	72 15.70 240	119 15.69 124	76 9.99	83 72 9.98	45*
8	*119	240	15.70	1.57	67	*10.00	334 149	5.70 75	64 15.70 240	119 15.69 121	74 9.99	75 64 9.98	40*
9	*119	240	15.70	1.57	78	*10.00	338 153	5.70 67	58 15.70 240	119 15.69 118	71 9.99	67 58 9.98	35 36*
10	*119	240	24.29	1.05	56	*23.13	247 121	1.16 89	71 24.29 240	119 24.27 134	77 23.11	89 71 23.10	55*
11	*119	240	24.29	1.05	67	*23.13	247 121	1.16 80	69 24.29 240	119 24.27 130	75 23.11	80 69 23.10	53*
12	*119	240	24.29	1.05	78	*23.13	248 122	1.16 72	57 24.29 240	119 24.27 127	72 23.11	72 57 23.10	48 43*
13	*119	240	24.29	1.31	56	*18.54	286 134	5.75 91	69 24.29 240	119 24.27 143	77 18.52	91 69 18.51	54 50*
14	*119	240	24.29	1.31	67	*18.54	289 132	5.75 80	68 24.29 240	119 24.27 139	75 18.52	80 68 18.51	49*
15	*119	240	24.29	1.31	78	*18.54	291 137	5.75 74	56 24.29 240	119 24.27 136	72 18.52	74 56 18.51	45 40*
16	*119	240	24.29	1.57	56	*15.47	323 140	8.82 93	67 24.29 240	119 24.27 147	75 15.45	93 67 15.44	50 46*
17	*119	240	24.29	1.57	67	*15.47	330 151	8.82 82	60 24.29 240	119 24.27 152	75 15.45	82 60 15.44	45 41*
18	*119	240	24.29	1.57	78	*15.47	334 153	8.82 75	55 24.29 240	119 24.27 144	72 15.45	75 55 15.44	42 38*
19	*119	240	31.06	1.05	56	*29.58	247 121	1.48 95	68 31.06 240	119 31.03 145	75 29.55	95 68 29.54	56*
20	*119	240	31.06	1.05	67	*29.58	247 121	1.48 83	61 31.06 240	119 31.03 142	73 29.55	83 61 29.54	50*
21	*119	240	31.06	1.05	78	*29.58	248 121	1.48 77	56 31.06 240	119 31.03 156	73 29.55	77 56 29.54	45*
22	*119	240	31.06	1.31	56	*23.71	284 135	7.35 97	66 31.06 240	119 31.03 160	78 23.68	97 66 23.67	51*
23	*119	240	31.06	1.31	67	*23.71	288 137	7.35 85	58 31.06 240	119 31.03 152	75 23.68	85 58 23.67	44*
24	*119	240	31.06	1.31	78	*23.71	289 136	7.35 79	58 31.06 240	119 31.03 152	73 23.68	79 58 23.67	51 44*
25	*119	240	31.06	1.57	56	*19.78	320 150	11.28 99	64 31.06 240	119 31.03 169	78 19.76	99 64 19.75	47*
26	*119	240	31.06	1.57	67	*19.78	327 153	11.28 87	57 31.06 240	119 31.03 165	75 19.76	87 57 19.75	50 42*
27	*119	240	31.06	1.57	78	*19.78	331 143	11.28 80	63 31.06 240	119 31.03 161	73 19.76	80 63 19.75	46*

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R.=MEANSFLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70,MAX=31.06

INPUT VECTORS ***** OUTPUT -- PRIMARY TANK CHARACTERISTICS *****

TRIAL#	#	BOD	FLOW	RECIRC	T	#	#	IN	B O D		#	REM	%	#	#	IN	#	OU	SUSPENDED		#	REM	%	SLUDGE	REMARK
									#	OUT									#	OU					
1	*119	240	15.70	1.05	56	*	31425.1	14431.5	16993.6	54.2	*	15581.6	10033.7	5548.0	36.1	13304.			10033.7	5548.0	36.1	13304.			
2	*119	240	15.70	1.05	67	*	31425.1	14024.8	17400.3	55.4	*	15581.6	9694.7	5886.9	37.8	14117.			9694.7	5886.9	37.8	14117.			
3	*119	240	15.70	1.05	78	*	31425.1	13689.1	17736.0	56.7	*	15581.6	9416.4	6165.2	40.3	14785.			9416.4	6165.2	40.3	14785.			
4	*119	240	15.70	1.31	56	*	31425.1	15444.7	15980.5	50.8	*	15581.6	10033.7	5548.0	36.1	13304.			10033.7	5548.0	36.1	13304.			
5	*119	240	15.70	1.31	67	*	31425.1	15009.4	16415.7	52.5	*	15581.6	9694.7	5886.9	37.8	14117.			9694.7	5886.9	37.8	14117.			
6	*119	240	15.70	1.31	78	*	31425.1	14650.1	16775.0	53.7	*	15581.6	9416.4	6165.2	40.3	14785.			9416.4	6165.2	40.3	14785.			
7	*119	240	15.70	1.57	56	*	31425.1	16326.5	15098.6	48.3	*	15581.6	10033.7	5548.0	36.1	13304.			10033.7	5548.0	36.1	13304.			
8	*119	240	15.70	1.57	67	*	31425.1	15866.4	15530.7	49.6	*	15581.6	9694.7	5886.9	37.8	14117.			9694.7	5886.9	37.8	14117.			
9	*119	240	15.70	1.57	78	*	31425.1	15486.6	15938.5	50.8	*	15581.6	9416.4	6165.2	40.3	14785.			9416.4	6165.2	40.3	14785.			
10	*119	240	24.29	1.05	56	*	48618.8	27196.9	21422.0	44.2	*	24106.8	15731.3	8375.6	35.3	20085.			15731.3	8375.6	35.3	20085.			
11	*119	240	24.29	1.05	67	*	48618.8	26430.5	22188.4	45.8	*	24106.8	15199.9	8906.9	37.0	21360.			15199.9	8906.9	37.0	21360.			
12	*119	240	24.29	1.05	78	*	48618.8	25797.8	22821.1	47.1	*	24106.8	14763.6	9343.3	39.5	22406.			14763.6	9343.3	39.5	22406.			
13	*119	240	24.29	1.05	56	*	48618.8	29106.2	19512.7	40.4	*	24106.8	15731.3	8375.6	35.3	20085.			15731.3	8375.6	35.3	20085.			
14	*119	240	24.29	1.31	67	*	48618.8	20286.0	20332.9	42.1	*	24106.8	15199.9	8906.9	37.0	21360.			15199.9	8906.9	37.0	21360.			
15	*119	240	24.29	1.31	78	*	48618.8	27608.8	21010.0	43.3	*	24106.8	14763.6	9343.3	39.5	22406.			14763.6	9343.3	39.5	22406.			
16	*119	240	24.29	1.57	56	*	48618.8	30768.0	17850.9	36.7	*	24106.8	15731.3	8375.6	35.3	20085.			15731.3	8375.6	35.3	20085.			
17	*119	240	24.29	1.57	67	*	48618.8	29900.9	18717.9	38.7	*	24106.8	15199.9	8906.9	37.0	21360.			15199.9	8906.9	37.0	21360.			
18	*119	240	24.29	1.57	78	*	48618.8	29185.2	19433.7	40.0	*	24106.8	14763.6	9343.3	39.5	22406.			14763.6	9343.3	39.5	22406.			
19	*119	240	31.06	1.05	56	*	62169.7	38865.2	23304.5	37.5	*	30825.8	20267.1	10558.7	34.5	25321.			20267.1	10558.7	34.5	25321.			
20	*119	240	31.06	1.05	67	*	62169.7	37770.0	24399.7	39.6	*	30825.8	19582.5	11243.3	37.0	26962.			19582.5	11243.3	37.0	26962.			
21	*119	240	31.06	1.05	78	*	62169.7	36865.9	25303.8	40.8	*	30825.8	19020.4	11805.4	38.7	28310.			19020.4	11805.4	38.7	28310.			
22	*119	240	31.06	1.31	56	*	62169.7	41593.7	20576.0	33.3	*	30825.8	20267.1	10558.7	34.5	25321.			20267.1	10558.7	34.5	25321.			
23	*119	240	31.06	1.31	67	*	62169.7	40421.6	22748.1	35.0	*	30825.8	19582.5	11243.3	37.0	26962.			19582.5	11243.3	37.0	26962.			
24	*119	240	31.06	1.31	78	*	62169.7	39454.0	22715.7	36.7	*	30825.8	19020.4	11805.4	38.7	28310.			19020.4	11805.4	38.7	28310.			
25	*119	240	31.06	1.57	56	*	62169.7	43968.4	18201.3	29.6	*	30825.8	20267.1	10558.7	34.5	25321.			20267.1	10558.7	34.5	25321.			
26	*119	240	31.06	1.57	67	*	62169.7	42729.4	19440.2	31.3	*	30825.8	19582.5	11243.3	37.0	26962.			19582.5	11243.3	37.0	26962.			
27	*119	240	31.06	1.57	78	*	62169.7	41706.6	20463.1	32.9	*	30825.8	19020.4	11805.4	38.7	28310.			19020.4	11805.4	38.7	28310.			

NOTE: # IN = POUNDS ENTERING
OUT = POUNDS LEAVING
REM = POUNDS REMOVED
% = PERCENTAGE OF SLUDGE PRODUCED FOR THAT UNIT
SLUDGE = GALLONS OF SLUDGE PRODUCED ASSUMING 5.0% SOLIDS
O.O.R. = MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED PLANT OPERATION
MIN=15.70, MAX=31.06

INPUT VECTORS

TRIAL#	#	*SS	BOD	FLOW	RECIRC	T	#	IN	#	OUT	BOD	#	REM	%	#	IN	#	OUT	SUSPENDED	#	REM	%	OVERALL	X	REM	REMARK
1	*119	240	15.70	1.05	56	*	14431.5	10630.8	3800.7	26.3	*	10033.7	9997.0	36.6	0.4	67.2	37.2									
2	*119	240	15.70	1.05	67	*	14024.8	9356.8	4668.1	33.3	*	9694.7	8884.4	810.4	8.4	71.4	44.6									
3	*119	240	15.70	1.05	78	*	13689.1	8397.1	5292.0	38.7	*	9416.4	8038.0	1377.6	14.6	74.2	49.6									
4	*119	240	15.70	1.31	56	*	15444.7	10882.0	4562.6	29.5	*	10033.7	9656.2	377.4	3.8	71.2	44.7									
5	*119	240	15.70	1.31	67	*	15009.4	9577.8	5431.6	36.2	*	9694.7	8581.5	1113.2	11.5	74.9	51.9									
6	*119	240	15.70	1.31	78	*	14650.1	8595.5	6054.6	41.3	*	9416.4	7764.8	1651.7	17.5	77.0	56.6									
7	*119	240	15.70	1.57	56	*	16326.5	11092.0	5234.5	32.1	*	10033.7	9386.0	647.6	6.5	74.5	51.0									
8	*119	240	15.70	1.57	67	*	15866.4	9762.6	6103.8	38.5	*	9694.7	8341.4	1353.3	14.0	77.8	57.7									
9	*119	240	15.70	1.57	78	*	15486.6	8761.3	6725.2	43.4	*	9416.4	7547.5	1869.0	19.8	80.5	62.7									
10	*119	240	24.29	1.05	56	*	27196.9	18358.5	8838.4	32.5	*	15731.3	14260.9	1470.4	9.3	63.6	48.8									
11	*119	240	24.29	1.05	67	*	26430.5	16158.2	10272.3	38.9	*	15199.9	12673.7	2526.2	16.6	68.0	48.8									
12	*119	240	24.29	1.05	78	*	25797.8	14501.0	11296.8	43.8	*	14763.6	11467.5	3296.1	22.3	71.4	54.1									
13	*119	240	24.29	1.31	56	*	29106.2	18792.2	10313.9	35.4	*	15731.3	13774.8	1956.5	12.4	67.8	49.3									
14	*119	240	24.29	1.31	67	*	28286.0	16540.0	11746.0	41.5	*	15199.9	12241.7	2958.2	19.5	72.0	54.5									
15	*119	240	24.29	1.31	78	*	27600.8	14843.6	12765.2	46.2	*	14763.6	11076.6	3687.0	25.0	74.9	60.6									
16	*119	240	24.29	1.57	56	*	30768.0	19154.0	11613.2	37.7	*	15731.3	13389.3	2342.0	14.9	70.9	55.4									
17	*119	240	24.29	1.57	67	*	29900.9	16859.1	13041.9	43.6	*	15199.9	11899.1	3300.8	21.7	74.8	61.6									
18	*119	240	31.06	1.57	78	*	29185.2	15130.0	14055.2	48.2	*	14763.6	10766.6	3996.9	27.1	77.8	65.4									
19	*119	240	31.06	1.05	56	*	38865.2	24974.9	13890.3	35.7	*	20267.1	17420.5	2846.6	14.0	61.1	44.6									
20	*119	240	31.06	1.05	67	*	37770.0	21981.7	15788.3	41.8	*	19582.5	15081.7	4100.9	20.9	66.0	51.2									
21	*119	240	31.06	1.05	78	*	36865.9	19727.3	17138.6	46.5	*	19020.4	14008.2	5012.2	26.4	69.4	55.4									
22	*119	240	31.06	1.31	56	*	41593.7	25565.0	16028.7	38.5	*	20267.1	16826.7	3440.4	17.0	65.5	51.9									
23	*119	240	31.06	1.31	67	*	40421.6	22501.1	17920.5	44.3	*	19582.5	14953.9	4628.6	23.6	70.1	58.4									
24	*119	240	31.06	1.31	78	*	39454.0	20193.4	19260.6	48.8	*	19020.4	13530.7	5489.7	28.9	73.0	61.8									
25	*119	240	31.06	1.57	56	*	43968.4	26058.2	17910.2	40.7	*	20267.1	16355.8	3911.3	19.3	68.8	58.0									
26	*119	240	31.06	1.57	67	*	42729.4	22935.2	19794.2	46.3	*	19582.5	14535.5	5047.1	25.8	73.1	63.4									
27	*119	240	31.06	1.57	78	*	41706.6	20583.0	21123.6	50.6	*	19020.4	13152.0	5868.3	30.9	76.1	65.0									

NOTE: # IN =POUNDS ENTERING
 # OUT =POUNDS LEAVING
 # REM =POUNDS REMOVED
 % =PERCENTAGE EFFICIENCY COMPARING THE OUTPUT FOR
 OVER,% REM=OVERALL EFFICIENCY COMPARING THE OUTPUT FOR
 THIS UNIT INPUT AT STATION # 2
 O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
 RANGE OF THE ESTABLISHED OPERATION
 MIN=15.70,MAX=31.06

INPUT VECTORS										SECONDARY TANK CHARACTERISTICS										SOLIDS										PLANT EFF										REMARK									
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COMPUTER SIMULATION OF TRICKLING FILTER OPERATION ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 8 : SS =178 MG/L
BOD =240 MG/L

OUTPUT STREAM VECTORS *****

TRIAL #	*SS	BOD	FLOW	RECIRC	T	* STATION # 2		STATION # 3		STATION # 4		STATION # 5		STATION # 7		STATION # 8		REMARK			
						FLOW	BOD SS	FLOW	BOD SS	FLOW	BOD SS	FLOW	BOD SS	FLOW	BOD SS	FLOW	BOD SS				
1	*178	240	15.70	1.05	56	*14.95	247 182	0.75	95	91	15.70	240 178	15.67	115	92	14.93	95	91	14.91	57	64*
2	*178	240	15.70	1.05	67	*14.95	247 182	0.75	83	81	15.70	240 178	15.67	112	89	14.92	83	81	14.91	51	56*
3	*178	240	15.70	1.05	78	*14.95	248 183	0.75	76	75	15.70	240 178	15.67	109	86	14.92	76	75	14.91	47	52*
4	*178	240	15.70	1.31	56	*11.98	284 205	3.72	97	88	15.70	240 178	15.67	123	92	11.96	97	88	11.95	52	58*
5	*178	240	15.70	1.31	67	*11.98	288 208	3.72	85	78	15.70	240 178	15.67	120	89	11.96	85	78	11.95	47	51*
6	*178	240	15.70	1.31	78	*11.98	290 210	3.72	78	73	15.70	240 178	15.67	117	86	11.96	78	73	11.95	44	48*
7	*178	240	15.70	1.57	56	*10.00	320 230	5.70	99	86	15.70	240 178	15.67	130	92	9.97	99	86	9.96	49	54*
8	*178	240	15.70	1.57	67	*10.00	327 236	5.70	87	76	15.70	240 178	15.67	126	89	9.97	87	76	9.96	44	47*
9	*178	240	15.70	1.57	78	*10.00	331 236	5.70	80	73	15.70	240 178	15.67	123	86	9.97	80	73	9.96	41	45*
10	*178	240	24.29	1.05	56	*23.13	246 182	1.16	106	84	24.29	240 178	24.25	140	93	23.09	106	84	23.08	67	65*
11	*178	240	24.29	1.05	67	*23.13	247 183	1.16	93	76	24.29	240 178	24.25	136	90	23.09	93	76	23.08	61	59*
12	*178	240	24.29	1.05	78	*23.13	247 183	1.16	83	69	24.29	240 178	24.25	133	88	23.09	83	69	23.08	56	53*
13	*178	240	24.29	1.31	56	*18.54	280 208	5.75	108	81	24.29	240 178	24.25	150	93	18.50	108	81	18.49	62	59*
14	*178	240	24.29	1.31	67	*18.54	284 210	5.75	95	74	24.29	240 178	24.25	146	90	18.50	95	74	18.49	56	54*
15	*178	240	24.29	1.31	78	*18.54	288 212	5.75	85	67	24.29	240 178	24.25	142	88	18.50	85	67	18.49	51	49*
16	*178	240	24.29	1.57	56	*15.47	314 234	8.82	110	79	24.29	240 178	24.25	159	93	15.43	110	79	15.42	58	55*
17	*178	240	24.29	1.57	67	*15.47	321 238	8.82	97	72	24.29	240 178	24.25	154	90	15.43	97	72	15.42	53	50*
18	*178	240	24.29	1.57	78	*15.47	327 242	8.82	87	65	24.29	240 178	24.25	150	88	15.43	87	65	15.42	48	45*
19	*178	240	31.06	1.05	56	*29.58	246 182	1.48	113	80	31.06	240 178	31.01	157	94	29.53	113	80	29.52	74	66*
20	*178	240	31.06	1.05	67	*29.58	247 183	1.48	99	73	31.06	240 178	31.01	152	91	29.53	99	73	29.52	67	60*
21	*178	240	31.06	1.05	78	*29.58	247 183	1.48	89	66	31.06	240 178	31.00	149	88	29.53	89	66	29.51	62	54*
22	*178	240	31.06	1.31	56	*23.71	278 208	7.35	115	78	31.06	240 178	31.01	168	94	23.66	115	78	23.65	68	61*
23	*178	240	31.06	1.31	67	*23.71	283 211	7.35	101	71	31.06	240 178	31.01	163	91	23.66	101	71	23.64	62	55*
24	*178	240	31.06	1.31	78	*23.71	286 213	7.35	91	64	31.06	240 178	31.00	159	88	23.65	91	64	23.64	57	49*
25	*178	240	31.06	1.57	56	*19.78	310 236	11.28	117	76	31.06	240 178	31.01	177	94	19.73	117	76	19.72	64	56*
26	*178	240	31.06	1.57	67	*19.78	318 240	11.28	103	69	31.06	240 178	31.01	172	91	19.73	103	69	19.72	58	51*
27	*178	240	31.06	1.57	78	*19.78	324 244	11.28	92	62	31.06	240 178	31.00	168	88	19.73	92	62	19.72	53	45*

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R. = MEAN FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70, MAX=31.06

INPUT VECTORS ***** O U T P U T P R I M A R Y T A N K C H A R A C T E R I S T I C S *****

TRIAL#	*SS	BOD	FLOW	RECIRC	T	* #	# IN	# OUT	B O D	# REM	%	* #	# IN	SUSPENDED	# OUT	# REM	SOLIDS	%	SLUDGE	REMARK
1	*178	240	15.70	1.05	56	*	31425.1	15096.8	16328.3	52.1	*	23307.0	12112.6	11194.4	48.3	26845.				
2	*178	240	15.70	1.05	67	*	31425.1	14671.3	16753.8	53.3	*	23307.0	11703.4	11603.5	50.0	27826.				
3	*178	240	15.70	1.05	78	*	31425.1	14320.1	17105.0	54.6	*	23307.0	11367.5	11939.5	51.7	28632.				
4	*178	240	15.70	1.31	56	*	31425.1	16156.6	15268.5	48.7	*	23307.0	12112.6	11194.4	48.3	26845.				
5	*178	240	15.70	1.31	67	*	31425.1	15701.3	15723.8	50.0	*	23307.0	11703.4	11603.5	50.0	27826.				
6	*178	240	15.70	1.31	78	*	31425.1	15325.5	16099.7	51.2	*	23307.0	11367.5	11939.5	51.7	28632.				
7	*178	240	15.70	1.57	56	*	31425.1	17079.1	14346.1	45.8	*	23307.0	12112.6	11194.4	48.3	26845.				
8	*178	240	15.70	1.57	67	*	31425.1	16597.8	14827.3	47.5	*	23307.0	11703.4	11603.5	50.0	27826.				
9	*178	240	15.70	1.57	78	*	31425.1	16200.5	15224.7	48.7	*	23307.0	11367.5	11939.5	51.7	28632.				
10	*178	240	24.29	1.05	56	*	48618.8	28450.5	20168.3	41.7	*	36059.0	18990.7	17068.3	47.8	40931.				
11	*178	240	24.29	1.05	67	*	48618.8	27648.8	20970.0	43.3	*	36059.0	18349.2	17709.7	49.4	42469.				
12	*178	240	24.29	1.05	78	*	48618.8	26986.9	21631.9	44.6	*	36059.0	17822.5	18236.5	50.6	43733.				
13	*178	240	24.29	1.31	56	*	48618.8	30447.9	18171.0	37.5	*	36059.0	18990.7	17068.3	47.8	40931.				
14	*178	240	24.29	1.31	67	*	48618.8	29589.8	19029.0	39.2	*	36059.0	18349.2	17709.7	49.4	42469.				
15	*178	240	24.29	1.31	78	*	48618.8	28881.5	19737.3	40.8	*	36059.0	17822.5	18236.5	50.6	43733.				
16	*178	240	24.29	1.57	56	*	48618.8	32186.3	16432.6	33.7	*	36059.0	18990.7	17068.3	47.8	40931.				
17	*178	240	24.29	1.57	67	*	48618.8	31279.3	17339.6	35.8	*	36059.0	18349.2	17709.7	49.4	42469.				
18	*178	240	24.29	1.57	78	*	48618.8	30530.5	18088.3	37.5	*	36059.0	17822.5	18236.5	50.6	43733.				
19	*178	240	31.06	1.05	56	*	62169.7	40656.8	21512.9	34.6	*	46109.2	24466.4	21642.8	47.2	51901.				
20	*178	240	31.06	1.05	67	*	62169.7	39511.1	22658.6	36.7	*	46109.2	23639.9	22469.2	48.9	53883.				
21	*178	240	31.06	1.05	78	*	62169.7	38565.3	23604.4	37.9	*	46109.2	22961.3	23147.9	50.6	55510.				
22	*178	240	31.06	1.31	56	*	62169.7	43511.0	18658.7	30.0	*	46109.2	24466.4	21642.8	47.2	51901.				
23	*178	240	31.06	1.31	67	*	62169.7	42284.9	19884.8	32.1	*	46109.2	23639.9	22469.2	48.9	53883.				
24	*178	240	31.06	1.31	78	*	62169.7	41272.7	20897.0	33.7	*	46109.2	22961.3	23147.9	50.6	55510.				
25	*178	240	31.06	1.57	56	*	62169.7	45995.2	16174.5	26.2	*	46109.2	24466.4	21642.8	47.2	51901.				
26	*178	240	31.06	1.57	67	*	62169.7	44699.1	17470.5	28.3	*	46109.2	23639.9	22469.2	48.9	53883.				
27	*178	240	31.06	1.57	78	*	62169.7	43629.1	18540.5	30.0	*	46109.2	22961.3	23147.9	50.6	55510.				

NOTE: # IN = POUNDS ENTERING
OUT = POUNDS LEAVING
REM = POUNDS REMOVED
% = PERCENTAGE REMOVED FOR THAT UNIT
SLUDGE=GALLONS OF SLUDGE PRODUCED ASSUMING 5.0% SOLIDS
O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED PLANT OPERATION
MIN=15.70,MAX=31.06

TRICKLING FILTER CHARACTERISTICS

TRIAL #	SS	INPUT VECTORS		RECIRC	T	#	# IN	# OUT	BOD	OUTPUT				# REM	SUSPENDED	# REM	SOLIDS	OVERALL	S.S.	REMARK
		BOD	FLOW							# REM	%	#	IN	# OUT	%	# REM	%	BOD	%	
1	*178	240	15.70	1.05	56	*	15096.8	12553.4	2543.3	16.8	*	12112.6	12145.1	732.5	-0.3	61.1	49.5			
2	*178	240	15.70	1.05	67	*	14671.3	11049.0	3622.3	24.7	*	11703.4	10793.5	910.0	7.8	66.0	54.9			
3	*178	240	15.70	1.05	78	*	14320.1	9915.9	4404.3	30.8	*	11367.5	9766.2	1601.2	14.1	69.8	59.6			
4	*178	240	15.70	1.31	56	*	16156.6	12850.0	3306.6	20.5	*	12112.6	11731.1	381.5	3.1	65.5	56.6			
5	*178	240	15.70	1.31	67	*	15701.3	11310.1	4391.3	28.0	*	11703.4	10425.5	1277.9	10.9	70.1	62.0			
6	*178	240	15.70	1.31	78	*	15325.5	10150.1	5175.3	33.8	*	11367.5	9433.3	1934.2	17.0	73.4	65.7			
7	*178	240	15.70	1.57	56	*	17079.1	13097.9	3981.1	23.3	*	12112.6	11402.8	709.8	5.9	68.8	62.2			
8	*178	240	15.70	1.57	67	*	16597.8	11528.3	5069.5	30.5	*	11703.4	10133.8	1569.6	13.4	73.1	67.4			
9	*178	240	15.70	1.57	78	*	16200.5	10346.0	5854.5	36.1	*	11367.5	9169.3	2198.1	19.3	76.1	70.3			
10	*178	240	24.29	1.05	56	*	28050.5	21678.5	6772.1	31.0	*	18349.2	15397.0	2952.3	16.1	61.9	58.5			
11	*178	240	24.29	1.05	67	*	27648.8	19080.5	8568.3	36.5	*	17822.5	13731.7	3890.8	21.8	66.0	62.8			
12	*178	240	24.29	1.05	78	*	26986.9	17123.7	9863.2	37.1	*	18990.7	16734.5	2256.2	11.9	61.1	60.6			
13	*178	240	24.29	1.31	56	*	30447.9	22190.6	8257.2	34.0	*	18349.2	14872.1	3477.1	18.9	66.2	65.2			
14	*178	240	24.29	1.31	67	*	29589.8	19531.3	10058.5	39.3	*	17822.5	13456.7	4365.8	24.5	70.1	68.9			
15	*178	240	24.29	1.31	78	*	28881.5	17528.3	11353.3	39.3	*	18990.7	16266.2	2724.5	14.3	64.6	65.8			
16	*178	240	24.29	1.57	56	*	32186.3	22618.8	9567.4	29.7	*	18990.7	14456.0	3893.3	21.2	69.5	70.2			
17	*178	240	24.29	1.57	67	*	31279.3	19908.2	11371.1	36.4	*	18349.2	13080.2	4742.3	26.6	73.1	73.6			
18	*178	240	24.29	1.57	78	*	30530.5	17866.5	12664.0	41.5	*	17822.5	13080.2	3302.8	13.5	53.7	55.5			
19	*178	240	31.06	1.05	56	*	40656.0	29491.4	11165.3	27.5	*	24466.4	21163.5	4831.6	20.4	59.5	60.7			
20	*178	240	31.06	1.05	67	*	39511.1	25957.2	13553.9	34.3	*	23639.9	18808.3	5943.0	25.9	63.6	64.5			
21	*178	240	31.06	1.05	78	*	38565.3	23295.1	15270.1	39.6	*	22961.3	17018.3	4024.3	16.4	58.3	62.0			
22	*178	240	31.06	1.31	56	*	43511.0	30188.2	13322.8	30.6	*	24466.4	20442.1	5472.8	23.2	64.0	66.8			
23	*178	240	31.06	1.31	67	*	42284.9	26570.5	15714.4	37.2	*	23639.9	18167.2	6523.1	28.4	67.8	70.4			
24	*178	240	31.06	1.31	78	*	41272.7	23845.6	17427.1	42.2	*	22961.3	16438.2	4596.3	18.8	61.9	67.8			
25	*178	240	31.06	1.57	56	*	45995.2	30770.6	15224.6	33.1	*	24466.4	19870.1	5981.1	25.3	67.3	71.7			
26	*178	240	31.06	1.57	67	*	44699.1	27083.2	17616.0	39.4	*	23639.9	17658.8	6983.2	30.4	71.3	75.0			
27	*178	240	31.06	1.57	78	*	43629.1	24305.6	19323.6	44.3	*	22961.3	15978.2							

NOTE: # IN =POUNDS ENTERING
OUT =POUNDS LEAVING
REM =POUNDS REMOVED
% =PERCENTAGE REMOVED FOR THAT UNIT
OVER, % REM=OVERALL EFFICIENCY COMPARING THE OUTPUT FOR
THIS UNIT INPUT AT STATION # 2
O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70, MAX=31.06

SECONDARY TANK CHARACTERISTICS

TRIAL#	INPUT VECTORS				OUTPUT				BOD				SUSPENDED				SOLIDS				PLANT EFF		REMARK
	*SS	BOD	FLOW	RECIRC	T	#	IN	#	OUT	#	REM	%	#	IN	#	OUT	#	REM	%	BOD	SS		
1	*178	240	15.70	1.05	56	*11825.5	7097.6	4727.9	40.0	*11327.6	7960.6	3367.0	29.7	12616.	76.9	64.8				76.9	64.8		
2	*178	240	15.70	1.05	67	*10331.1	6372.6	3958.5	38.3	*10082.1	7047.8	3034.3	30.1	11370.	79.4	69.2				79.4	69.2		
3	*178	240	15.70	1.05	78	*9459.3	5937.4	3521.8	37.2	*9334.8	6502.5	2832.3	30.3	10613.	81.0	71.6				81.0	71.6		
4	*178	240	15.70	1.31	56	*9673.7	5236.7	4437.0	45.9	*8776.1	5823.1	2953.0	33.6	11065.	81.7	71.7				81.7	71.7		
5	*178	240	15.70	1.31	67	*8476.3	4700.2	3776.1	44.5	*7778.2	5132.6	2645.6	34.0	9913.	83.7	75.5				83.7	75.5		
6	*178	240	15.70	1.31	78	*7777.7	4402.2	3375.5	43.4	*7279.1	4788.7	2490.4	34.2	9332.	84.8	77.1				84.8	77.1		
7	*178	240	15.70	1.57	56	*8234.4	4096.2	4138.2	50.3	*7153.1	4529.0	2624.1	36.7	9833.	84.7	76.5				84.7	76.5		
8	*178	240	15.70	1.57	67	*7235.6	3676.8	3558.8	49.2	*6320.8	3979.4	2341.3	37.0	8773.	86.5	80.1				86.5	80.1		
9	*178	240	15.70	1.57	78	*6652.9	3482.7	3170.2	47.7	*6078.8	3815.0	2255.8	37.2	8452.	87.6	80.9				87.6	80.9		
10	*178	240	24.29	1.05	56	*20410.6	13048.1	7366.4	36.1	*16177.6	12646.5	3531.1	21.8	13231.	72.8	64.3				72.8	64.3		
11	*178	240	24.29	1.05	67	*17909.7	11814.1	6095.6	34.0	*14635.9	11389.1	3246.8	22.2	12166.	75.3	67.8				75.3	67.8		
12	*178	240	24.29	1.05	78	*15983.1	10795.2	5187.9	32.5	*13287.1	10294.0	2993.2	22.5	11215.	77.3	71.0				77.3	71.0		
13	*178	240	24.29	1.31	56	*16664.2	9607.0	7057.2	42.3	*12498.2	9223.4	3274.8	26.2	12271.	77.9	71.6				77.9	71.6		
14	*178	240	24.29	1.31	67	*14657.1	8742.2	5915.0	40.4	*11417.1	8390.8	3026.3	26.5	11340.	80.3	74.3				80.3	74.3		
15	*178	240	24.29	1.31	78	*13113.4	7988.5	5124.9	39.1	*10336.4	7562.1	2774.3	26.8	10395.	82.3	76.9				82.3	76.9		
16	*178	240	24.29	1.57	56	*14155.8	7501.7	6654.1	47.0	*10166.5	7158.6	3007.9	29.6	11270.	81.5	76.5				81.5	76.5		
17	*178	240	24.29	1.57	67	*12481.6	6827.4	5654.2	45.3	*9264.7	6496.0	2768.7	29.9	10374.	83.5	79.0				83.5	79.0		
18	*178	240	24.29	1.57	78	*11194.0	6238.3	4955.6	44.3	*8363.3	5836.6	2526.7	30.2	9468.	85.3	81.4				85.3	81.4		
19	*178	240	31.06	1.05	56	*27828.8	18379.2	7692.6	34.0	*19701.8	16350.8	3350.9	17.0	12556.	69.9	63.7				69.9	63.7		
20	*178	240	31.06	1.05	67	*24379.3	16686.7	7692.6	31.6	*17976.7	14856.8	3119.8	17.4	11690.	72.9	67.2				72.9	67.2		
21	*178	240	31.06	1.05	78	*21915.6	15272.5	6643.0	30.3	*16252.0	13369.8	2882.2	17.7	10800.	74.9	70.5				74.9	70.5		
22	*178	240	31.06	1.31	56	*22690.4	13584.8	9105.6	40.1	*15390.0	12063.7	3326.3	21.6	12464.	75.5	70.7				75.5	70.7		
23	*178	240	31.06	1.31	67	*19926.4	12335.2	7591.3	38.1	*14007.7	10933.0	3074.7	21.9	11521.	78.1	73.9				78.1	73.9		
24	*178	240	31.06	1.31	78	*17952.3	11287.5	6664.7	37.1	*12625.8	9807.8	2818.0	22.3	10559.	80.1	77.0				80.1	77.0		
25	*178	240	31.06	1.57	56	*19253.6	10598.3	8655.3	45.0	*12506.6	9353.6	3153.0	25.2	11814.	79.4	76.3				79.4	76.3		
26	*178	240	31.06	1.57	67	*16948.1	9623.6	7324.4	43.2	*11353.6	8453.7	2899.8	25.5	10866.	81.8	78.7				81.8	78.7		
27	*178	240	31.06	1.57	78	*15136.8	8760.4	6376.4	42.1	*10200.9	7558.4	2642.5	25.9	9901.	83.6	81.6				83.6	81.6		

NOTE: # IN =POUNDS ENTERING
OUT =POUNDS LEAVING
% =PERCENTAGE REMOVED FOR THAT UNIT
SLUDGE=GALLONS OF SLUDGE PRODUCED ASSUMING 3.2% SOLIDS
PLANT EFF= OVERALL PLANT EFFICIENCY
O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70,MAX=31.06

COMPUTER SIMULATION OF TRICKLING FILTER OPERATION
ALEXANDRIA SANITATION AUTHORITY HIGH RATE TRICKLING FILTER SYSTEM

SET # 9 : SS =278 MG/L
BOD =240 MG/L

O U T P U T S T R E A M V E C T O R S

INPUT VECTORS

TRIAL#	#	*SS	BOD	FLOW	RECIRC	T	STATION # 2			STATION # 3			STATION # 4			STATION # 5			STATION # 7			STATION # 8			REMARK
							*FLOW	BOD	SS	FLOW	BOD	SS	FLOW	BOD	SS	FLOW	BOD	SS	FLOW	BOD	SS	FLOW	BOD	SS	
1		*278	240	15.70	1.05	56	*14.95	246	286	0.75	114	114	15.70	240	278	15.65	121	114	14.90	114	114	14.89	67	80*	
2		*278	240	15.70	1.05	67	*14.95	246	286	0.75	100	101	15.70	240	278	15.65	118	110	14.90	100	101	14.89	60	71*	
3		*278	240	15.70	1.05	78	*14.95	247	287	0.75	90	91	15.70	240	278	15.65	115	107	14.90	90	91	14.89	55	63*	
4		*278	240	15.70	1.31	56	*11.98	278	330	3.72	117	110	15.70	240	278	15.65	130	114	11.93	117	110	11.92	62	73*	
5		*278	240	15.70	1.31	67	*11.98	282	333	3.72	103	98	15.70	240	278	15.65	126	110	11.93	103	98	11.92	56	65*	
6		*278	240	15.70	1.31	78	*11.98	285	336	3.72	92	88	15.70	240	278	15.65	123	107	11.93	92	88	11.92	51	58*	
7		*278	240	15.70	1.57	56	*10.00	308	375	5.70	119	107	15.70	240	278	15.65	137	114	9.95	119	107	9.94	58	68*	
8		*278	240	15.70	1.57	67	*10.00	316	302	5.70	105	95	15.70	240	278	15.65	133	110	9.95	105	95	9.94	52	60*	
9		*278	240	15.70	1.57	78	*10.00	323	387	5.70	94	86	15.70	240	278	15.65	130	107	9.95	94	86	9.94	48	54*	
10		*278	240	24.29	1.05	56	*23.13	245	286	1.16	128	105	24.29	240	278	24.21	148	115	23.05	128	105	23.04	80	82*	
11		*278	240	24.29	1.05	67	*23.13	246	287	1.16	112	93	24.29	240	278	24.21	143	111	23.05	112	93	23.04	72	73*	
12		*278	240	24.29	1.05	78	*23.13	246	287	1.16	101	84	24.29	240	278	24.21	140	108	23.05	101	84	23.04	66	65*	
13		*278	240	24.29	1.31	56	*18.54	273	332	5.75	131	101	24.29	240	278	24.21	158	115	18.46	131	101	18.45	74	75*	
14		*278	240	24.29	1.31	67	*18.54	278	336	5.75	115	90	24.29	240	278	24.21	154	111	18.46	115	90	18.45	66	66*	
15		*278	240	24.29	1.31	78	*18.54	282	339	5.75	103	81	24.29	240	278	24.21	150	108	18.46	103	81	18.45	60	59*	
16		*278	240	24.29	1.57	56	*15.47	300	380	8.82	133	98	24.29	240	278	24.21	167	115	15.39	133	98	15.38	69	69*	
17		*278	240	24.29	1.57	67	*15.47	310	386	8.82	117	87	24.29	240	278	24.21	162	111	15.39	117	87	15.38	62	61*	
18		*278	240	24.29	1.57	78	*15.47	316	391	8.82	105	79	24.29	240	278	24.21	158	108	15.39	105	79	15.38	57	55*	
19		*278	240	31.06	1.05	56	*29.58	245	286	1.48	136	100	31.06	240	278	30.96	165	116	29.48	136	100	29.47	88	83*	
20		*278	240	31.06	1.05	67	*29.58	246	287	1.48	119	89	31.06	240	278	30.96	160	112	29.48	119	89	29.46	79	74*	
21		*278	240	31.06	1.05	78	*29.58	246	287	1.48	107	80	31.06	240	278	30.96	157	109	29.48	107	80	29.46	72	66*	
22		*278	240	31.06	1.31	56	*23.71	271	334	7.35	139	97	31.06	240	278	30.96	177	116	23.61	139	97	23.59	81	76*	
23		*278	240	31.06	1.31	67	*23.71	276	337	7.35	122	86	31.06	240	278	30.96	172	112	23.61	122	86	23.59	73	67*	
24		*278	240	31.06	1.31	78	*23.71	280	339	7.35	110	78	31.06	240	278	30.96	168	109	23.61	110	78	23.59	67	61*	
25		*278	240	31.06	1.57	56	*19.78	295	382	11.28	142	94	31.06	240	278	30.96	187	116	19.68	142	94	19.67	76	70*	
26		*278	240	31.06	1.57	67	*19.78	305	389	11.28	125	83	31.06	240	278	30.96	181	112	19.68	125	83	19.67	68	62*	
27		*278	240	31.06	1.57	78	*19.78	312	393	11.28	112	76	31.06	240	278	30.96	177	109	19.68	112	76	19.67	63	56*	

NOTE: FLOWS ARE IN MGD
BOD & SS ARE IN MG/L
O.O.R.=MEANSFLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED OPERATION
MIN=15.70,MAX=31.06

INPUT VECTORS ***** OUTPUT PRIMARY TANK CHARACTERISTICS *****

TRIAL #	SS	BOD	FLOW	RECIRC	T	#	#	IN	B O D		#	REM	%	#	IN	#	OUT	SUSPENDED		#	REM	%	SLUDGE	REMARK
									#	OUT								#	OUT					
1	*278	240	15.70	1.05	56	*	31425.1		15869.2	1555.9	49.6	*	36400.8		14920.5	21480.3	59.0	5151.1						
2	*278	240	15.70	1.05	67	*	31425.1		15422.0	16003.1	50.8	*	36400.8		14416.5	21984.2	60.4	52720.						
3	*278	240	15.70	1.05	78	*	31425.1		15052.8	16372.3	52.1	*	36400.8		14002.7	22398.1	61.5	53712.						
4	*278	240	15.70	1.31	56	*	31425.1		16983.2	14441.9	45.8	*	36400.8		14920.5	21480.3	59.0	5151.1						
5	*278	240	15.70	1.31	67	*	31425.1		16504.7	14920.4	47.5	*	36400.8		14416.5	21984.2	60.4	52720.						
6	*278	240	15.70	1.31	78	*	31425.1		16109.6	15315.5	48.7	*	36400.8		14002.7	22398.1	61.5	53712.						
7	*278	240	15.70	1.57	56	*	31425.1		17952.9	13472.2	42.9	*	36400.8		14920.5	21480.3	59.0	5151.1						
8	*278	240	15.70	1.57	67	*	31425.1		17447.0	13978.1	44.6	*	36400.8		14416.5	21984.2	60.4	52720.						
9	*278	240	15.70	1.57	78	*	31425.1		17029.3	14395.8	45.8	*	36400.8		14002.7	22398.1	61.5	53712.						
10	*278	240	24.29	1.05	56	*	48618.8		29906.2	18712.7	38.3	*	56316.8		23393.1	32923.7	58.6	78954.						
11	*278	240	24.29	1.05	67	*	48618.8		29063.4	19555.4	40.4	*	56316.8		22603.0	33713.9	60.1	80849.						
12	*278	240	24.29	1.05	78	*	48618.8		28367.7	20251.1	41.7	*	56316.8		21954.1	34362.7	61.2	82405.						
13	*278	240	24.29	1.31	56	*	48618.8		32005.7	16613.2	34.2	*	56316.8		23393.1	32923.7	58.6	78954.						
14	*278	240	24.29	1.31	67	*	48618.8		31103.8	17515.1	35.8	*	56316.8		22603.0	33713.9	60.1	80849.						
15	*278	240	24.29	1.31	78	*	48618.8		30359.2	18259.6	37.5	*	56316.8		21954.1	34362.7	61.2	82405.						
16	*278	240	24.29	1.57	56	*	48618.8		33833.0	14785.8	30.4	*	56316.8		23393.1	32923.7	58.6	78954.						
17	*278	240	24.29	1.57	67	*	48618.8		32879.6	15739.2	32.5	*	56316.8		22603.0	33713.9	60.1	80849.						
18	*278	240	24.29	1.57	78	*	48618.8		32092.6	16526.3	34.2	*	56316.8		21954.1	34362.7	61.2	82405.						
19	*278	240	31.06	1.05	56	*	62169.7		42737.0	19432.7	31.3	*	72013.2		30138.1	41875.1	58.3	100420.						
20	*278	240	31.06	1.05	67	*	62169.7		41532.4	20637.0	33.3	*	72013.2		29120.1	42893.1	59.7	102861.						
21	*278	240	31.06	1.05	78	*	62169.7		40538.4	21631.2	34.6	*	72013.2		28284.2	43729.0	60.8	104866.						
22	*278	240	31.06	1.31	56	*	62169.7		45737.2	16432.4	26.2	*	72013.2		30138.1	41875.1	58.3	100420.						
23	*278	240	31.06	1.31	67	*	62169.7		44448.3	17721.3	28.3	*	72013.2		29120.1	42893.1	59.7	102861.						
24	*278	240	31.06	1.31	78	*	62169.7		43384.4	18785.3	30.0	*	72013.2		28284.2	43729.0	60.8	104866.						
25	*278	240	31.06	1.57	56	*	62169.7		48348.6	13821.1	22.1	*	72013.2		30138.1	41875.1	58.3	100420.						
26	*278	240	31.06	1.57	67	*	62169.7		46986.1	15183.6	24.6	*	72013.2		29120.1	42893.1	59.7	102861.						
27	*278	240	31.06	1.57	78	*	62169.7		45861.4	16308.3	26.2	*	72013.2		28284.2	43729.0	60.8	104866.						

NOTE: # IN =POUNDS ENTERING
OUT =POUNDS LEAVING
REM =POUNDS REMOVED
% =PERCENTAGE REMOVED FOR THAT UNIT
SLUDGE=GALLONS OF SLUDGE PRODUCED ASSUMING 5.0% SOLIDS
O.O.R.=MEANS FLOW THRU THE PLANT IS OUT OF THE
RANGE OF THE ESTABLISHED PLANT OPERATION
MIN=15.70,MAX=31.06

CHARACTERISTICS									
TRICKLING FILTER									
SOLIDS									
SUSPENDED									
OVERALL									
BOD									
REMARK									

TRIAL#	#	INPUT VECTORS			RECIRC	T	O U T P U T			S E C O N D A R Y			T A N K C H A R A C T E R I S T I C S			REMARK			
		BOD	FLOW				#	IN	#	OUT	B O D	%	#	IN	#		OUT	SUSPENDED	%
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