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Surface Loading of a Thin-walled Toroidal Shell .

by

Fan Zhang

Thesis submitted to School of Graduate Studies
as partial fulfillment of the requirements
for the degree of M.A.Sc. in
Mechanical Engineering

UNIVERSITY OF OTTAWA

 Fan Zhang, Ottawa, Canada, 1992.



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ABSTRACT

The present study is concerned with the series solution based on the Sanders shell theory for the linear elastic problems of the surface loading of thin-walled toroidal shells. The Sanders theory is considered to be one of the most accurate first order theories. For toroidal shells, series solutions have been given by several authors, using other theories and furthermore using a stress approach. In the present study a displacement approach is taken. The governing equations are first developed in toroidal coordinates. The loading case of a pad of uniform normal pressure is then considered in detail, and series expansions are written for the load, displacement and stress terms. Results are computed using the shell theory for sample problems. To check the accuracy of the theory, the results are compared with numerical results obtained using the Finite Element Method (FEM), the Mushtari-Vlasov-Donnel (MVD) and Flügge shell theories. There is a close agreement in the results. The Sanders shell theory is then applied to the problem of local loads on sectorial toroidal shells. The results are compared with results for corresponding cylindrical shells. Three tables are given summarizing results for characteristic displacements and stresses in a number of shells, covering a wide range of geometric parameters. The results given provide practical information for structural analysts and designers of piping and vessels, and furthermore give information about the Sanders shell theory and FEM solution characteristics.

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Nomenclature

a, b, c, d	constant coefficients
D	bending rigidity, defined as $Et^3/[12(1 - \nu^2)]$
E	Young's modulus
e_θ, e_η, e_r	base unit vectors at point P
K	extensional rigidity, defined as $Et/(1 - \nu^2)$
\bar{K}	pipe factor, defined as $[lt^{1/2}/r^{3/2}]^{1/2}$
i, j, k	unit vectors
l	center-line length of shell
M, N	truncation integers
$M_\theta, M_\eta, M_{\theta\eta}$	bending stress resultants
$N_\theta, N_\eta, N_{\theta\eta}$	membrane stress resultants
p	magnitude of normal pressure
Q_θ, Q_η	normal shear resultants
r	cross-sectional radius of shell
\mathbf{r}	radius vector
R	mean toroidal radius of shell
R_1, R_2	radii of curvature
t	thickness of shell
u, v, w	displacement components

x, y, z	Cartesian coordinates
α	longitudinal angular distance to center of load region from shell mid-length
β	circumferential angular distance to center of load region from extrados
γ	radius ratio, defined as r/R
Γ	pipe factor, defined as r^2/Rt
δ	longitudinal angular length of load region
$\epsilon_\theta, \epsilon_\eta, \epsilon_{\theta\eta}$	strain components
ζ	$= 1 + \gamma \cos\theta$
η	toroidal coordinate, defined as $\rho\phi$
θ	circumferential angular coordinate
$\kappa_\theta, \kappa_\eta, \kappa_{\theta\eta}$	change of curvature components
λ	circumferential angular length of load region
μ_m	$= m\pi\gamma/\psi$
ν	Poisson ratio
$\nu_1 - \nu_{41}$	coefficients for the Sanders theory
ξ_1, ξ_2	<i>Lamé</i> parameters
ρ	radius ratio, defined as R/r
$\sigma_\theta, \sigma_\eta$	direct stress components
ϕ	longitudinal angular coordinate
ψ	angular length of shell
ϕ'	$= \psi/2$
Ω	pipe factor, defined as $lt^{1/2}/r^{3/2}$

Chapter 1

Introduction

1.1 Thin-walled Toroidal Shell

Because of its specific geometrical configuration, the toroidal shell is utilized in many areas of mechanical engineering. Thin-walled toroidal shells (sectorial or complete) are widely used in nuclear and other power industries. Examples include Tokamak-type fusion reactors, cooling towers, piping elbows and pipe bends (Figs. 1.1-1.4). Trunnion supports (Fig. 1.5), ram bending (Fig. 1.6), construction attachments, and accidental loads are among the causes of surface loading of toroidal shells. A toroidal shell is much more flexible in bending than a cylindrical shell. Therefore, it is of great importance to examine the effects of stress and displacement of a toroidal shell.

The general methods for resolving the problem of a toroidal shell are the series method and the finite element method. The series solution is generally more efficient and more accurate in calculations. Although the finite element method is often more convenient than the series method, it is only a discrete approximate value. In the present study,

a series solution is used based on the linear Sanders shell theory [1]. This theory is considered to be one of the most accurate first order theories [2].

1.2 The Sanders Shell Theory

This theory was originally derived by Sanders in 1959, on the basis of Love's [3] first-approximation theory, in which all of Love's original assumptions are preserved. In contrast to the results of Love's theory, all strains in the proposed theory vanish for rigid-body motions. This theory is recognized as one of the most accurate first order theories. The full theoretical development, in toroidal coordinates, is given in Chapter 3.

1.3 Mushtari-Vlasov-Donnel (MVD) Shell Theory

This theory was developed from Love's theory on the basis of order-of-magnitude considerations and is much easier to get results than the Sanders theory. It is therefore preferable in engineering applications if its accuracy is satisfactory for the purposes of engineering. The full displacement development, in toroidal coordinates, was given by Redekop [4].

1.4 Finite Element Method (FEM)

The finite element method is a powerful and popular technique for the numerical solution of a variety of problems in engineering. The fundamental concept of this method is

that any continuous quantity can be approximated by a discrete model having a finite number of degrees of freedom. The finite element method combines several mathematical concepts to produce a system of linear or non-linear equations. The number of equations is usually very large, anywhere from twenty thousand or more, and requires the computational power of the digital computer.

Numerous commercial software packages are now available for FEM implementation. The commercial program ADINA [5] is one of them. ADINA (Automatic Dynamic Incremental Non-linear Analysis) was developed at MIT (Massachusetts Institute of Technology) by Klaus-Jürgen Bathe. It is used in displacement and stress analysis. The isoparametric thin shell element and shell pressure loading are available in these computer programs. There are three displacement and two rotation degrees of freedom at each node. The element was used here as part of a linear elastic analysis.

In general, a proper finite element solution should converge and the accuracy of the calculation usually (but not always) increases as the number of degrees of freedom in the model increases.

The phenomenon of an element being much too stiff is referred to in the literature as element locking [5]. In essence, the phenomenon arises because the interpolation functions used for an element are not able to represent zero (or very small) shearing or membrane strains. If the element cannot represent zero shearing strains, but the physical situation corresponds to zero (or very small) shearing strains, then the element becomes very stiff as its thickness over length ratio decreases. Namely, for thin shells when full integration is used to evaluate the stiffness matrix, over stiff solutions are often produced owing to shear and membrane locking.

1.5 Outline of Present Investigation

The present study is concerned mainly with the series solution based on the Sanders shell theory. Attention is given to the problems of surface loading of a thin-walled toroidal shell. The study is carried out with two main objectives:

First, to develop Sanders shell theory in toroidal coordinates in a displacement form. In this study, the loading case of a pad of uniform normal pressure on the toroidal shell surface is considered in detail. The results are obtained by means of a computer program based on the Sanders shell theory. To check the accuracy of the theory, the results are compared with numerical results of some other method and shell theories.

Second, to provide practical information for toroidal shells, detailed displacement and stress results are given for a specific toroidal shell with loadings centered at three positions; the crown circles, the extrados, and the intrados. These results are compared with results for a corresponding cylindrical shell. Summaries of results are given for displacements and stresses in a number of toroidal shells, covering a wide range of geometric parameters.

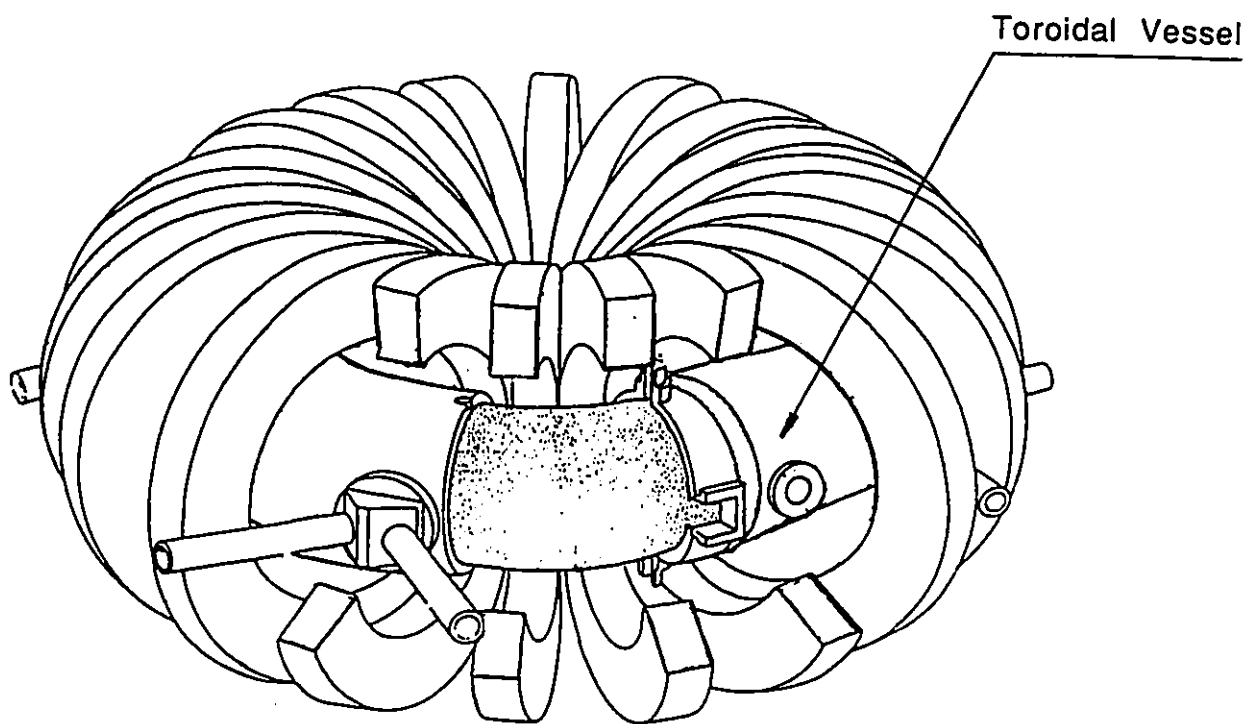


Figure 1.1: Tokamak-type fusion reactor

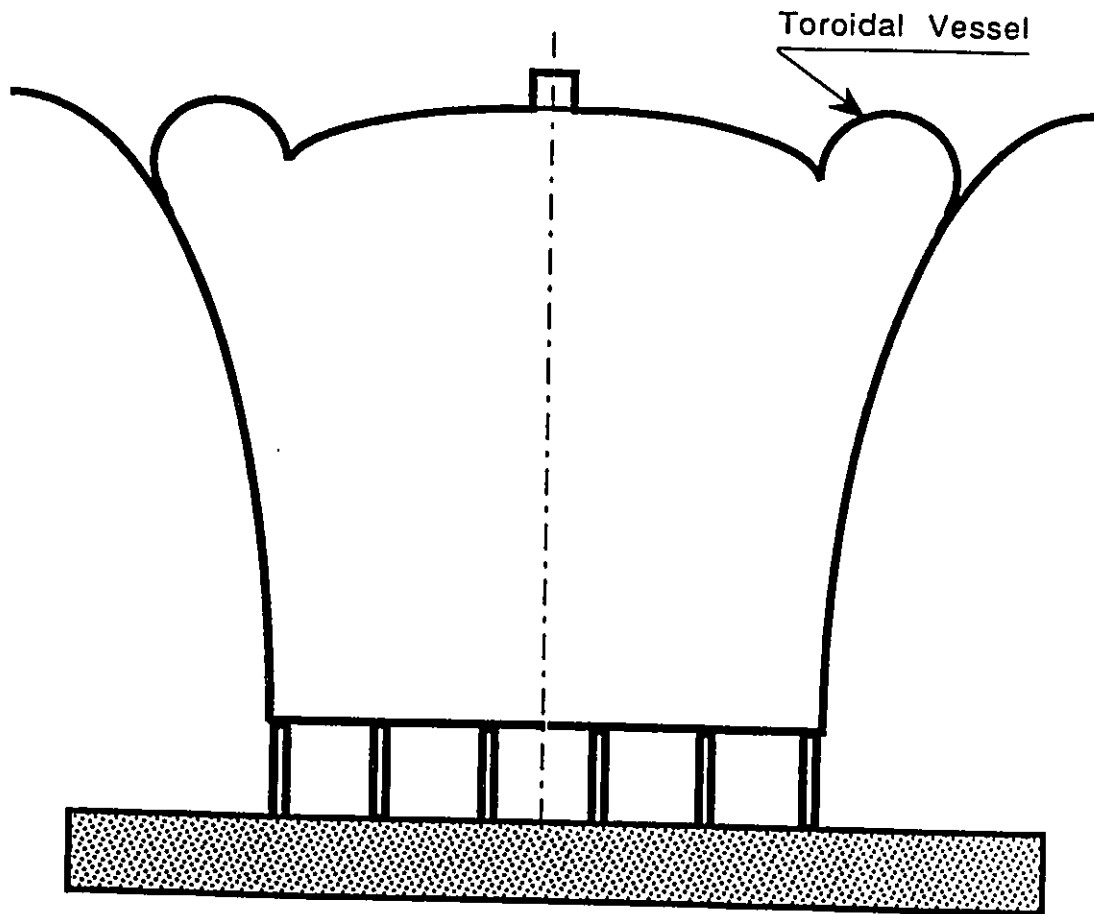


Figure 1.2: Schematic view of cooling tower

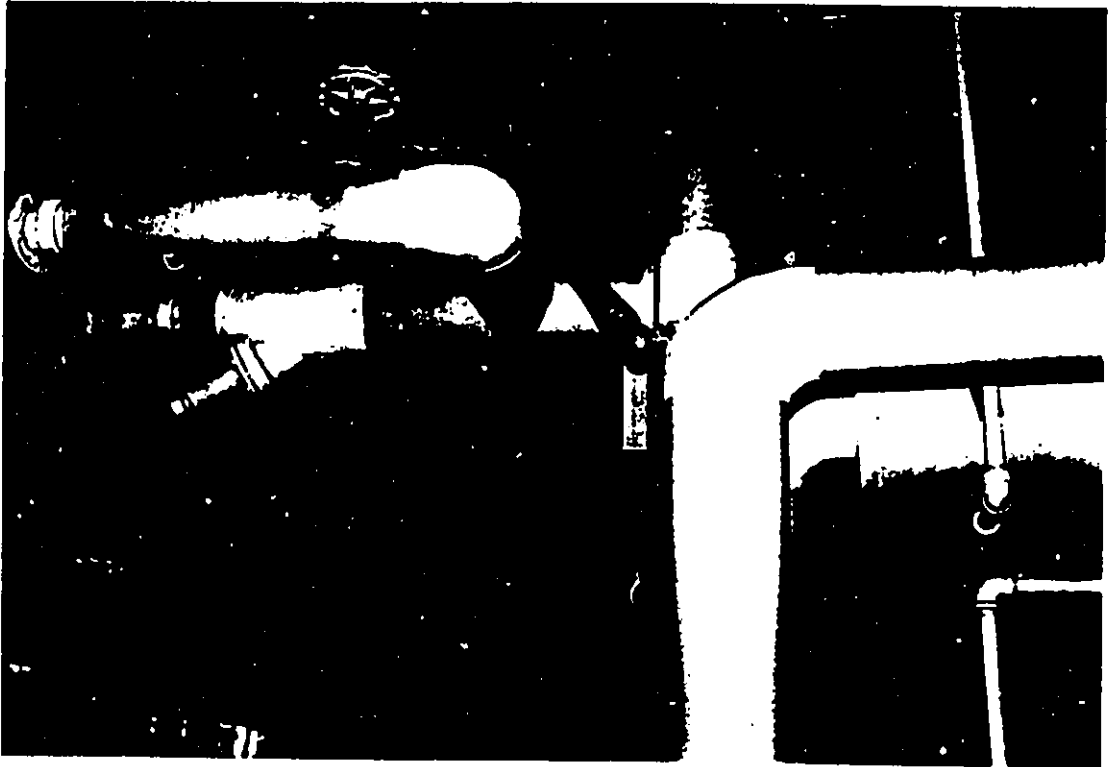


Figure 1.3: Piping elbows

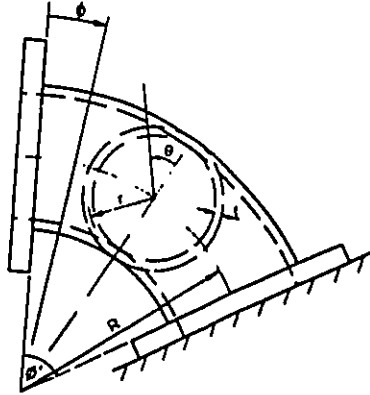


Figure 1.4: Flange-ended pipe bend

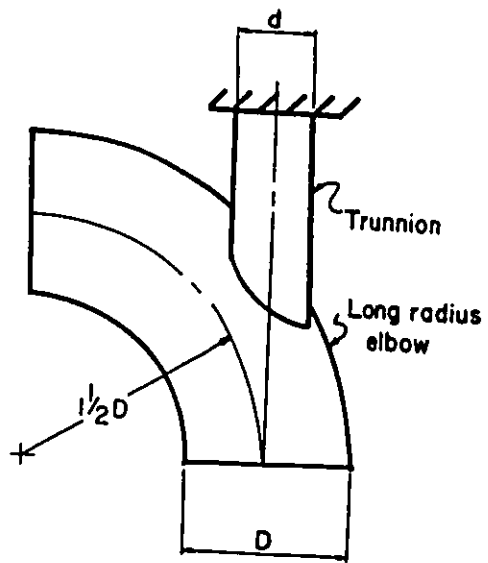


Figure 1.5: Trunnion elbow support

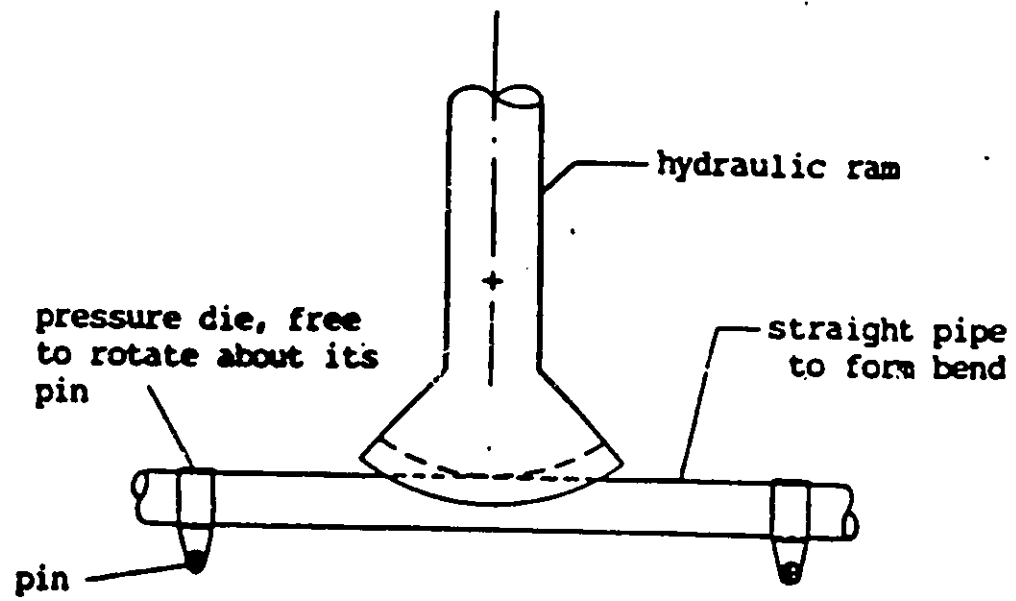


Figure 1.6: Schematic view of ram bending

Chapter 2

Literature Survey

In this chapter a survey of the literature of the linear thin-walled models of the state of stress and displacement in toroidal shells (sectorial or complete) is presented. Emphasis is placed on models using the shell bending theory and the FEM.

2.1 Previous Work on Thin Shell Theory Solutions

The early models developed were for thin-walled pipe bends, and were of the one-variable type accounting for variation of stress values in the circumferential direction (of the cross-section) only. Results thus could be exact only for complete, axisymmetric toroidal shells, as there is no treatment of end conditions.

Two-variable models are required to account for a longitudinal variation of stresses, in addition to circumferential. Such models can account for the effect of end restraints in sectorial toroidal shells, such as elbows and pipe bends with terminal flanges or tan-

gent pipes. For classical geometries involving circular cross-sections and constant wall thickness, series solutions are effective. An early such solution [6] covered U-bends with fixed ends and was based on the energy method. In Thailer and Cheng presented paper, a general solution for a thin-walled pipe bend containing rigid end constraints, subjected to in-plane bending moments. Inextensibility of the meridional center line is assumed. The displacements are expanded in series form and the equations of equilibrium are guaranteed by minimizing the total potential energy. The minimization is achieved in the manner of Rayleigh-Ritz.

Linear flexure of pipe bends with flanged tangents has been comprehensively studied by Whatham and Thompson [7]. Their original solution used as dependent variables a set of the stress functions. It included Fourier series expansion in the circumferential direction and computation of a substantial number of eigenvalues for the 'end' solution. The flexibility factor was represented in graphs for 90° and 180° bends. The same problem was treated by Thomson, Spence and Findlay [8-10] by means of a trigonometric-series solution, minimizing the total potential energy.

A complete set of series solutions, based on the Novozhilov [11] equations, has been presented by Whatham [12-15]. The system equations were presented in the first of these papers. This paper gives the equations, in a form suitable for computation, for the pressurizing of pipe bends and, by superposition, for any combination of forces and moments acting on the ends, neglecting end effects for the present. The 'end effect' was covered in the second paper. Where a number of solutions of the basic equations which represent the effect of self-equilibrating end loads, decaying with distance from the ends, were developed. These may be added to the earlier solution to enable the constraints imposed by the flanges to be satisfied and yield the complete flanged bend solution. Further results, and discussion of a general computer program based on the theory, were given in last two papers [14,15]. In the first of these latter two papers, thin shell

theory is applied to pipe bends terminated by flanges or flange-ended tangent pipes and subjected to any end loading, either in-plane or out-of-plane. Graphs of flexibility factor versus pipe bend characteristic are presented for in-plane bending of a wide range of pipe elbows terminated by flanges or short flange-ended tangents. Experimental results verify the thin shell solutions for in-plane and out-of-plane bending of a flanged pipe elbow.

Further shell theory solutions for cylindrical and toroidal shells have been reviewed by Axelrad and Emmerling [20], and Redekop, Colle, and Nguyen [21]. Recently, Whatham [22] conducted a series of studies using the semi-membrane theory to determine the effect of end loads on pipe bends and elbows. A considerable portion of his work was concerned with the determination of the flexibility in bending of a toroidal shell compared with a cylindrical shell. Thompson and Spence [23] have presented a solution employing the theorem of minimum potential energy, with suitable kinematically admissible displacements. Hui and Wei [24] have presented results for toroidal shells based on the Novozhilov equations for a number of problems involving end loadings.

The effect of local loads on cylindrical shells has been studied extensively [16-19]. In [17], a Fourier series solution was detailed which enabled displacement and stresses to be obtained in the region of local loads on cylindrical vessels. To facilitate the analysis of a general load system, in the radial, tangential and longitudinal directions, a compendium of 'loading term' solutions is presented. Redekop and Azar [19] have considered the ram bending of a straight cylindrical pipe using the shell theories and FEM. The loading is idealized as a set of pads of uniform radial pressure, and results are given for the elastic range. Results for local loads represent to varying degrees of accuracy the effect of attachments, lugs, supports, blast loading, or projectile impact on the surface of the shells.

Toroidal shells are also subject to local loads, although to a lesser extent than cylin-

dricial shells. Trunnion supports, construction attachments, and accidental loads are among the causes of local loads on the surface of toroidal shells. In contrast to the case of cylindrical shells, the literature for toroidal shells subject to local loads is relatively sparse. Redekop [4] has used the Mushtari-Vlasov-Donnel (MVD) [2] shell theory to obtain solutions for local loads on a toroidal shell. The problem of ram bending of a curved pipe in the linear elastic range is considered in the study.

2.2 Finite Element Modeling for Thin Shell Element

With the development of fast and powerful digital computers in recent years, the FEM has played a predominant role in numerical solutions.

The linear two-variable FEM models for toroidal shells are of two main types. The first type, employing 'elbow' elements, features a shell-type variation of values in the circumferential direction and a beam-type variation in the longitudinal direction. The second type of model, employing 'shell' elements, features shell type behavior in both directions.

A number of elbow elements have been developed for practical use. The cost of an analysis using these elements is substantially reduced. However, the elements only provide moderate accuracy.

In view of the foregoing, shell elements have been chosen by a large number of researchers in recent years. There is greater accuracy of the results using these elements. The earlier attempts to develop a general shell element were made by, among others, Gallagher [25] in 1966. He presented a quadrilateral shell element with two constant

principal radii of curvature, and Connor and Brebbia [26] in 1967 introduced a rectangular element based on shallow shell theory. Both elements used only linear distributions for membrane displacements and consequently did not include all the required rigid body modes. The resulting elements are too stiff and not efficient enough. Brebbia and Deb Nath [27] in 1970 reviewed the shell finite elements and compared their relative convergence rates with an established finite difference solution for a clamped hyper shell under uniformly distributed normal load. Yang [28] in 1973 developed a high order shell finite element with two principal radii of curvature and a twist radius. Yang and Kapania [29] in 1983 extended this element to have the geometry of a quadrilateral defined by principal lines of curvature of the shell.

A significant contribution to shell analysis was the definition of geometry using variable order polynomials and rational B-spline functions [30]. This allowed the modeling of a wide variety of shell structures with arbitrary distributions of curvatures. Recently, Hansen and Heppler [31] presented a method to enable the shell-coordinate finite elements to exactly reproduce all of the six Cartesian rigid body modes. This was accomplished by forcing the rigid-body capability as an essential requirement for the derivation of the basis functions. The method was applied to shells of revolution with straight meridians. This work was extended by Heppler *et al.* [32] to include the more general situation of arbitrary shells of revolution.

Most of the quadrilateral elements published in the literature are limited to regular shapes. This may impose a limitation on the application for this kind of elements. Recently, tensor mathematics has been used in the formulation of curved quadrilateral shell elements to overcome such a limitation [33,34].

Kirchhoff shell theory has been widely used in the formulation of curved thin shell elements due to its well-established nature. However, such a formulation entails some limitations [35]. From the considerations of the principle of minimum potential energy,

Kirchhoff theory requires the continuity of the normal slope across the inter-element boundaries for the formulation of conforming shell elements. Irons and Draper [36] showed that it was impossible to use the Kirchhoff theory to formulate a conforming triangular plate or shell element with simple nodal connections.

Such limitations have led to the development of another type of popular shell element: the 'degenerated' shell elements, which are degenerated from the three-dimensional isoparametric formulations with additional consideration to incorporate the Kirchhoff hypothesis.

With a view to extending the isoparametric element concept, Ahmad *et al.* [37] first presented the degenerated shell element. Ahmad degenerated a three-dimensional brick element into a general curved shell element which has nodes only at the midsurface. The stress in the thickness direction is assumed to be equal to zero and the element displacement field is expressed in terms of three displacements of the midsurface, two rotations of the midsurface 'normal' and appropriate shape functions. Bathe [38], Crisfield [39] and Hughes [40] offer comprehensive overviews of the degenerated solid approach and related methodologies which involve some type of reduction to resultant form.

The original degenerated shell element performs reasonably well for moderately thick shell situations. However, for thin shells when full integration is used to evaluate the stiffness matrix, over stiff solutions are often produced due to shear or membrane locking.

Locking has been the subject of analysis by many authors in an attempt to both explain the phenomena and eventually remedy them. Attempts have been made to correct this behavior by use of reduced or selective integration techniques. Such schemes are not always successful in overcoming locking behaviour and the resulting solutions may still be over stiff for problems with highly constrained boundaries, and element mechanisms or spurious zero energy modes may form. These mechanisms can spread

from element to element causing either rank deficiency in the overall stiffness matrix and consequently no solution, or the even more dangerous situation in which the solution obtained may be polluted by a near mechanism.

Shear locking is the locking phenomenon associated with the development of spurious transverse shear strain. Shear locking in shell elements has been circumvented by the use of selective or reduced integration. This may, however, lead to matrices that are rank deficient and possess spurious (or zero-energy) modes called hourglass modes. Stabilization matrices are used to remove the spurious modes. The stabilization matrices are given by Jacquotte and Oden [41] for linear analysis.

In addition to using the selective or reduced integration scheme, some investigators utilized other techniques to overcome the shear locking problem. For example, Razzaque [42,43], Baldwin *et al.* [44] and Irons [45] applied discrete Kirchhoff constraints which enforced the Kirchhoff constraint only at discrete Gaussian points. Crisfield [46] utilized shear constraints to formulate a discrete Kirchhoff element in which the transverse shear strains are effectively zero. Bathe and Dvorkin [33,47] and Park and Stanley [48] used the assumed strain concept (both transverse shear and membrane strains) and the curvilinear natural coordinate system to formulate a curved quadrilateral shell element, respectively. Huang and Hinton [49,50] and Jang and Pinsky [34] utilized the same assumed strain concept and tensorial representations to formulate degenerated curved quadrilateral shell elements.

Membrane locking is the locking phenomenon associated with the development of non-zero membrane strain under a state of constant curvature. Stolarski and Belytschko [51] found that reduced shear integration in curved C^0 elements does also mitigate the effects of membrane locking. They also found that reduced integration in either the shear or the membrane energy expressions leads to improved accuracy in bending response. However, reduced shear integration is accompanied by a deterioration of membrane-

flexural coupling which is one of the essential features of the behavior of a curved element.

The assumed strain concept has been another technique used to avoid membrane locking. Huang and Hinton [49,50] used an enhanced interpolation of the membrane strains to formulate their nine-node degenerated shell element to avoid membrane locking. Stolarski and Belytschko [52] demonstrated that when low order middle surface displacement functions are used in the formulation of curved elements, an artificial constraint appears that causes a bending-dominated response to be replaced by a membrane-dominated response. This effect, due to the inability of an element to bend without stretching, is termed membrane locking. Membrane locking may also be avoided if the middle surface displacement field of the finite element is of sufficiently high order [52].

Chapter 3

The Sanders Shell Theory

3.1 Introduction

In this chapter a series solution based on the linear Sanders shell theory [1] is presented. At all times linear elastic small-displacement behavior is assumed. The governing equations are developed in toroidal coordinates in a convenient displacement form. They are solved for the case of a pad load of uniform normal pressure. Such a loading gives a first approximation of the effect of a trunnion pipe on an elbow [53]. Further applications are for the problems of the surface loading of a toroidal shell during construction or fabrication, and for the ram bending of curved pipes. In Chapter 4 numerical results, obtained from the Sanders theory, are compared with results from the isoparametric FEM [5,38,54]. These results define shell theory and FEM solution characteristics. In Chapter 5 further results from the Sanders theory are given which serve to provide information about elastic curved pipe displacement and stress characteristics.

3.2 Shell Geometry

The toroidal shell is assumed loaded by a pad of uniform normal pressure on the outside surface (Fig. 3.1). This loading is symmetrical about the horizontal $x - y$ plane and about a transverse plane bisecting the shell. The shell has a toroidal radius of R , a cross-sectional radius of r , and a thickness of t (Fig. 3.2). A general point P on the shell mid-surface is defined by the circumferential and longitudinal angular coordinates θ and ϕ . The radius vector \mathbf{r} to P is given by

$$\mathbf{r} = (R + r\cos\theta)\sin\phi\mathbf{i} + (R + r\cos\theta)\cos\phi\mathbf{j} + r\sin\theta\mathbf{k} \quad (3.1)$$

where $\mathbf{i}, \mathbf{j}, \mathbf{k}$ are unit vectors forming a base in the Cartesian x, y, z coordinate system. The first fundamental form of the middle surface is given by

$$ds^2 = A_1^2 d\xi_1^2 + A_2^2 d\xi_2^2 \quad (3.2)$$

Quantity ds is the differential line element, A_1 and A_2 are the *Lamé* parameters, while ξ_1 and ξ_2 are the base coordinates. Adopting the convention used by Whatham [22] the base coordinates are taken as

$$\xi_1 = \theta; \xi_2 = \eta = \rho\phi \quad (3.3)$$

where $\rho = R/r$. Both η and ϕ are used in the following as variables of the longitudinal direction. The *Lamé* parameters are

$$A_1 = r; A_2 = r\zeta \quad (3.4)$$

where

$$\zeta = 1 + \gamma\cos\theta; \gamma = r/R = \rho^{-1} \quad (3.5)$$

The principal radius of curvatures for the circumferential (R_1) and longitudinal (R_2) directions are

$$R_1 = r; R_2 = r\zeta/(\gamma\cos\theta) \quad (3.6)$$

The set of base unit vectors at P (Fig. 3.2) is given by

$$\begin{aligned}
 \mathbf{e}_\theta &= -\sin\theta\sin\phi\mathbf{i} - \sin\theta\cos\phi\mathbf{j} + \cos\theta\mathbf{k} \\
 \mathbf{e}_\eta &= \cos\phi\mathbf{i} - \sin\phi\mathbf{j} \\
 \mathbf{e}_r &= \cos\theta\sin\phi\mathbf{i} + \cos\theta\cos\phi\mathbf{j} + \sin\theta\mathbf{k}
 \end{aligned} \tag{3.7}$$

The subscripts θ and η in eqn. (3.7) and in the following indicate respectively the shell circumferential and longitudinal directions, while r indicates the outward normal direction.

3.3 The Sanders Shell Theory

The shell theory of Sanders [1] is considered one of the most accurate first order theories. Mainly due to its relative complexity it has not been used previously in toroidal coordinates. The governing equations are written in terms of the three displacement components u, v , and w (Fig. 3.3). Membrane stress resultants $N_\theta, N_\eta, N_{\theta\eta}$, and bending stress resultants, $M_\theta, M_\eta, M_{\theta\eta}$, are defined in the theory, as well as normal shear resultants Q_θ, Q_η (Fig. 3.3).

The five equilibrium equations of this theory may be reduced, by eliminating the normal shear stress resultants, to the following:

$$\begin{aligned}
 &\frac{\partial(\zeta N_\theta)}{\partial\theta} + \frac{\partial N_{\theta\eta}}{\partial\eta} + \gamma\sin\theta N_\eta \\
 &+ \frac{1}{r} \frac{\partial(\zeta M_\theta)}{\partial\theta} + \frac{1 + (2\zeta)^{-1}}{r} \frac{\partial M_{\theta\eta}}{\partial\eta} + \frac{\gamma\sin\theta M_\eta}{r} = 0 \\
 &\frac{\partial(\zeta N_{\theta\eta})}{\partial\theta} + \frac{\partial N_\eta}{\partial\eta} - \gamma\sin\theta N_{\theta\eta} + \frac{\gamma\cos\theta}{r\zeta} \frac{\partial M_\eta}{\partial\eta} \\
 &+ \frac{2\gamma\cos\theta - 1}{2r} \frac{\partial M_{\theta\eta}}{\partial\theta} - \frac{2\gamma^2\sin 2\theta + \gamma\sin\theta}{2r\zeta} M_{\theta\eta} = 0
 \end{aligned} \tag{3.8}$$

$$\frac{N_\theta}{r} + \frac{\gamma \cos \theta}{\zeta r} N_\eta - \frac{1}{r^2} \frac{\partial^2 M_\theta}{\partial \theta^2} - \frac{1}{(r\zeta)^2} \frac{\partial^2 M_\eta}{\partial \eta^2} - \frac{2}{(r\zeta)^2} \frac{\partial}{\partial \theta} \left(\zeta \frac{\partial M_{\theta\eta}}{\partial \eta} \right) - \frac{\gamma \sin \theta}{r^2 \zeta} \frac{\partial (M_\eta - 2M_\theta)}{\partial \theta} - \frac{\gamma \cos \theta (M_\eta - M_\theta)}{r^2 \zeta} = q_r$$

where $q_r = q_r(\theta, \eta)$ is the normal surface loading (positive outward). Surface loadings in the circumferential and longitudinal directions are taken as zero in the present study.

The constitutive equations relating the stress resultants with the mid-surface strain and curvature functions are

$$\begin{aligned} N_\theta &= K(\epsilon_\theta + \nu \epsilon_\eta); \quad M_\theta = D(\kappa_\theta + \nu \kappa_\eta) \\ N_\eta &= K(\epsilon_\eta + \nu \epsilon_\theta); \quad M_\eta = D(\kappa_\eta + \nu \kappa_\theta) \\ N_{\theta\eta} &= K(1 - \nu)\epsilon_{\theta\eta}; \quad M_{\theta\eta} = D(1 - \nu)\kappa_{\theta\eta} \end{aligned} \quad (3.9)$$

where

$$D = Et^3/[12(1 - \nu^2)]; \quad K = Et/(1 - \nu^2) \quad (3.10)$$

and E = Young's modulus; ν = Poisson ratio; $\epsilon_\theta, \kappa_\theta$ = circumferential strain, curvature; $\epsilon_\eta, \kappa_\eta$ = longitudinal strain, curvature; and $\epsilon_{\theta\eta}, \kappa_{\theta\eta}$ = shear strain, curvature.

The shear stress resultants Q_θ, Q_η are available from the other resultants through

$$\begin{aligned} Q_\theta &= \frac{1}{r} \frac{\partial M_\theta}{\partial \theta} + \frac{1}{r\zeta} \frac{\partial M_{\theta\eta}}{\partial \eta} + \frac{\gamma \sin \theta (M_\eta - M_\theta)}{r\zeta} \\ Q_\eta &= \frac{1}{r} \frac{\partial M_{\theta\eta}}{\partial \theta} + \frac{1}{r\zeta} \frac{\partial M_\eta}{\partial \eta} - \frac{2\gamma \sin \theta M_{\theta\eta}}{r\zeta} \end{aligned} \quad (3.11)$$

The circumferential and longitudinal normal stress components $\sigma_\theta, \sigma_\eta$, on the surfaces of the shell, are found from the stress resultants by

$$\begin{aligned} \sigma_\theta &= N_\theta/t \pm M_\theta(6/t^2) \\ \sigma_\eta &= N_\eta/t \pm M_\eta(6/t^2) \end{aligned} \quad (3.12)$$

The plus sign in these relations applies to points on the shell outside surface, while the minus sign to points on the inside surface. The first term of the right hand members indicates in each case the membrane stress contribution and the second term the bending stress contribution.

The strain-displacement and curvature-displacement relations in the theory of Sanders [1], expressed in toroidal coordinates, are

$$\begin{aligned}
\epsilon_{\theta} &= \frac{1}{r} \left(\frac{\partial u}{\partial \theta} + w \right) \\
\epsilon_{\eta} &= \frac{1}{r\zeta} \left(-\gamma \sin\theta u + \frac{\partial v}{\partial \eta} + \gamma \cos\theta w \right) \\
\epsilon_{\theta\eta} &= \frac{1}{2r\zeta} \left(\frac{\partial u}{\partial \eta} + \gamma \sin\theta v \right) + \frac{1}{2r} \frac{\partial v}{\partial \theta} \\
\kappa_{\theta} &= \frac{1}{r^2} \left(\frac{\partial u}{\partial \theta} - \frac{\partial^2 w}{\partial \theta^2} \right) \\
\kappa_{\eta} &= -\frac{\sin\theta}{rR\zeta} \left(u - \frac{\partial w}{\partial \theta} \right) + \frac{\cos\theta}{rR\zeta^2} \frac{\partial v}{\partial \eta} - \frac{1}{(r\zeta)^2} \frac{\partial^2 w}{\partial \eta^2} \\
\kappa_{\theta\eta} &= \frac{(1+2\zeta)}{(2r\zeta)^2} \frac{\partial u}{\partial \eta} + \frac{1}{2r^2\zeta} \left(\gamma \cos\theta - \frac{1}{2} \right) \frac{\partial v}{\partial \theta} \\
&\quad + \frac{2\gamma^2 \sin 2\theta + (1-2\zeta)\gamma \sin\theta}{(2r\zeta)^2} v - \frac{1}{r^2\zeta} \frac{\partial^2 w}{\partial \theta \partial \eta} - \frac{1}{rR\zeta^2} \frac{\partial w}{\partial \eta}
\end{aligned} \tag{3.13}$$

By combining eqns. (3.13), (3.9), and (3.8) one obtains a set of three equations which govern the system. These equations which are in terms of the three displacement components are

$$\begin{aligned}
&[\nu_1 \frac{\partial^2}{\partial \theta^2} + \nu_2 \frac{\partial^2}{\partial \eta^2} + \nu_3 \frac{\partial}{\partial \theta} + \nu_4]u + [\nu_5 \frac{\partial^2}{\partial \theta \partial \eta} + \nu_6 \frac{\partial}{\partial \eta}]v \\
&+ [\nu_7 \frac{\partial^3}{\partial \theta^3} + \nu_8 \frac{\partial^3}{\partial \theta \partial \eta^2} + \nu_9 \frac{\partial^2}{\partial \theta^2} + \nu_{10} \frac{\partial^2}{\partial \eta^2} + \nu_{11} \frac{\partial}{\partial \theta} + \nu_{12}]w = 0 \\
&[\nu_{13} \frac{\partial^2}{\partial \theta \partial \eta} + \nu_{14} \frac{\partial}{\partial \eta}]u + [\nu_{15} \frac{\partial^2}{\partial \theta^2} + \nu_{16} \frac{\partial^2}{\partial \eta^2} + \nu_{17} \frac{\partial}{\partial \theta} + \nu_{18}]v \\
&+ [\nu_{19} \frac{\partial^3}{\partial \eta^3} + \nu_{20} \frac{\partial^3}{\partial \theta^2 \partial \eta} + \nu_{21} \frac{\partial^2}{\partial \theta \partial \eta} + \nu_{22} \frac{\partial}{\partial \eta}]w = 0
\end{aligned} \tag{3.14}$$

$$\begin{aligned}
& [\nu_{23} \frac{\partial^3}{\partial \theta^3} + \nu_{24} \frac{\partial^3}{\partial \theta \partial \eta^2} + \nu_{25} \frac{\partial^2}{\partial \eta^2} + \nu_{26} \frac{\partial^2}{\partial \theta^2} + \nu_{27} \frac{\partial}{\partial \theta} + \nu_{28}] u \\
& + [\nu_{29} \frac{\partial^3}{\partial \eta^3} + \nu_{30} \frac{\partial^3}{\partial \theta^2 \partial \eta} + \nu_{31} \frac{\partial^2}{\partial \theta \partial \eta} + \nu_{32} \frac{\partial}{\partial \eta}] v \\
& + [\nu_{33} \frac{\partial^4}{\partial \theta^4} + \nu_{34} \frac{\partial^4}{\partial \eta^4} + \nu_{35} \frac{\partial^4}{\partial \theta^2 \partial \eta^2} - \nu_{36} \frac{\partial^3}{\partial \theta^3} + \nu_{37} \frac{\partial^3}{\partial \theta \partial \eta^2} \\
& + \nu_{38} \frac{\partial^2}{\partial \eta^2} + \nu_{39} \frac{\partial^2}{\partial \theta^2} + \nu_{40} \frac{\partial}{\partial \theta} + \nu_{41}] w = -(r^4/D) \zeta^4 q_r
\end{aligned}$$

where the $\nu_i, i = 1, 41$ are given in the Appendix A.

Fourier series expansions are written for the displacement components and the normal load as follows

$$\begin{aligned}
u &= \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} u_{mn} \sin n \theta \sin \mu_m \eta \\
v &= \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} v_{mn} \cos n \theta \cos \mu_m \eta \\
w &= \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} w_{mn} \cos n \theta \sin \mu_m \eta \\
q_r &= \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} q_{mn} \cos n \theta \sin \mu_m \eta
\end{aligned} \tag{3.15}$$

where

$$\mu_m = m \pi \gamma / \psi \tag{3.16}$$

and ψ equals the shell angular length (Fig. 3.1). Quantities N and M (unsubscripted) are the series truncation integers. These are selected large enough to ensure convergence of the solution.

The selection of the series expansions in eqn. (3.15) implies that simply supported conditions exist at the ends of the shell. Thus the following boundary conditions are satisfied at the shell ends $\phi = 0^\circ$ and $\phi = \psi$

$$N_\eta = u = w = M_\eta = 0 \tag{3.17}$$

Furthermore selection of the $\cos n\theta$ term instead of the $\sin n\theta$ term in the series for q_r restricts the loading to cases of in-plane bending for the toroidal shell.

The Fourier series expansions of eqns. (3.15) are next substituted into the governing equations (3.14), and the coefficients of like products of trigonometric terms are collected. The resulting algebraic expressions for the coefficients are cumbersome. They were developed with the aid of the algebraic computation software MAPLE [55]. The resulting expressions have the form

$$\begin{aligned} & \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} [(a_{19m,n-4}u_{m,n-4} + a_{29m,n-4}v_{m,n-4} + a_{39m,n-4}w_{m,n-4} \\ & + a_{18m,n-3}u_{m,n-3} + a_{28m,n-3}v_{m,n-3} + a_{38m,n-3}w_{m,n-3} \\ & + a_{17m,n-2}u_{m,n-2} + a_{27m,n-2}v_{m,n-2} + a_{37m,n-2}w_{m,n-2} \\ & + a_{16m,n-1}u_{m,n-1} + a_{26m,n-1}v_{m,n-1} + a_{36m,n-1}w_{m,n-1} \\ & + a_{15mn}u_{mn} + a_{25mn}v_{mn} + a_{35mn}w_{mn} \end{aligned} \quad (3.18)$$

$$\begin{aligned} & + a_{14m,n+1}u_{m,n+1} + a_{24m,n+1}v_{m,n+1} + a_{34m,n+1}w_{m,n+1} \\ & + a_{13m,n+2}u_{m,n+2} + a_{23m,n+2}v_{m,n+2} + a_{33m,n+2}w_{m,n+2} \\ & + a_{12m,n+3}u_{m,n+3} + a_{22m,n+3}v_{m,n+3} + a_{32m,n+3}w_{m,n+3} \\ & + a_{11m,n+4}u_{m,n+4} + a_{21m,n+4}v_{m,n+4} + a_{31m,n+4}w_{m,n+4}) \sin n\theta] \sin \mu_m \eta = 0 \end{aligned}$$

$$\begin{aligned} & \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} [(b_{19m,n-4}u_{m,n-4} + b_{29m,n-4}v_{m,n-4} + b_{39m,n-4}w_{m,n-4} \\ & + b_{18m,n-3}u_{m,n-3} + b_{28m,n-3}v_{m,n-3} + b_{38m,n-3}w_{m,n-3} \\ & + b_{17m,n-2}u_{m,n-2} + b_{27m,n-2}v_{m,n-2} + b_{37m,n-2}w_{m,n-2} \\ & + b_{16m,n-1}u_{m,n-1} + b_{26m,n-1}v_{m,n-1} + b_{36m,n-1}w_{m,n-1} \\ & + b_{15mn}u_{mn} + b_{25mn}v_{mn} + b_{35mn}w_{mn} \end{aligned} \quad (3.19)$$

$$\begin{aligned} & + b_{14m,n+1}u_{m,n+1} + b_{24m,n+1}v_{m,n+1} + b_{34m,n+1}w_{m,n+1} \\ & + b_{13m,n+2}u_{m,n+2} + b_{23m,n+2}v_{m,n+2} + b_{33m,n+2}w_{m,n+2} \\ & + b_{12m,n+3}u_{m,n+3} + b_{22m,n+3}v_{m,n+3} + b_{32m,n+3}w_{m,n+3} \end{aligned}$$

$$\begin{aligned}
& +b_{11m,n+4}u_{m,n+4} + b_{21m,n+4}v_{m,n+4} + b_{31m,n+4}w_{m,n+4})\cos n\theta]\cos\mu_m\eta = 0 \\
& \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} [(c_{19m,n-4}u_{m,n-4} + c_{29m,n-4}v_{m,n-4} + c_{39m,n-4}w_{m,n-4} \\
& + c_{18m,n-3}u_{m,n-3} + c_{28m,n-3}v_{m,n-3} + c_{38m,n-3}w_{m,n-3} \\
& + c_{17m,n-2}u_{m,n-2} + c_{27m,n-2}v_{m,n-2} + c_{37m,n-2}w_{m,n-2} \\
& + c_{16m,n-1}u_{m,n-1} + c_{26m,n-1}v_{m,n-1} + c_{36m,n-1}w_{m,n-1} \\
& + c_{15mn}u_{mn} + c_{25mn}v_{mn} + c_{35mn}w_{mn} \\
& + c_{14m,n+1}u_{m,n+1} + c_{24m,n+1}v_{m,n+1} + c_{34m,n+1}w_{m,n+1} \\
& + c_{13m,n+2}u_{m,n+2} + c_{23m,n+2}v_{m,n+2} + c_{33m,n+2}w_{m,n+2} \\
& + c_{12m,n+3}u_{m,n+3} + c_{22m,n+3}v_{m,n+3} + c_{32m,n+3}w_{m,n+3} \\
& + c_{11m,n+4}u_{m,n+4} + c_{21m,n+4}v_{m,n+4} + c_{31m,n+4}w_{m,n+4})\cos n\theta]\sin\mu_m\eta = \\
& \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} [d_9q_{m,n-4} + d_8q_{m,n-3} + d_7q_{m,n-2} + d_6q_{m,n-1} + d_5q_{mn} \\
& + d_4q_{m,n+1} + d_3q_{m,n+2} + d_2q_{m,n+3} + d_1q_{m,n+4}]\cos n\theta\sin\mu_m\eta
\end{aligned} \tag{3.20}$$

where the coefficients a_{ijmn} , b_{ijmn} , c_{ijmn} and d_i are lengthy, but known, functions of n , μ_m , ν , and K . These generic expressions involve negative-indexed terms for $n = 0, 1, 2, 3$ which are not present in the original expansions. It is necessary to modify the equations for these values of n , setting the appropriate terms to zero and employing the identities

$$\cos(-n\theta) = \cos n\theta; \sin(-n\theta) = -\sin n\theta \tag{3.21}$$

for the cases of negative arguments of the \cos and \sin functions. The quantities q_{mn} are related to the loading, and are prescribed in eqns. (3.23-3.24) below.

By equating coefficients of like trigonometric terms on the two sides of the equations, one obtains $M' = (M-1)/2 + 1$ sets of simultaneous equations, each consisting of $3N + 2$ equations. By neglecting coefficients of order higher than N , there are in each set, also $3N + 2$ unknowns. Each of the M' sets of equations is banded, consisting of one group

of two equations and N groups of three equations. The band-width is uniform at 27 terms. A customized band solution algorithm was developed to solve such an equation set efficiently.

The solution is developed for a normal pressure q_r on the shell surface which is everywhere zero, except in the following four regions where the pressure is uniform at a value of $-p$ (i.e. radially inwards):

$$\begin{aligned}
 (-\beta - \lambda) \leq \theta \leq (-\beta + \lambda); (\phi' - \alpha - \delta) \leq \phi \leq (\phi' - \alpha + \delta) \\
 (\beta - \lambda) \leq \theta \leq (\beta + \lambda); (\phi' - \alpha - \delta) \leq \phi \leq (\phi' - \alpha + \delta) \\
 (-\beta - \lambda) \leq \theta \leq (-\beta + \lambda); (\phi' + \alpha - \delta) \leq \phi \leq (\phi' + \alpha + \delta) \\
 (\beta - \lambda) \leq \theta \leq (\beta + \lambda); (\phi' + \alpha - \delta) \leq \phi \leq (\phi' + \alpha + \delta)
 \end{aligned} \tag{3.22}$$

The four regions (3.22) are symmetric with respect to the planes $z = 0$ and $\phi = \phi'$ (Fig. 3.1). Quantity β is the circumferential angle to the center of the load pad, while 2λ is the angular length of the pad in the circumferential direction. Quantity α is the longitudinal angle measured from the transverse plane of symmetry to the center of the load pad, while 2δ is the angular length of the pad in the longitudinal direction. In considering the loading to be applied in four regions it is possible to represent a single load, two loads, or four loads with a single theoretical expression, by simply specifying the regions to be adjacent or separated with respect to each other. In the present study only the first two of these capabilities were used.

The Fourier series for such a bisymmetric set of four uniform pad loads is given by eqn.

$$q_r = \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} q_{mn} \cos n\theta \sin \mu_m \eta \tag{3.23}$$

where the q_{mn} are constants to be determined for a specified loading.

$$q_{m0} = -(16p/\pi^2)(\lambda/m)k_m \sin \mu_m \eta$$

$$q_{mn} = -(16\rho/\pi^2)(2/mn)k_m \cos n\beta \sin n\lambda \cos n\theta \sin \mu_m \eta, (n > 0) \quad (3.24)$$

$$k_m = \sin(m\pi/2) \cos m\pi\alpha/\psi \sin m\pi\delta/\psi$$

The following series expansions are written for the strain and curvature functions

$$\begin{aligned} \epsilon_\theta &= \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} \epsilon_{\theta mn} \cos n\theta \sin \mu_m \eta \\ \epsilon_\eta &= \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} \epsilon_{\eta mn} \cos n\theta \sin \mu_m \eta \\ \epsilon_{\theta\eta} &= \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} \epsilon_{\theta\eta mn} \sin n\theta \cos \mu_m \eta \\ \kappa_\theta &= \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} \kappa_{\theta mn} \cos n\theta \sin \mu_m \eta \\ \kappa_\eta &= \sum_{n=0}^{\infty} \sum_{m=1}^{\infty} \kappa_{\eta mn} \cos n\theta \sin \mu_m \eta \\ \kappa_{\theta\eta} &= \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} \kappa_{\theta\eta mn} \sin n\theta \cos \mu_m \eta \end{aligned} \quad (3.25)$$

Expressions are required for the strain and curvature coefficients of the right hand members in terms of the displacement coefficients. By substituting the series (3.15) into the eqns. (3.13) and equating coefficients of like terms one obtains

$$\begin{aligned} \epsilon_{\theta mn} &= (nu_{mn} + w_{mn})/r \\ \epsilon_{\eta mn} &= -(u_{m,n+1} - u_{m,n-1})/(2\rho r\zeta) - \mu_m v_{mn}/(r\zeta) \\ &\quad + (w_{m,n+1} + w_{m,n-1})/(2\rho r\zeta) \\ \epsilon_{\theta\eta mn} &= \mu_m u_{mn}/(2r\zeta) + (v_{m,n-1} - v_{m,n+1})/(4\rho r\zeta) - nv_{mn}/(2r) \end{aligned} \quad (3.26)$$

and

$$\begin{aligned} \kappa_{\theta mn} &= n(u_{mn} + nw_{mn})/r^2 \\ \kappa_{\eta mn} &= -(u_{m,n+1} - u_{m,n-1})/(2\rho r^2\zeta) \\ &\quad - \mu_m (v_{m,n+1} + v_{m,n-1})/(2\rho r^2\zeta^2) \\ &\quad + \mu_m^2 w_{mn}/(r\zeta^2) - n(w_{m,n+1} - w_{m,n-1})/(2\rho r^2\zeta) \end{aligned} \quad (3.27)$$

$$\begin{aligned}
\kappa_{\theta\eta mn} = & \mu_m u_{mn}(1 + 2\zeta)/(2r\zeta)^2 - n(v_{m,n-1} + v_{m,n+1})/(4rR\zeta) \\
& + nv_{mn}/(4r^2\zeta) + (v_{m,n-2} - v_{m,n+2})/(2R\zeta)^2 \\
& + (1 - 2\zeta)(v_{m,n-1} - v_{m,n+1})/(8rR\zeta^2) \\
& + n\mu_m w_{mn}/(2r^2\zeta) - \mu_m(w_{m,n-1} - w_{m,n+1})/(2rR\zeta^2)
\end{aligned}$$

Terms with negative indices are understood to be zero.

To summarize, the displacement coefficients for a loading specified by the eqns. (3.23)-(3.24), are found by solving the eqn. set (3.18)-(3.20), while the total displacement components are found from eqns. (3.15a-c). The strain-curvature coefficients are from the relations (3.26)-(3.27), while the total strain and curvature components are found from eqn. (3.25). The stress resultants are found from eqn. (3.9), while the normal stress components on the shell surface are found from eqn. (3.12).

The theory presented in this section was coded in a FORTRAN computer program labelled TORSAN, and it is given in Appendix B. Results from this section are presented in Chapters 4 and 5.

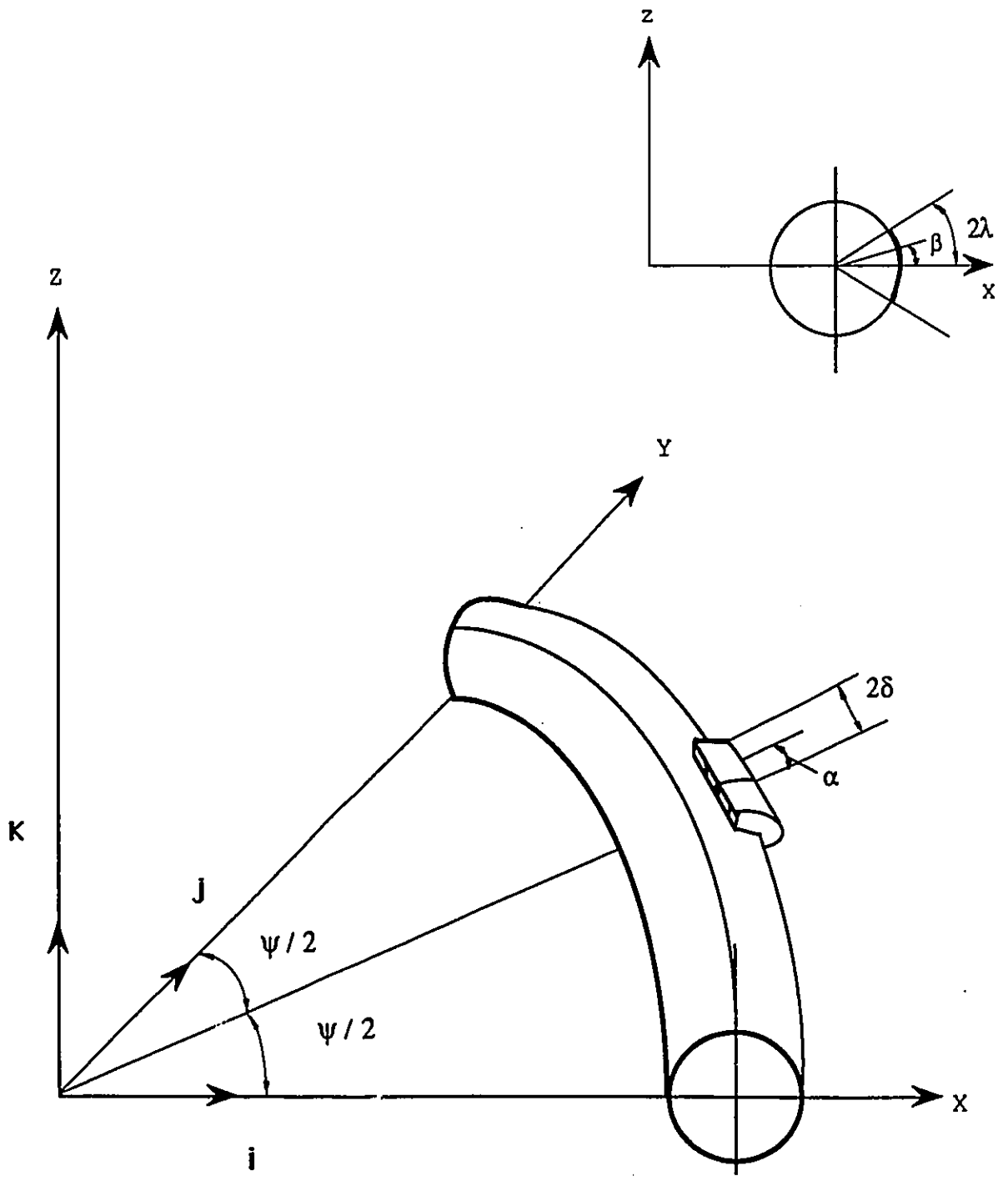


Figure 3.1: Surface loading of a toroidal shell.

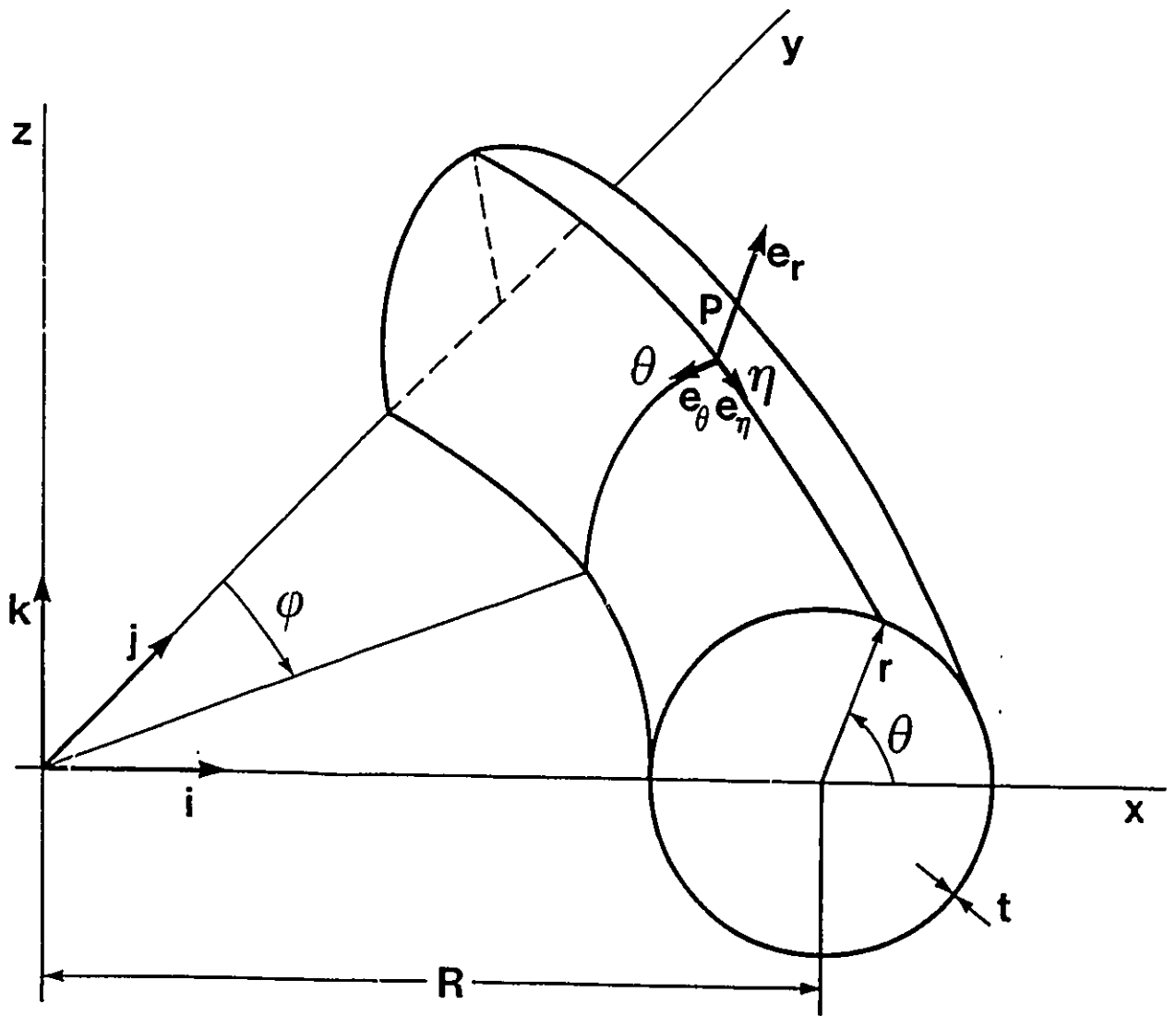


Figure 3.2: Toroidal coordinate system.

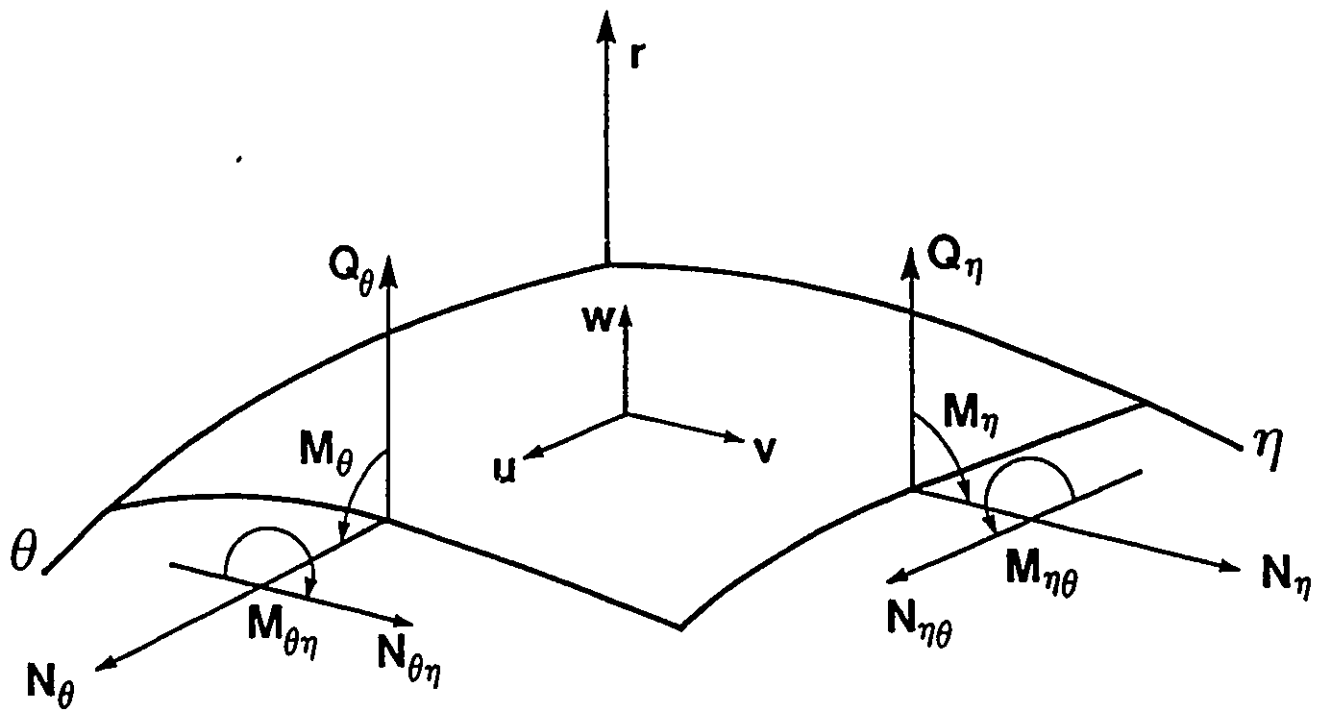


Figure 3.3: Displacement components and stress resultants.

Chapter 4

Comparison of The Sanders Shell Theory Results with Other Results

4.1 Introduction

In this chapter, the solution of the Sanders shell theory for three problems is compared with solutions from either the FEM method or other shell theories. The first two problems cover cases of local loads on sectorial toroidal shells. The first problem is for a small R/r ratio while the second for a large R/r ratio. The final problem concerns the case of a half-filled cylindrical vessel. Efforts are made to assess the accuracy of the Sanders shell theory by comparison with the other results.

4.2 Local Load on a 90°-Elbow

Results obtained using the Sanders theory are compared in this section with results obtained from the FEM (ADINA package). The problem considered has relevance to the problem of an elbow connected to a trunnion pipe [53]. The loading corresponds to a single pad of inward uniform normal pressure centered at the extrados ($\theta = 0^\circ$), on the $\phi = 45^\circ$ plane (Fig. 3.1). The pad extends for 60° in the circumferential direction and 30° in the longitudinal direction. The numerical values for the geometric, material and loading parameters are as follows (Fig. 3.2)

$$\begin{aligned}r &= 84.15 \text{ mm}; E = 207,000 \text{ MPa} \\R &= 252.45 \text{ mm}; \nu = 0.3 \\t &= 7.1 \text{ mm}; p = 1 \text{ MPa} \\ \beta &= 15^\circ; \lambda = 15^\circ \\ \alpha &= 7.5^\circ; \delta = 7.5^\circ \\ \psi &= 90^\circ; \phi_l = 45^\circ\end{aligned}\tag{4.1}$$

Two sets of results from the Sanders theory are given. The first set, represented by SAN in the figures, is considered to be very close to the converged solution, and corresponds to truncation integers of $M = 51$, $N = 25$. The second set, represented by SANU in the figures, is an unconverged approximation, corresponding to truncation integers of $M = 11$, $N = 7$. The finite element results, represented by FEM in the figures, correspond to the 80-element mesh and a total of 775 nodal points (Fig. 4.1). There are three displacement and two rotation degrees of freedom at each node. Fig. 4.1 shows a projected view of FEM mesh. The projected view is easy to draw and clearly indicates the mesh.

A comparison of results for the radial displacement component w at the extrados is

given in Fig. 4.2. The deflection pattern indicates a localized bulging under the load, superimposed on a beam bending mode. The three sets of results (SAN, FEM, and SANU) show very good agreement over the entire length of the elbow. At $\phi = 45^\circ$, where the deflection is the largest, the SAN values differ by only 0.23% from the FEM values.

A comparison of the results for the radial displacement component w on the transverse plane $\phi = 45^\circ$ is given in Fig. 4.3. There is a relatively large inward deflection of the shell under the load pad near the extrados and a smaller outward deflection elsewhere. It is observed that the pad load is located in a positive curvature portion of the toroidal shell and the deflection pattern in the immediate vicinity approximates that in a spherical shell. There is very good agreement among the three sets of results.

Fig. 4.4 shows a comparison of the results for the longitudinal stress σ_n at the extrados. The stress is shown for the inside surface of the shell. A variation of stress is observed which corresponds to a superposition of beam bending and local bending at the pad edges. There is a sharp rise and drop in stress near the pad edges, as was observed in the study by Redekop for a slightly curved toroidal shell [4]. There is good agreement in the SAN and FEM set of values over the entire length of the elbow. At $\phi = 45^\circ$, where the stress is largest, the agreement is to two figures. The SANU values do not closely represent the variation of stress near the pad edges.

Fig. 4.5 shows a comparison of results for the longitudinal stress σ_n on the transverse plane $\phi = 45^\circ$. There is good agreement in the three sets of results. The absolute maximum stress occurs on the outside surface at the extrados. The SAN value here is 27.3 MPa while the FEM value is 26.9 MPa. The maximum stress on the inside surface also occurs at the extrados.

In Fig. 4.6 a comparison of results is given for the circumferential stress σ_θ for the

transverse plane $\phi = 45^\circ$. The stress shows oscillatory variation, with the maximum values located at $\theta = 0^\circ$, and nodes located at approximately $\theta = 30^\circ$ and $\theta = 90^\circ$. The membrane stresses are compressive from the extrados to approximately $\theta = 90^\circ$ and tensile thereafter. Good agreement is observed in the three sets of values. The overall maximum absolute stress occurs on the outside surface at the extrados. The SAN value is 21.8 MPa while the FEM value is 21.3 MPa.

Table 4.1 shows a comparison of predicted changes in the horizontal and vertical cross-sectional diameters at various positions along the length of the elbow. The three sets of values show very close agreement. At the central transverse plane, where the largest changes in diameter take place, the SAN values are 1.3% smaller for the horizontal direction, and 0.1% smaller for the vertical direction, compared to the FEM values.

The commercial program ADINA was used to obtain the FEM results. Taking into account the symmetry about the horizontal xy plane of the toroidal shell, only a half of it needs to be considered. This portion is modelled by a mesh of 80 unequal-sized 16-noded shell elements and a total of 775 nodal points. In order to obtain a useful approximate solution for stress concentrations, a finer subdivision is necessary in regions where stress concentrations are expected. Therefore, the elements span respectively, 15° , 22.5° and 30° in the circumferential direction and 7.5° and 11.25° in the longitudinal direction. A representation of the mesh of elements is given in Fig. 4.1. Element matrices were evaluated using Gaussian integration based on a $4 \times 4 \times 2$ (r-s-t) grid, and a total of 3605 equilibrium equations were solved in the numerical solution.

The ADINA information for CPU time was not available but believed to be of the same order as a FEM computer program SHELL FORTRAN. CPU times in sec. are 3.2, 40.8 and 225.8, respectively for SANU, SAN and FEM on an AMDAHL 5860/CMS mainfram computer.

4.3 Ram Bending of a Sectorial Toroidal shell

In this section the problem of the ram bending of a sectorial toroidal shell in the elastic range is considered. The loading is idealized as a set of pads of uniform normal pressure. Numerical results for displacements and stresses are obtained using the MVD [2] and Sanders [1] shell theories, and these are compared with results from the FEM.

The curved pipe is assumed bent by three dies which impart a uniform normal pressure on the shell outside surface at three positions (Fig. 4.7). The loading is symmetrical about the horizontal plane $z = 0$, and about a vertical transverse plane which divides the shell into two equal parts of angular length $\psi/2$. This satisfies the symmetrical conditions inherent in equation (3.15).

Numerical results are presented in this section for the following values of the geometric, material, and load parameters (Fig. 3.2)

$$\begin{aligned}r &= 57.5 \text{ mm}; E = 207,000 \text{ MPa} \\R &= 5750 \text{ mm}; \nu = 0.3 \\t &= 3.6 \text{ mm}; p = 6 \text{ MPa} \\ \beta' &= 135^\circ; \beta'' = 45^\circ \\ \lambda' &= 45^\circ; \lambda'' = 45^\circ \\ \alpha' &= 0.28648^\circ; \alpha'' = 2.5783^\circ \\ \delta' &= 0.28648^\circ; \delta'' = 0.28648^\circ \\ \psi &= 5.7296^\circ; \phi' = 2.8648^\circ\end{aligned}\tag{4.2}$$

This loading corresponds to the case where the ram and reactive dies extend respectively through one-fifth and one-tenth of the shell length, and wrap half-way around the circumference.

Three sets of results are given in Fig. 4.9-4.13 and Table 4.2 below. The first two sets, labelled MVD and SAN, were obtained respectively using the MVD and Sanders shell theories. These results are from solutions for which the truncation integers of equations (3.15-3.16) were $M = 101$ and $N = 51$. The displacements in these solutions agreed to at least two figures with results from solutions for which the truncation integers were $M = 51$ and $N = 25$. They thus can be considered to be the converged values. The third set of results was obtained using the program ADINA, with a mesh as depicted in Fig. 4.8. To make some comparisons at specific positions results were also computed using elementary straight beam theory. A simply supported beam was assumed with a transverse point load applied at the mid-span. The assumed beam length was 517.5 mm representing the distance between the centers of the two outer dies.

Fig. 4.9 shows a comparison of results for the radial displacement w on the intrados ($\theta = 180^\circ$), on the right side of the transverse plane of symmetry. The deflection pattern corresponds to a beam bending mode onto which is superimposed a localized bulging due to the pad load. The SAN and FEM values agree to within 1%, while the MVD values are consistently lower. At the transverse plane of symmetry where the displacement is the largest, the MVD value is 4.9% below the FEM value. For the comparable cylindrical shell (ref. [19], Fig. 4.8) the maximum deflection on the intrados was 0.61 mm. The current maximum value of 0.690 mm exceeds this value by 11.5%. From the beam theory a prediction of 0.515 mm was obtained for the mid-span deflection. The comparable MVD value, found by taking the mean of the absolute deflections on the intrados and extrados ($\theta = 0^\circ$) is 0.590 mm, while the FEM result is 0.593 mm.

A comparison of results for the radial displacement w on the transverse plane of symmetry $\phi = \psi/2$ is shown in Fig. 4.10. The SAN and FEM results again show close agreement (maximum difference less than 1%). The MVD results are 5.4% greater than

the FEM results on the extrados and 4.9% smaller on the intrados.

Fig. 4.11 shows a comparison of the results for the longitudinal stress σ_r at the intrados on the right side of the transverse plane of symmetry. The SAN and FEM results for this stress show close agreement, while the MVD values are consistently smaller. The absolute maximum stress predicted by the MVD theory is 241.1 MPa, by the Sanders theory 253.1 MPa, while by the FEM 265.5 MPa. The straight beam theory predicts a maximum bending stress of 274.4 MPa. The comparable maximum stress for a cylindrical shell (ref. [19], Fig. 3.3) using the MVD theory is 214.9 MPa.

A comparison of results for the longitudinal stress σ_r on the transverse plane of symmetry is shown in Fig. 4.12. There is close agreement in the SAN and FEM results. The absolute maximum stress predicted by MVD is 241.1 MPa, that by the Sanders theory 253.1 MPa, while that by the FEM 265.5 MPa. This maximum occurs on the inside surface at the intrados. The maximum stress on the outside surface occurs approximately at $\theta = 125^\circ$. Finally the comparable maximum stress for a cylindrical shell (ref. [19], Fig. 4.11) using the MVD theory is 239.3 MPa.

Fig. 4.13 shows a comparison of results for the circumferential stress σ_θ on the transverse plane of symmetry. The agreement in the SAN and FEM results is not as close as for the longitudinal stress. The overall maximum absolute stress predicted by MVD is 232.5 MPa, that by the Sanders theory 253.0 Mpa, while that by FEM 283.5 MPa. This maximum occurs on the outside surface at approximately $\theta = 112^\circ$. Finally the comparable maximum stress for a cylindrical shell (ref. [19], Fig. 4.10) using the MVD theory is 295.0 MPa.

A comparison of the predicted changes in the horizontal and vertical diameters of the shell is shown in Table 4.2. The MVD, SAN, and FEM results are given using the longitudinal coordinate ϕ . The agreement between the SAN and FEM results is good.

Quantitatively, the maximum value predicted by the MVD theory is 30.4% below that predicted by the FEM for the horizontal diameter, and 34.0% below for the vertical diameter. The maximum value predicted by the Sanders theory is 6.8% above the value predicted by the FEM for the horizontal diameter, and 10.8% above for the vertical diameter.

Finally the prediction, for the current loading, of the rotation of the end section with respect to the transverse plane of symmetry is 0.167° using the MVD theory, 0.177° for the Sanders theory, and 0.175° using the FEM. The beam theory predicts a rotation of 0.171° .

The ADINA program was used to determine the FEM solution. Taking into account the symmetry about the horizontal xy plane of the toroidal shell, only a half of it needs to be considered. This portion is again modelled by a mesh of 80 unequal-sized 16-noded shell elements and a total of 775 nodal points. In order to obtain a useful approximate solution for stress concentrations, a finer subdivision is necessary in regions where stress concentrations are expected. Therefore, the elements span respectively, 22.5° in the circumferential direction and 0.17° , 0.28° and 0.4° in the longitudinal direction. A representation of the mesh of elements is given in Fig. 4.8. Element matrices were evaluated using Gaussian integration based on a $4 \times 4 \times 2(r-s-t)$ grid, and a total of 3605 equilibrium equations were again solved in the numerical solution.

In the previous work on a cylindrical shell with this element [19] a converged solution was obtained with a mesh containing the same number of nodes and distributed in the same pattern.

4.4 Half-filled Cylindrical Vessel

Consider an end supported circular cylindrical vessel half-filled with fluid. The height of the fluid is indicated by the angle $\bar{\alpha} = 90^\circ$ measured from the bottom of the vessel (Fig. 4.14). It is assumed that the radial pressure remains constant along the length and is directly proportional to the depth at the point of consideration.

In this section, results obtained using the Sanders theory are compared with results obtained from the Flügge theory. These problems are considered (Fig. 4.15); a straight cylindrical vessel, and two similar slightly curved toroidal vessels. There are equal radii, thickness and center-line length in three cases. The dimensions of the cylindrical vessel used here are; length $l = 12.19 \text{ m}$, mean diameter $2r = 2.44 \text{ m}$ and thickness $t = 12.7 \text{ mm}$.

Fig. 4.16 presents circumferential bending stresses ($6M_\theta/t^2$) and longitudinal membrane stresses (N_x/t) predicted around the central profile ($x=l/2$). For each stress four sets of results are presented. Three sets of results from the Sanders theory are given. The first set, represented by CYL-SAN in the figures, corresponds to a half-filled cylindrical vessel. The second and third sets, respectively represented by TOR-SAN-DEFL. 3T and TOR-SAN-DEFL. 6T in the figures, correspond to half-filled toroidal vessels whose deflections are three times and six times the thickness (Fig. 4.15). Flügge results represented by CYL-FLG, correspond to a half-filled cylindrical vessel. It is noted that the depth of fluid was not quite constant everywhere along the length of toroidal vessels. As only the stresses around the central profile are considered, the fact that the vessels were slightly curved has a negligible effect.

The results labelled SAN were obtained using the FORTRAN program TORSAN, which was discussed in Chapter 3. The Flügge theory results are from ref. [17].

The agreement of the two sets of results (CYL-FLG, CYL-SAN) in the Fig. 4.16 is considered very satisfactory for both stresses. The maximum longitudinal membrane stress occurs on the datum generator ($\bar{\phi} = 0^\circ$). The maximum circumferential bending stress is located at the fluid surface ($\bar{\phi} = 90^\circ$). Both sets of stress values for the toroidal vessel are in general smaller than the values for the corresponding cylindrical vessel. The larger the deflection, the smaller both stress values for the toroidal vessel. The circumferential bending stress values decrease faster than the longitudinal stress values. The quantitative results for the stresses indicate that a cylindrical vessel is more highly stressed under the fluid loading than a toroidal vessel.

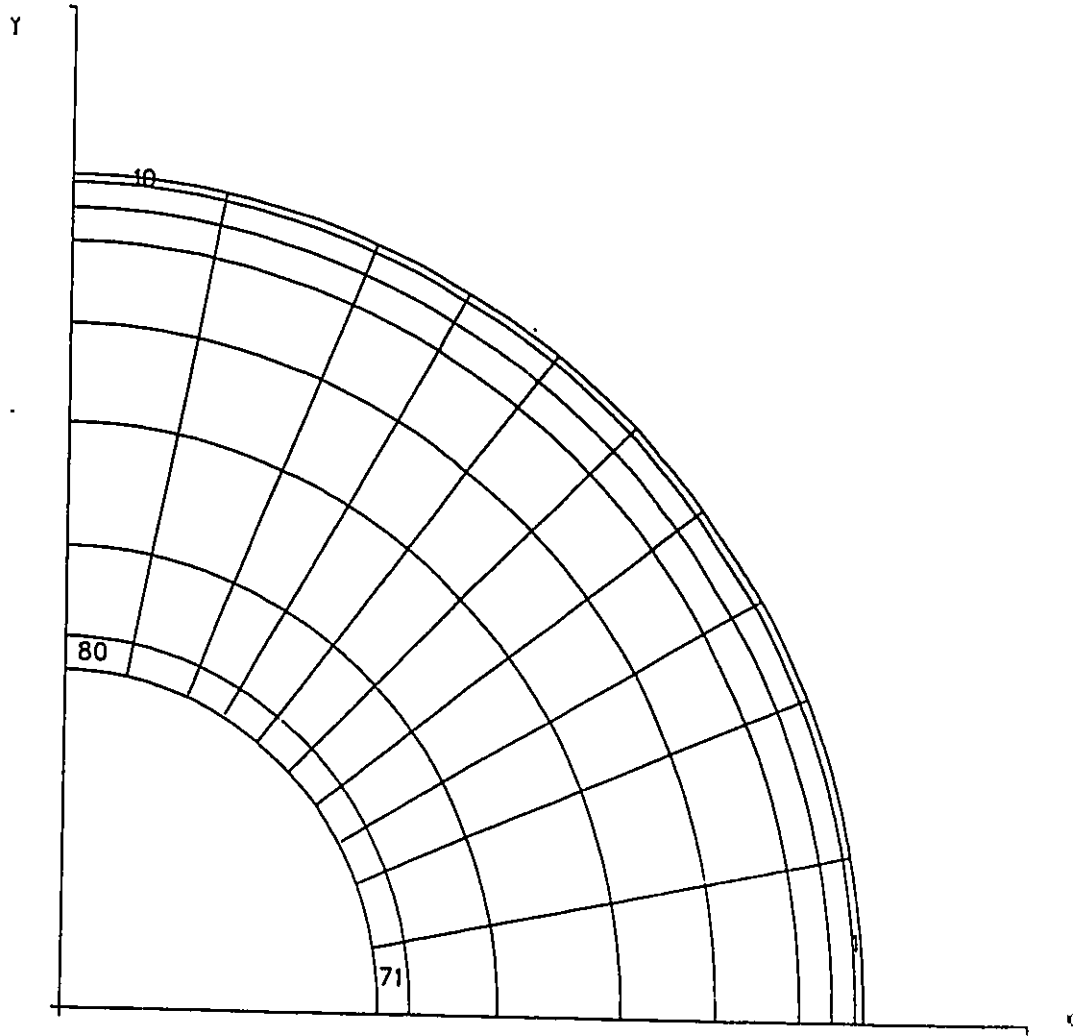


Figure 4.1: Projected view of FEM mesh for the case of the surface loading, showing some element numbers.

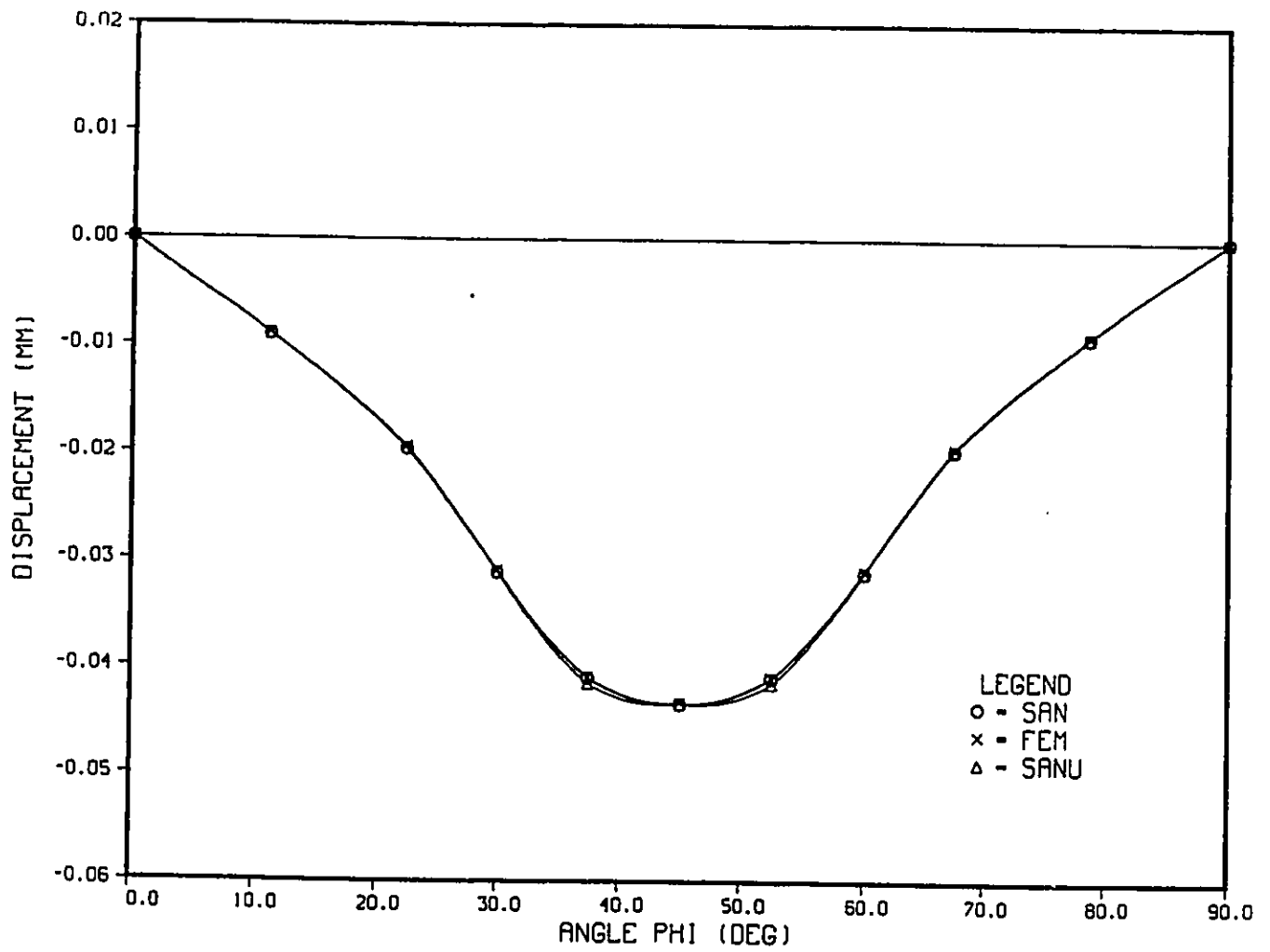


Figure 4.2: Comparison of results for radial displacement w on plane $\theta = 0^\circ$.

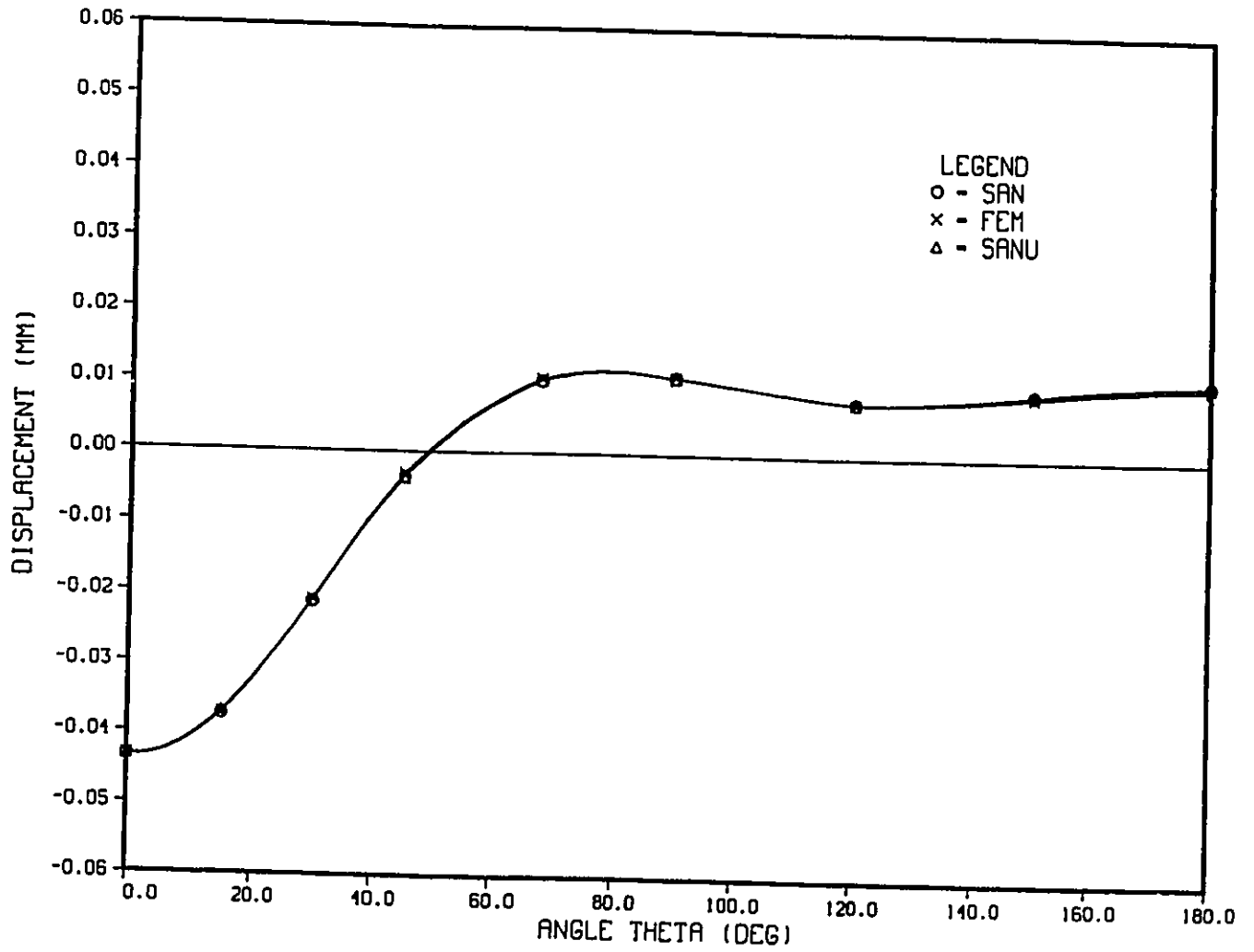


Figure 4.3: Comparison of results for radial displacement w on plane $\phi = 45^\circ$.

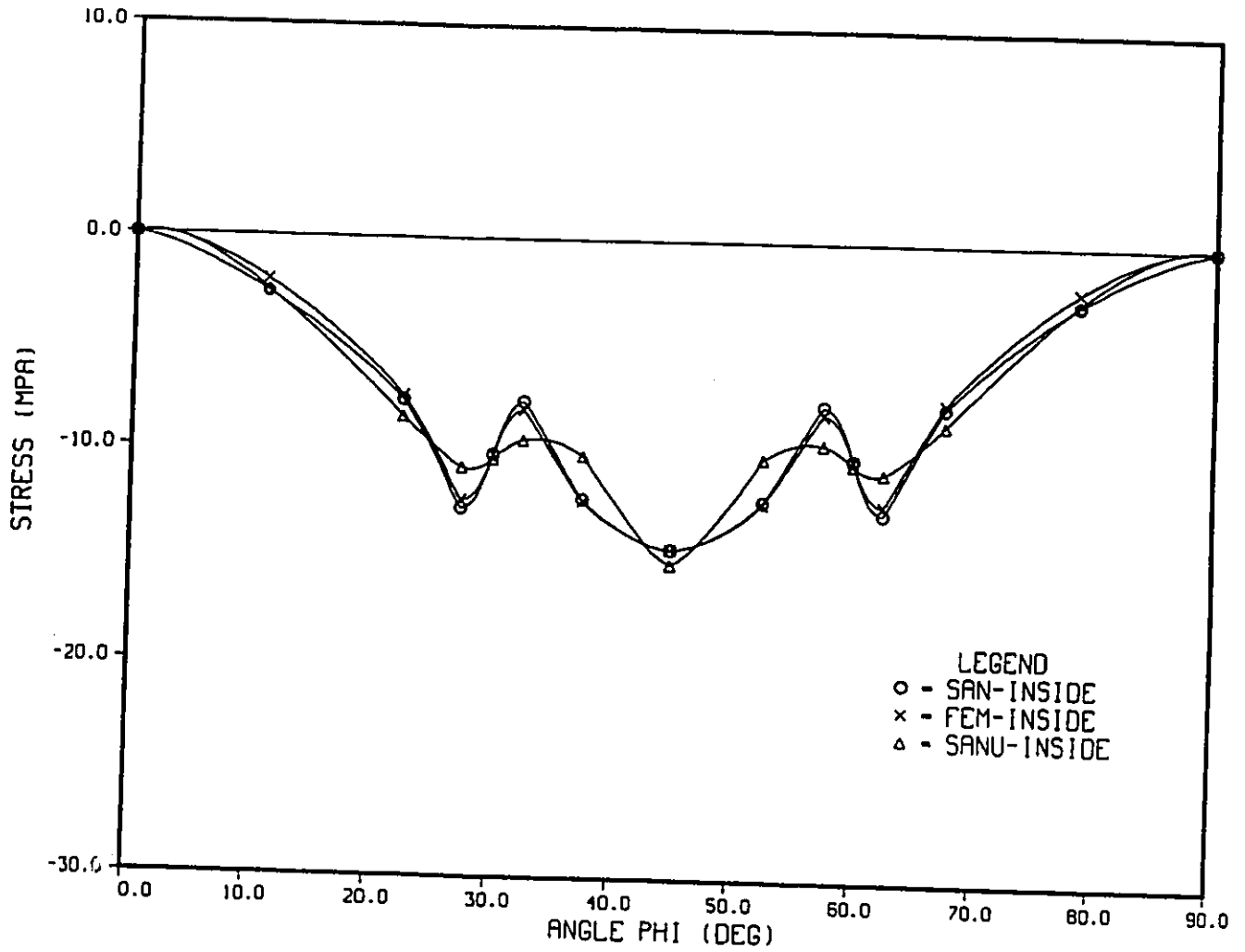


Figure 4.4: Comparison of results for longitudinal stress σ_η on plane $\theta = 0^\circ$.

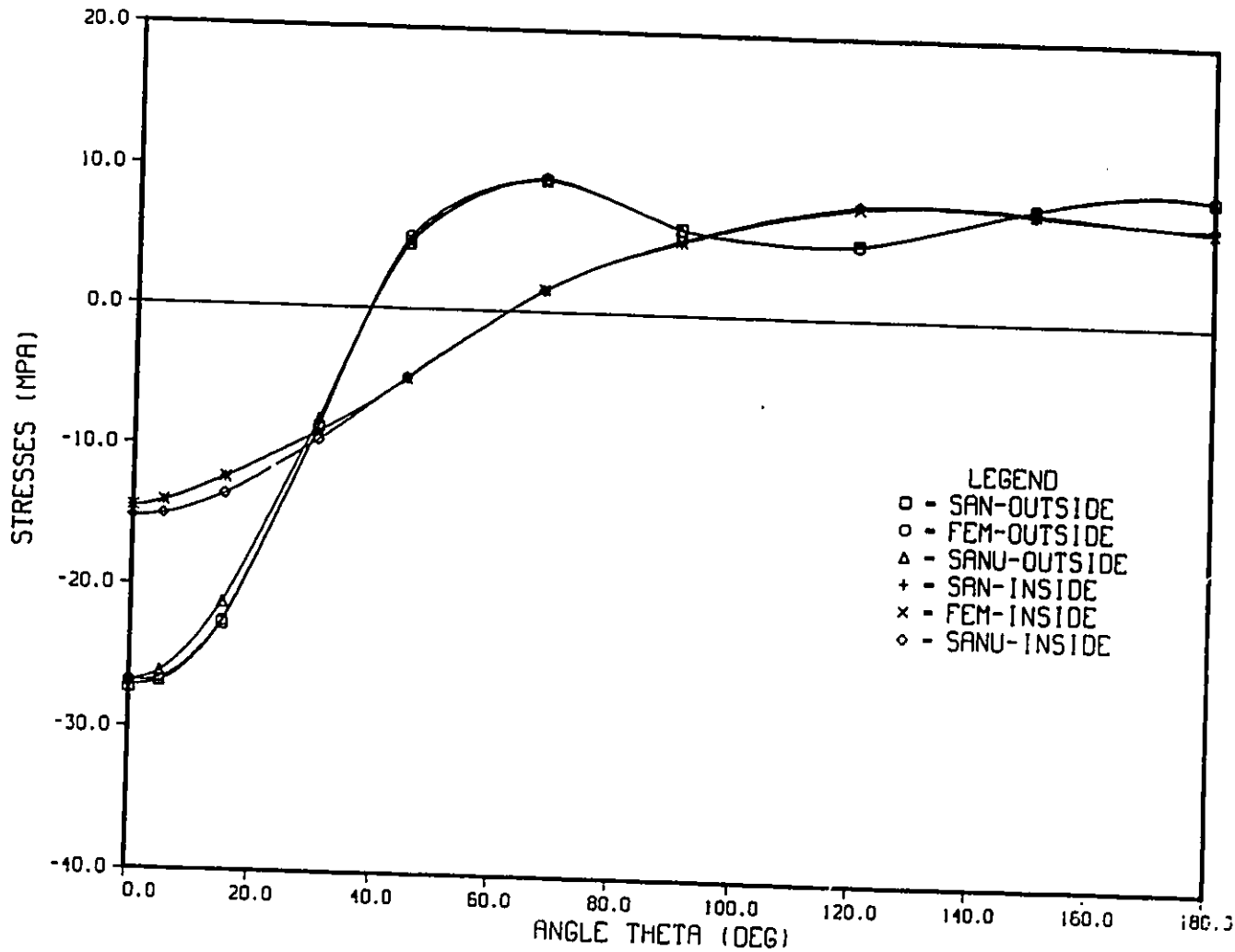


Figure 4.5: Comparison of results for longitudinal stress σ_n on plane $\phi = 45^\circ$.

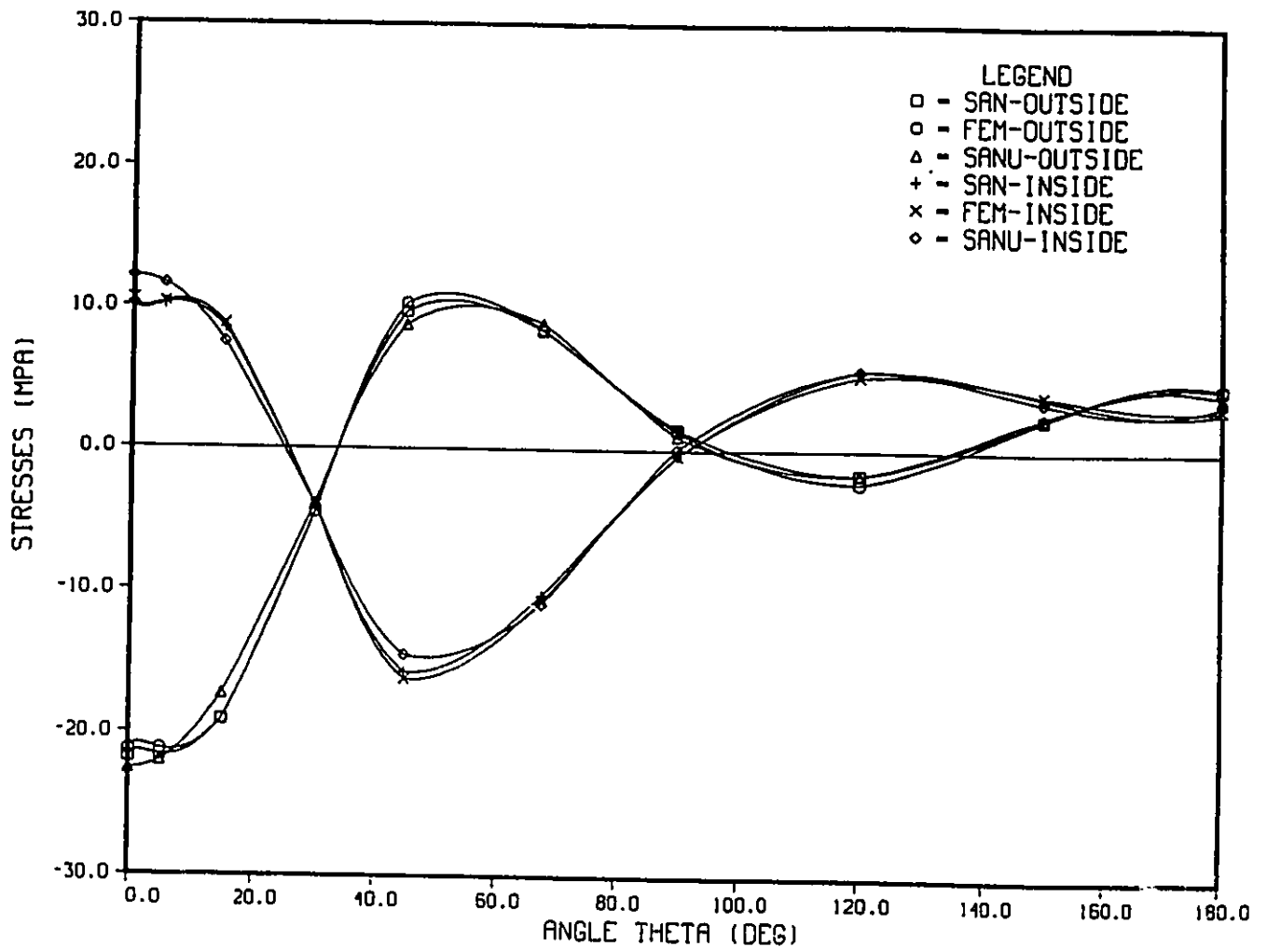


Figure 4.6: Comparison of results for circumferential stress σ_θ on plane $\phi = 45^\circ$.

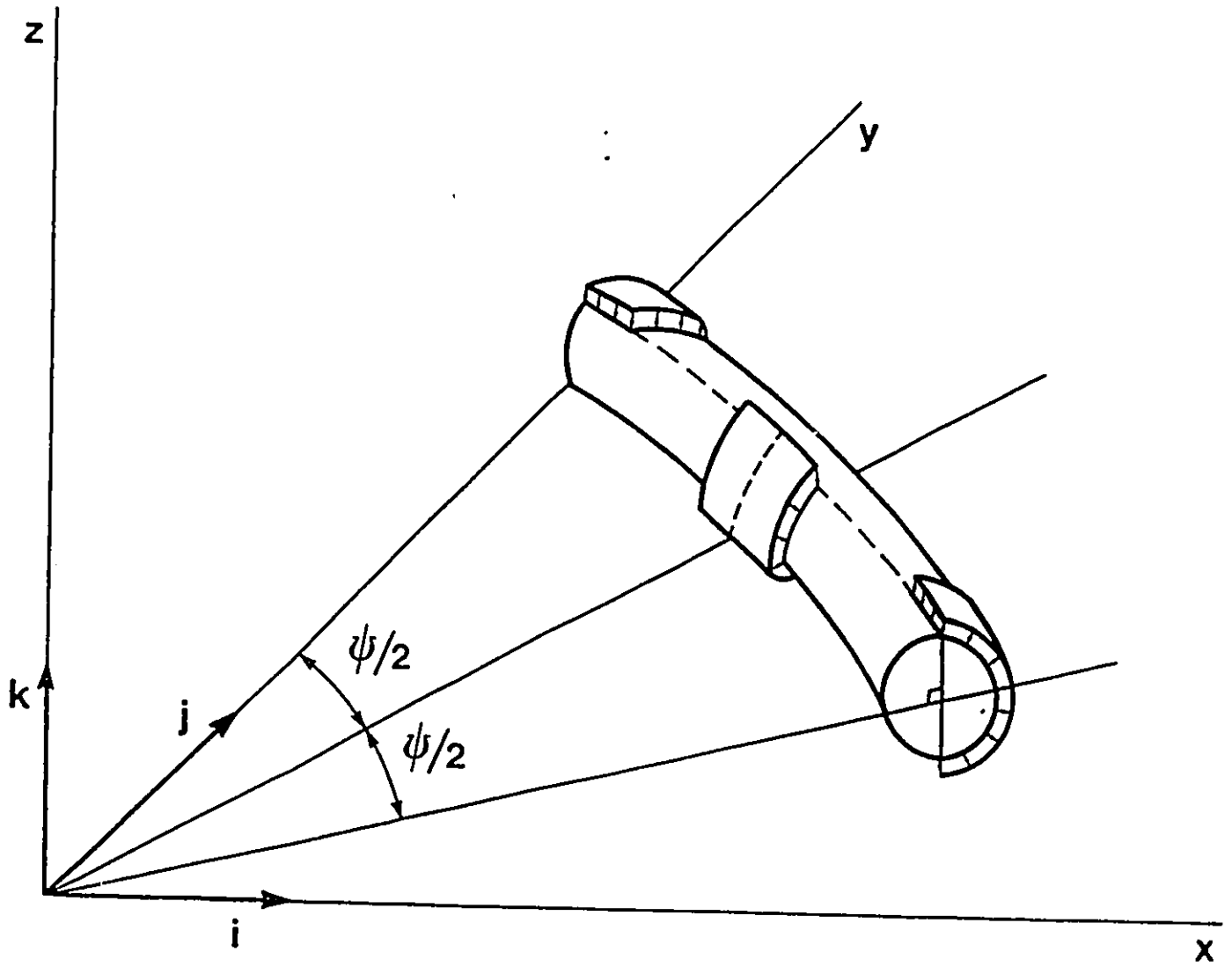


Figure 4.7: Ram bending of a curved pipe.

ORIGINAL  0.0161



10	9	8	7	6	5	4	3	2	1
20	19	18	17	16	15	14	13	12	11
30	29	28	27	26	25	24	23	22	21
40	39	38	37	36	35	34	33	32	31
50	49	48	47	46	45	44	43	42	41
60	59	58	57	56	55	54	53	52	51
70	69	68	67	66	65	64	63	62	61
80	79	78	77	76	75	74	73	72	71

Figure 4.8: FEM mesh for the case of the ram bending.

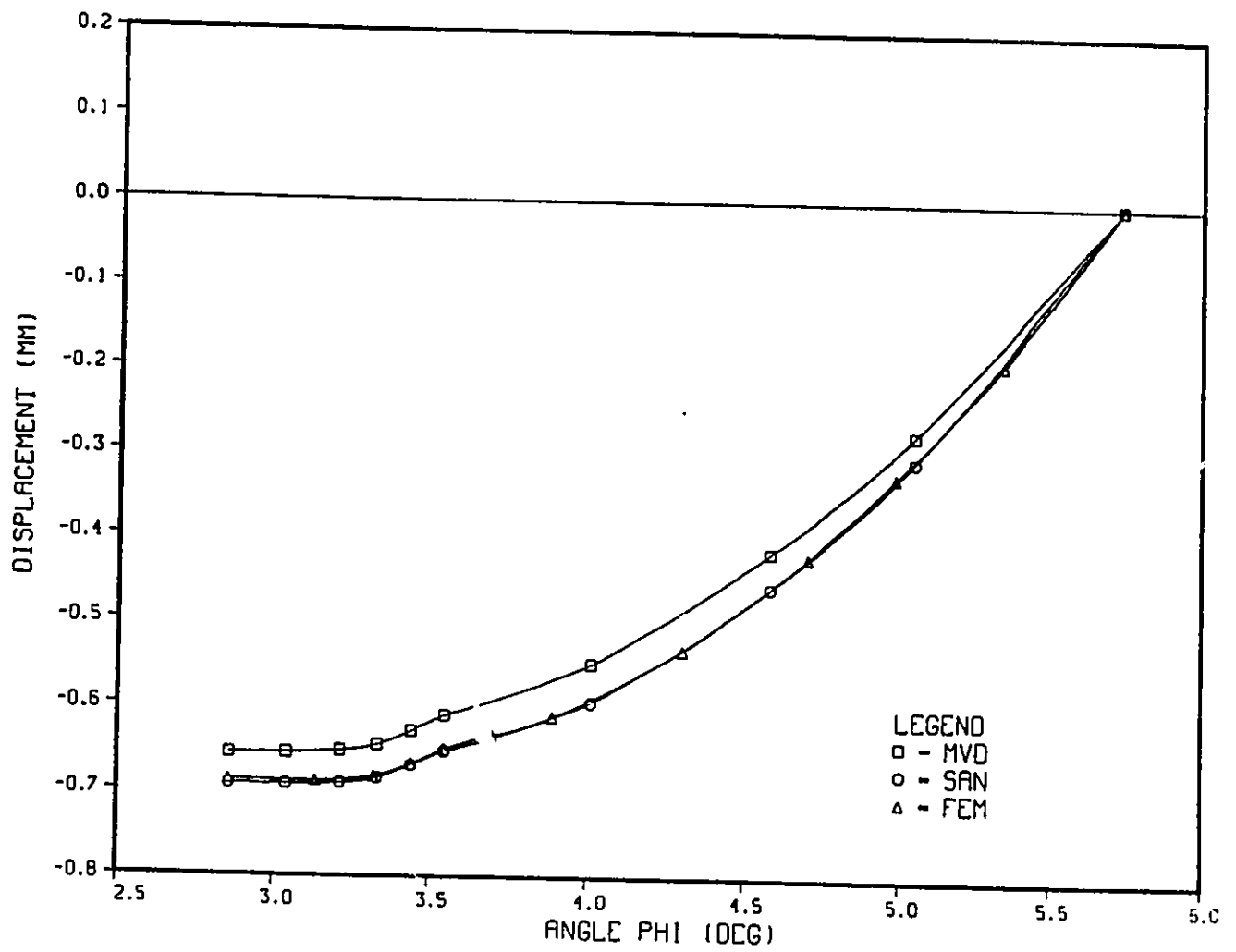


Figure 4.9: Comparison of results for radial displacement w at $\theta = 180^\circ$.

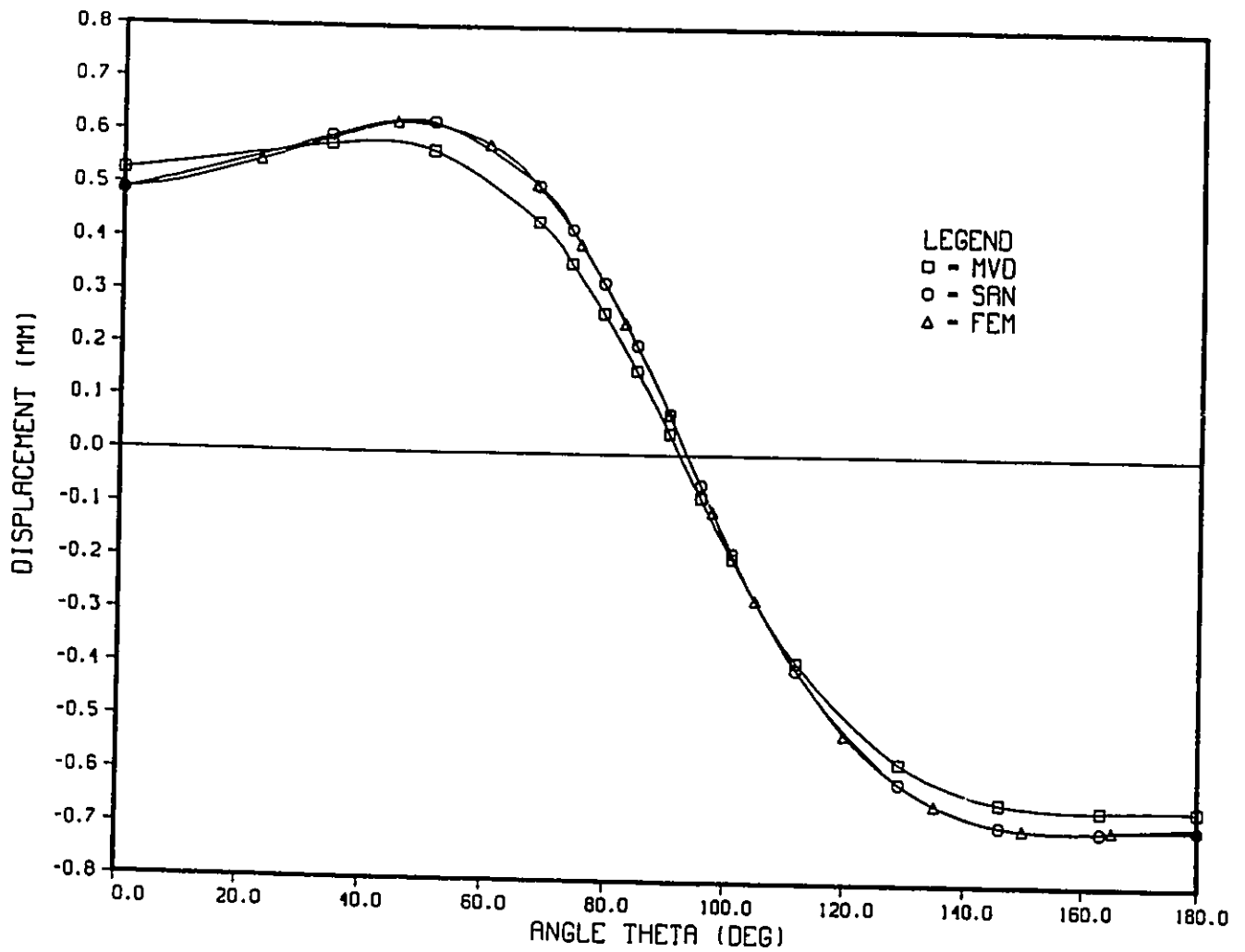


Figure 4.10: Comparison of results for radial displacement w on transverse plane of symmetry.

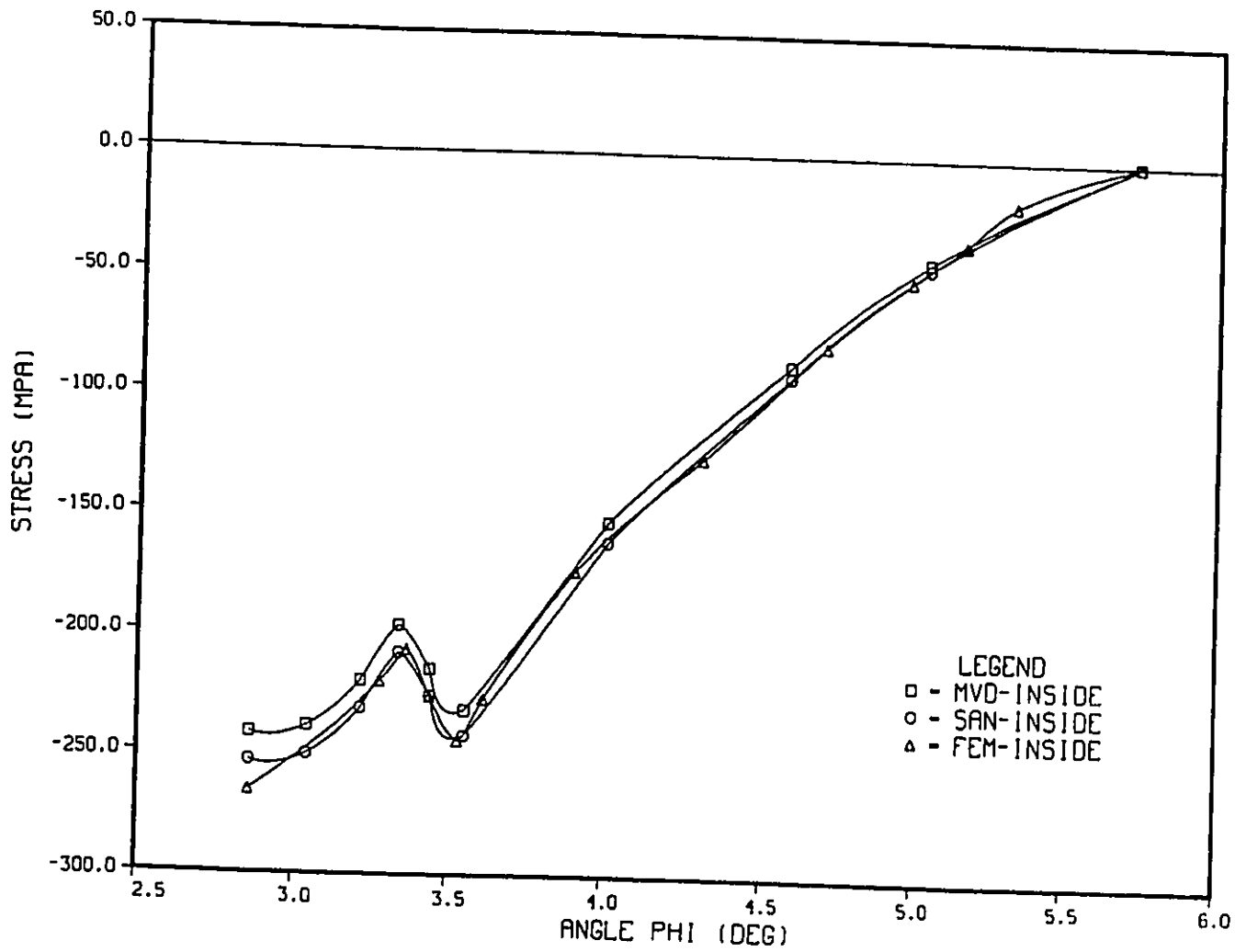


Figure 4.11: Comparison of results for longitudinal stress σ_n at $\theta = 180^\circ$.

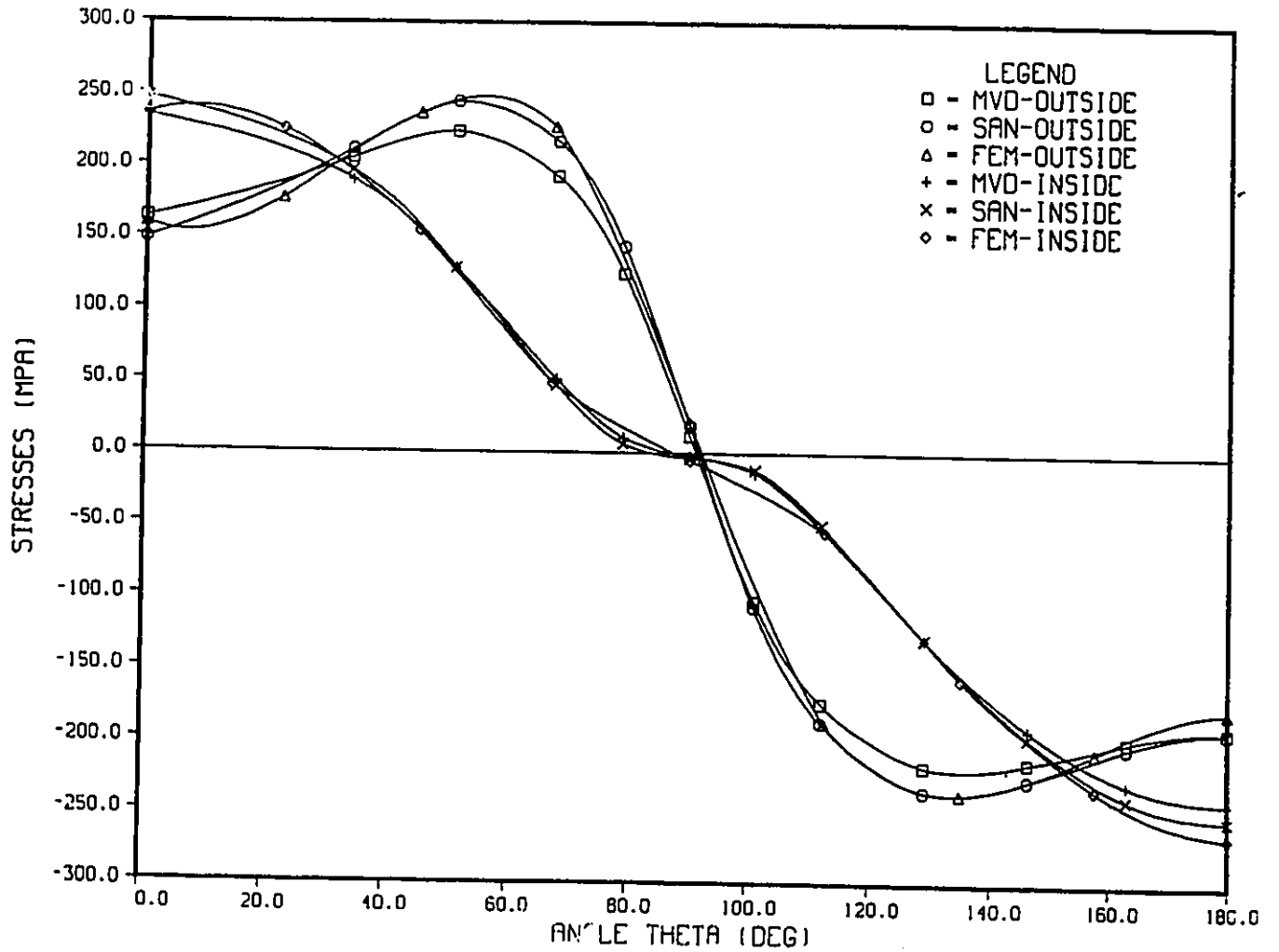


Figure 4.12: Comparison of results for longitudinal stress σ_n on transverse plane of symmetry.

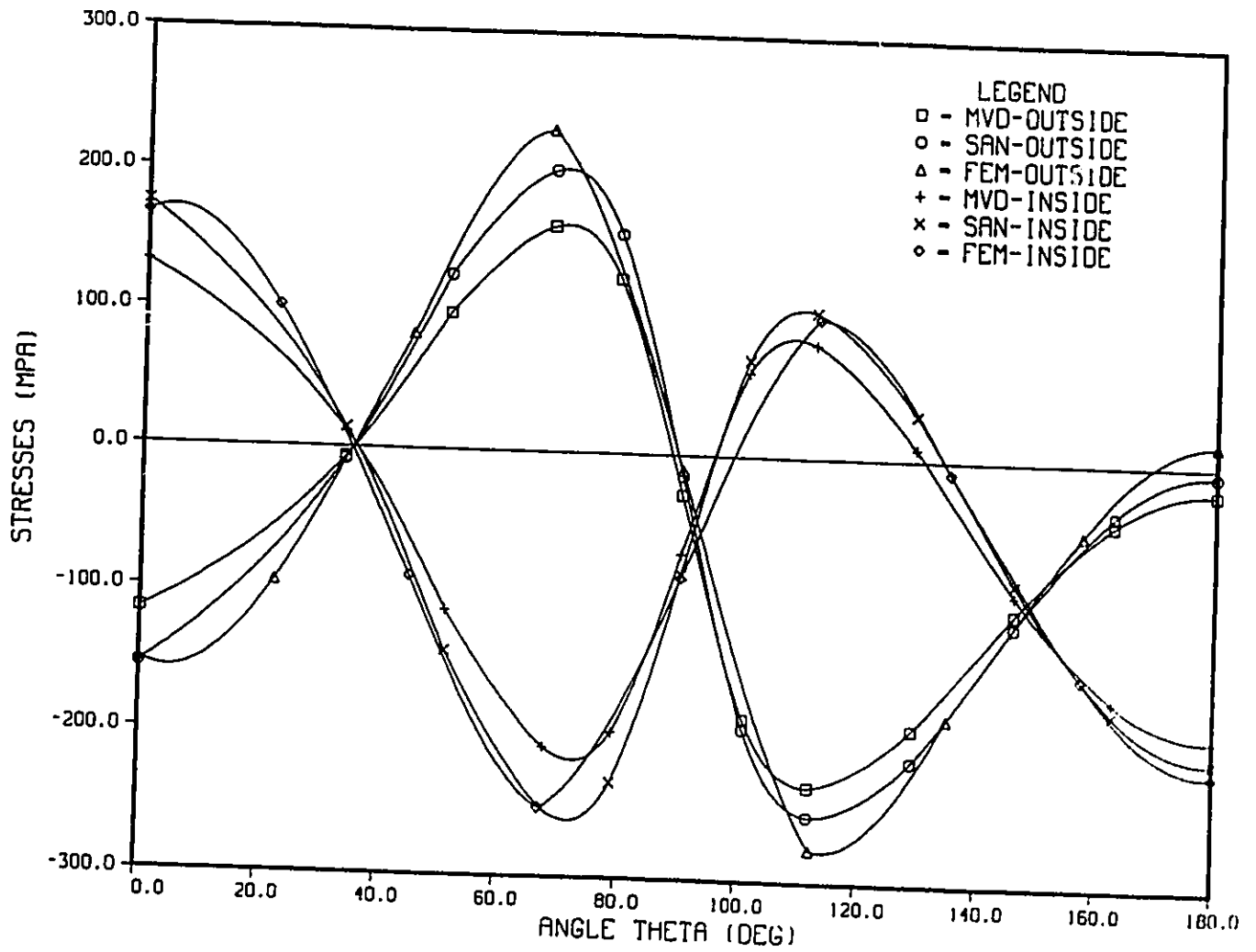


Figure 4.13: Comparison of results for circumferential stress σ_θ on transverse plane of symmetry.

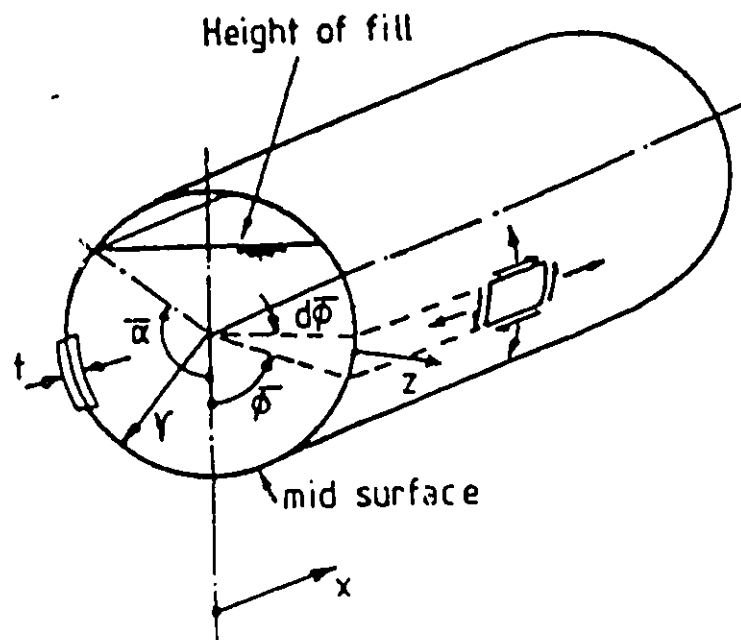


Figure 4.14: Cylindrical vessel.

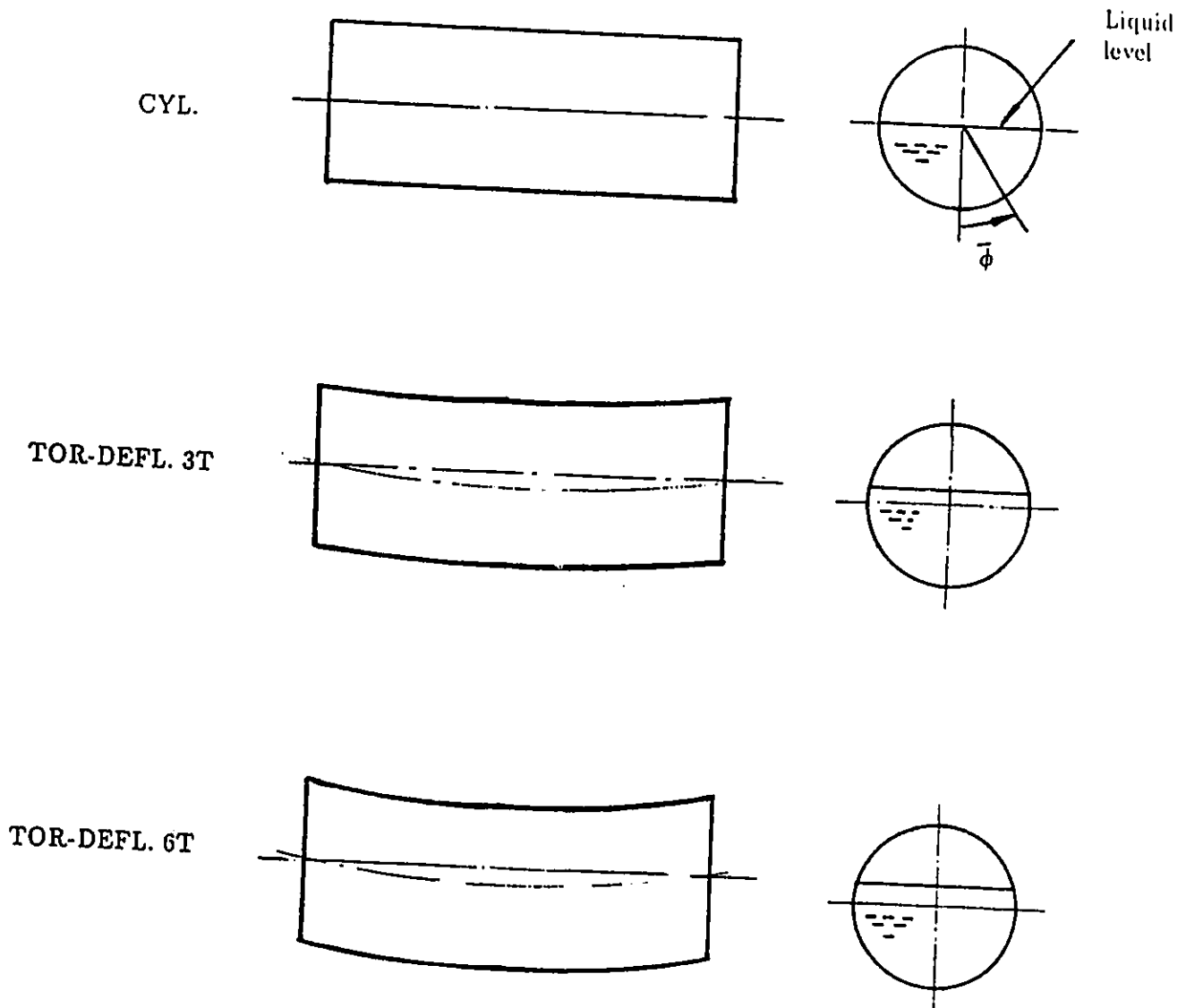


Figure 4.15: Vessels half-filled with fluid.

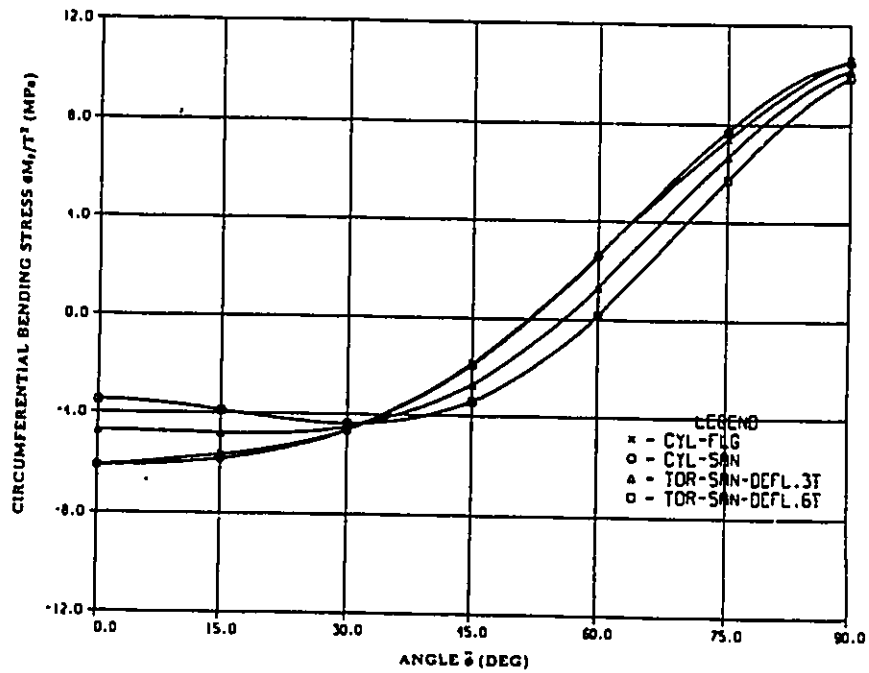
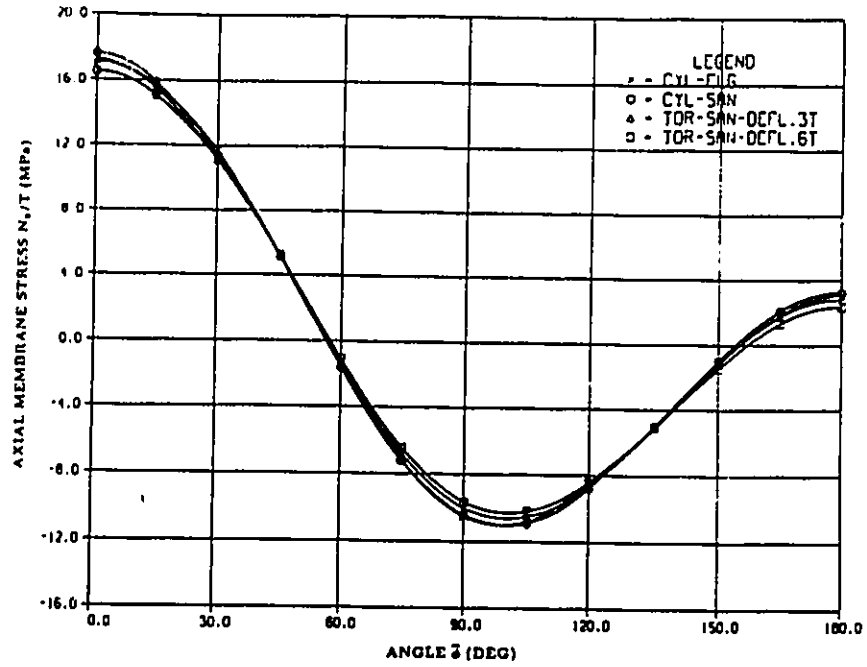


Figure 4.16: Comparison with Flügge solution

Table 4.1: Changes in the diameters for the surface loading problem

ϕ°	$\Delta Horiz. Diam. (mm)$			$\Delta Vert. Diam. (mm)$		
	<i>SAN</i>	<i>FEM</i>	<i>SANU</i>	<i>SAN</i>	<i>FEM</i>	<i>SANU</i>
0.00	0.0	0.0	0.0	0.0	0.0	0.0
11.25	-0.0049	-0.0052	-0.0049	0.0056	0.0055	0.0056
22.50	-0.0119	-0.0121	-0.0118	0.0123	0.0124	0.0122
30.00	-0.0216	-0.0219	-0.0216	0.0169	0.0171	0.0169
37.50	-0.0303	-0.0307	-0.0309	0.0206	0.0208	0.0204
45.00	-0.0323	-0.0327	-0.0324	0.0219	0.0221	0.0217
52.50	-0.0303	-0.0307	-0.0309	0.0206	0.0208	0.0204
60.00	-0.0216	-0.0219	-0.0216	0.0169	0.0171	0.0169
67.50	-0.0119	-0.0121	-0.0118	0.0123	0.0124	0.0122
78.75	-0.0049	-0.0052	-0.0049	0.0056	0.0055	0.0056
90.00	0.0	0.0	0.0	0.0	0.0	0.0

Table 4.2: Changes in the diameters for the ram bending problem

ϕ°	$\Delta Horiz. Diam.$			$\Delta Vert. Diam.$		
	<i>MVD</i>	<i>SAN</i>	<i>FEM</i>	<i>MVD</i>	<i>SAN</i>	<i>FEM</i>
2.86	-0.1312	-0.2038	-0.1909	0.0793	0.1525	0.1372
3.04	-0.1313	-0.2036	-0.1887	0.0785	0.1514	0.1340
3.21	-0.1307	-0.2021	-0.1832	0.0768	0.1489	0.1334
3.44	-0.1142	-0.1837	-0.1656	0.0882	0.1581	0.1426
3.66	-0.0964	-0.1630	-0.1446	0.0982	0.1650	0.1489
4.01	-0.0885	-0.1486	-0.1322	0.0880	0.1480	0.1316
4.58	-0.0667	-0.1113	-0.0985	0.0652	0.1094	0.0922
5.04	-0.0452	-0.0735	-0.0657	0.0393	0.0671	0.0496
5.33	-0.0529	-0.0696	-0.0603	-0.0023	0.0141	0.0096
5.73	0.0	0.0	0.0	0.0	0.0	0.0

Chapter 5

Local Loads on a Toroidal Shell

5.1 Introduction

In this chapter, the solution of the Sanders shell theory is applied to the problem of local loads on a sectorial toroidal shell. The solution corresponds to the case of a shell simply supported at the two ends. The effect of pads of uniform normal pressure on the surface at three positions of a 90° piping elbow is considered. These loads are centered at the crown circles, the extrados, and the intrados (Fig. 5.1). Displacement results, as well as stress results, are presented. All results assume linear elastic small-displacement behavior. The results are compared with a similarly loaded cylindrical shell of equal radius, thickness and center-line length. The locations of maximum displacement and stress are identified. Finally three tables are given summarizing characteristic displacements and stresses for a number of shells, covering a wide range of geometric parameters.

5.2 Results for Typical Piping Elbow

Detailed numerical results are presented in this section for a 90° piping elbow having the following values for the geometric and material parameters

$$r = 84.2 \text{ mm}; R = 252 \text{ mm}; t = 7.1 \text{ mm} \quad (5.1)$$

$$E = 207,000 \text{ MPa}; \nu = 0.3$$

$$\phi = 45^\circ$$

These dimensions correspond to a typical pipe used in a power or chemical plant [53]. Results are also given for a comparable straight pipe.

To characterize the geometry of pipe bends the factor Γ has been used [56], while for straight pipes the factors Ω and \bar{K} are often used [56,57]. These factors are defined as

$$\Gamma = \frac{r^2}{Rt}; \Omega = \frac{lt^{1/2}}{r^{3/2}}; \bar{K} = \Omega^{1/2} \quad (5.2)$$

where l is the straight pipe center-line length. The values for these three factors for the shell of eqn. (5.1) are $\Gamma = 3.96$, $\Omega = 1.36$, and $\bar{K} = 1.16$, with l taken as 396mm.

Results are presented for three cases of local loads (Fig. 5.1). For the first case, corresponding to a 'pinched' shell, the load positions are described by

$$\beta = 90^\circ; \lambda = 15^\circ; \alpha = 7.5^\circ; \delta = 7.5^\circ$$

where the parameters are those of eqn. 3.22.

For convenience this is referred to as the ' $p_1 = 1\text{MPa}$ ' case. There are two patches of loadings on the crown circles centered circumferentially at $\theta = 90^\circ$ and 270° . Each load extends 30° circumferentially and from 30° to 60° longitudinally (Fig. 3.2).

For the second case, corresponding to a load at the extrados, the load position is

given by

$$\beta = 15^\circ; \lambda = 15^\circ; \alpha = 7.5^\circ; \delta = 7.5^\circ$$

It is referred to as the ' $p_2 = 1MPa$ ' case. The load extends from -30° to 30° circumferentially, and from 30° to 60° longitudinally.

For the third case, corresponding to a load at the intrados, the load position is described by

$$\beta = 165^\circ; \lambda = 15^\circ; \alpha = 7.5^\circ; \delta = 7.5^\circ$$

It is referred to as the ' $p_3 = 1MPa$ ' case. The load extends from 150° to 210° circumferentially, and from 30° to 60° longitudinally.

The convergence of the toroidal shell series solution is further examined in Fig. 5.2 with regard to a $p_2 = 1MPa$ loading case. The longitudinal stress σ_η on the inside shell surface is plotted in the figure versus the longitudinal angle ϕ . Results are given for a 'short', 'medium-length', and 'long' series solution. It is seen that the convergence is rapid. For the 'medium-length' series for which the truncation integers are $M = 51, N = 25$, the results agree very closely with those for the 'long' series. Accordingly the 'medium-length' series was used throughout this study to determine the numerical results.

Results for the three loading cases (Fig. 5.1) are given in Figs. 5.3-5.11. For each load three figures are presented, giving respectively the normal displacement w , the circumferential and longitudinal normal stresses $\sigma_\theta, \sigma_\eta$ on the transverse plane of symmetry $\phi = 45^\circ$. For the stresses σ_θ and σ_η , results are given for the inside and outside surfaces. Plotted in each figure are results for the toroidal shell as well as results for a comparable cylindrical shell having the same radius, thickness and center-line length. In each case the boundary conditions at the shell ends are the same, and the total applied

force for the cylindrical shell is equal to that for the toroidal shell. The length of the load pad in the cylindrical shell equals in each case the curved length of the load pad of the toroidal shell.

The toroidal shell results were obtained using the FORTRAN program TORSAN, which is based on the theory presented in Chapter 3. The cylindrical shell results were obtained using the program CYLPAD [19], using the Sanders theory option, with a set of truncation integers of $M = 51, N = 25$. Comparison with FEM results were made for the TORSAN results in Chapter 4, and for the CYLPAD results in ref. [19]. For both programs close agreement was observed in the displacement and stress results from the two methods for local load cases.

Results for the radial displacement w for the 'pinched' shell case ($p_1 = 1MPa$) are given in Fig. 5.3. The maximum displacement under the load for the toroidal shell is very close to that of the cylindrical shell. The displacement pattern for the toroidal shell indicates an inward rigid-body translation of the cross-section resulting in an overall small outward displacement at the extrados and an overall large outward displacement at the intrados.

The results for the stresses σ_θ and σ_η for the 'pinched' shell (Fig. 5.4 and 5.5) indicate maximum values which are nearly equal for the toroidal and cylindrical shells. The stresses in the toroidal shell near the extrados are smaller than those in the cylindrical shell at 90° from the load center, while stresses σ_η on the outside surface and σ_θ at the intrados are greater. Overall for the 'pinched' shell case the cylindrical shell results give a close indication of the results in the toroidal shell.

The results for the case of loading at the extrados ($p_2 = 1MPa$) are presented in Figs. 5.6, 5.7 and 5.8. For the radial displacement w the maximum value for the toroidal shell is less than one-half that for the cylindrical shell. The load is situated at

a positive Gaussian curvature portion of the toroidal shell and thus superior results are to be expected with respect to the cylindrical shell. It is noted that the deflection in the cylindrical shell at a position of 180° from the load center is inward. A finite element method analysis was carried out which served to confirm this particular result.

The results for σ_r on the outside surface and σ_θ in Fig. 5.8 and 5.7 indicate that much greater stresses arise in the cylindrical shell than in the toroidal shell. The stresses are rather localized and decay rapidly away from the loading area. Overall the results indicate that for a local load at the extrados a toroidal shell experiences much smaller deflections and stresses than a comparable cylindrical shell.

The results for the case of loading at the intrados ($p_3 = 1 \text{ MPa}$) are presented in Figs. 5.9, 5.10 and 5.11. In this case the maximum value for the radial deflection w for the toroidal shell significantly exceeds that for the cylindrical shell. The quantitative results for the displacements indicate clearly that the intrados of a toroidal shell is most vulnerable to deformation under local loading. Stress results for the third loading case are given in Fig. 5.10 and 5.11. The stress patterns in the toroidal and cylindrical shells are remarkably similar. The maximum values σ_r on the inside surface and σ_θ in the toroidal shell exceed those in the cylindrical shell.

5.3 Parametric Study

The results discussed in the preceding section are for the particular shell described in eqns. (5.1). Ten additional shell cases are described in Tables 5.1-5.3. The geometric parameters are given in Table 5.1. Cases 1 to 3 are relatively thin shells whereas cases 9 and 10 are thick shells, not strictly within the validity of thin shell theory. The last two cases are considered to include cylindrical shells with $\Omega \geq 3$ ($\bar{K} \geq 1.7$), for which

Calladine [56] predicts mainly 'beam'-type behavior.

Presented in Tables 5.2 and 5.3 are the results for toroidal shells loaded at the extrados and intrados, and the results for comparable cylindrical shells. The four shells analyzed under each case were subjected to the same total load. For the toroidal shells the loads extended through a 30° longitudinal arc, giving longer load pad lengths at the extrados than at the intrados. The load intensity for the extrados loading thus was smaller than that for the intrados loading. Two cylindrical shells were analyzed for each case, with load pad lengths equal to the two corresponding toroidal shell load pad lengths, and load intensities to suit. In Tables 5.2 and 5.3 results are given for the radial displacement w , and the membrane and bending parts of the longitudinal (σ_η) and circumferential (σ_θ) normal stresses at the center of the load pad. Values at this position are at or near the maximum and are referred to in the following as the characteristic values. The membrane stress in each table appears above the bending stress. The results of Table 5.2 represent the case of the loading at the extrados while those of Table 5.3 the case of the loading at the intrados.

The characteristic radial displacement values for toroidal shells in general are smaller than the values for the corresponding cylindrical shell, if the load is at the extrados, and greater if the load is at the intrados. The differences tend to decrease as the \bar{K} ratio increases. For $\bar{K} > 1.7$ exceptions arise for the toroidal shell loaded at the extrados.

For the cylindrical shells the characteristic longitudinal membrane stresses are greater than the bending stresses, while the characteristic circumferential membrane stresses are smaller than the bending stresses, for all \bar{K} . For the toroidal shell loaded at the extrados the characteristic longitudinal membrane stresses are greater than the bending stresses for all \bar{K} , while the characteristic circumferential membrane stresses are greater than the bending stresses for $\bar{K} < 1$. For the toroidal shell loaded at the intrados the characteristic longitudinal and circumferential membrane stresses are smaller than the

bending stresses, for all \bar{K} . For loads at the intrados the theory predicts positive (tensile) characteristic longitudinal membrane stresses at the intrados for $\bar{K} < 1.3$. Relatively large outward longitudinal deflections v at the intrados are predicted for these cases at the shell ends. Such displacements are permissible with the boundary conditions defined.

For $\bar{K} < 1.7$ the characteristic membrane and bending stresses in the toroidal shells loaded at the extrados are generally smaller than those for the cylindrical shell. Also for $\bar{K} < 1.7$ the membrane stresses for the toroidal shell loaded at the intrados are generally smaller than those for the cylindrical shell, while the bending stresses are greater. For $\bar{K} > 1.7$ the membrane stresses in the toroidal shell loaded at either the extrados or intrados are similar in magnitude to the membrane stresses in the comparable cylindrical shells. Also for $\bar{K} > 1.7$, the characteristic bending stresses in the toroidal shells loaded at the extrados are smaller than the bending stresses in the comparable cylindrical shell, while the characteristic bending stresses in the toroidal shell loaded at the intrados are greater.

In summary, for $\bar{K} < 1.7$ a local load generally induces smaller characteristic bending stresses in a toroidal shell than in a cylindrical shell if the load is at the extrados, and greater stresses if the load is at the intrados. The results indicate that when shell action predominates a toroidal shell shows superior structural qualities compared to a cylindrical shell if the load is at the extrados, and inferior qualities if the load is at the intrados.

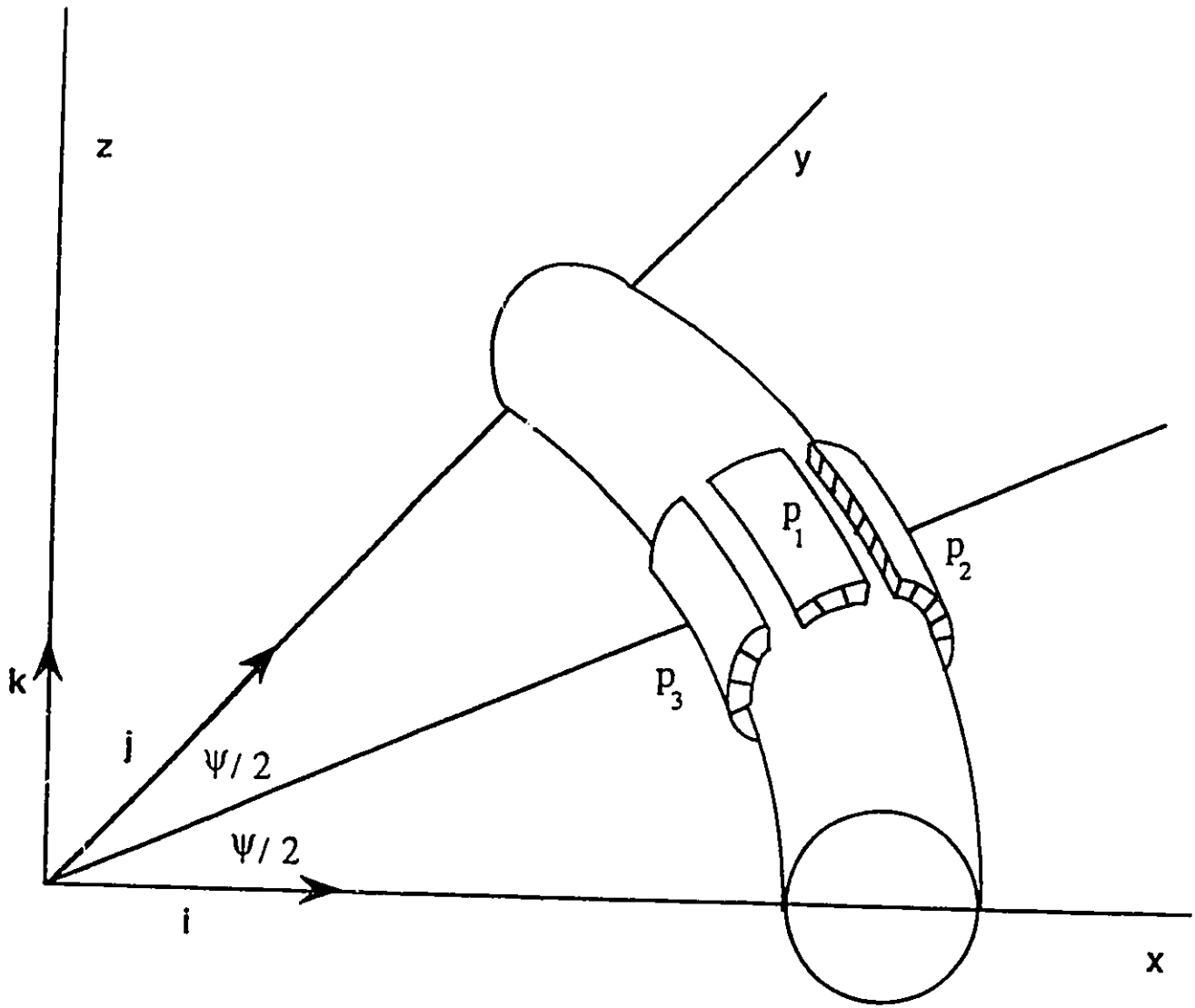


Figure 5.1: Loading cases considered.

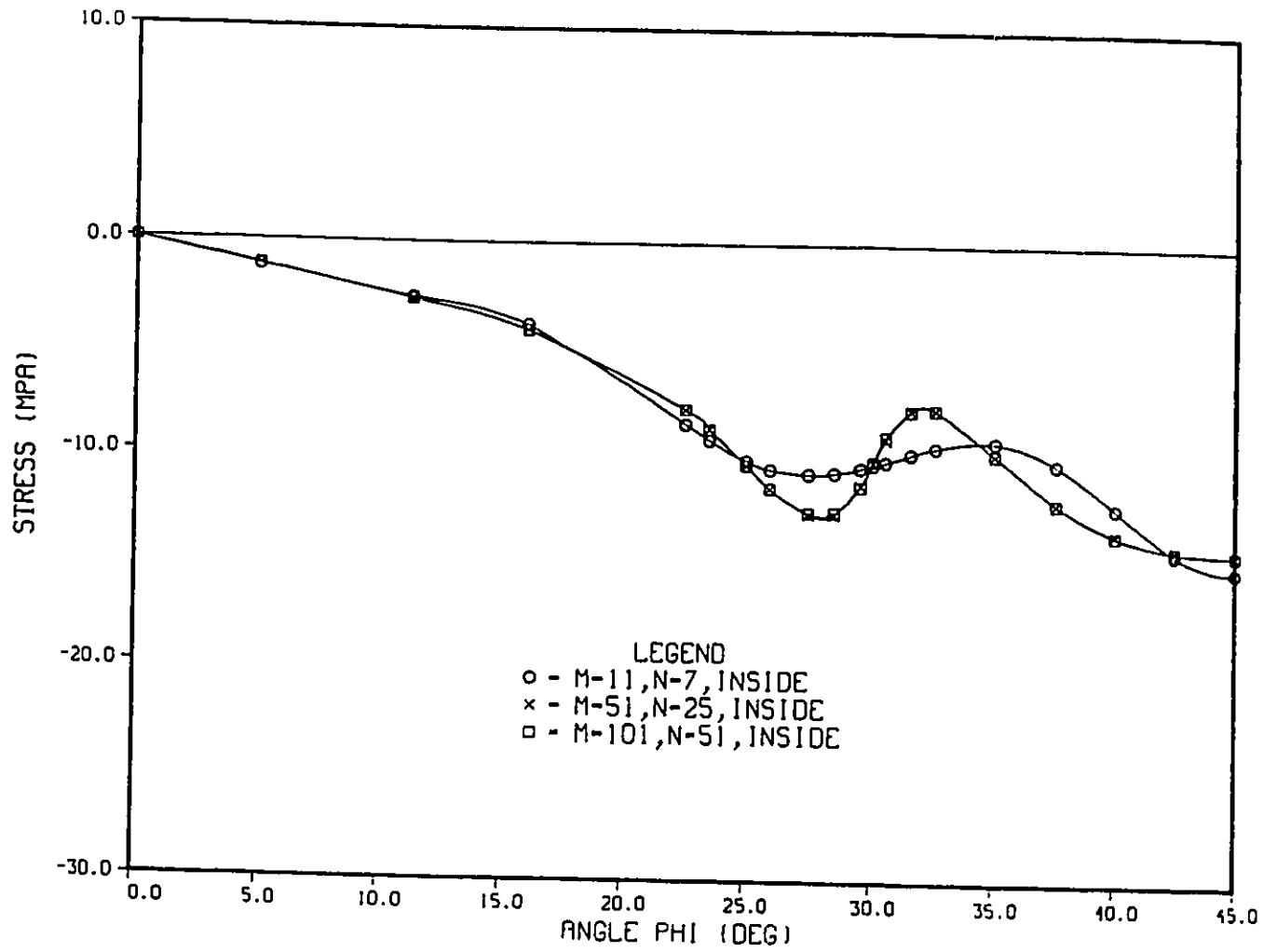


Figure 5.2: Convergence of the Fourier series for the stress σ_η .

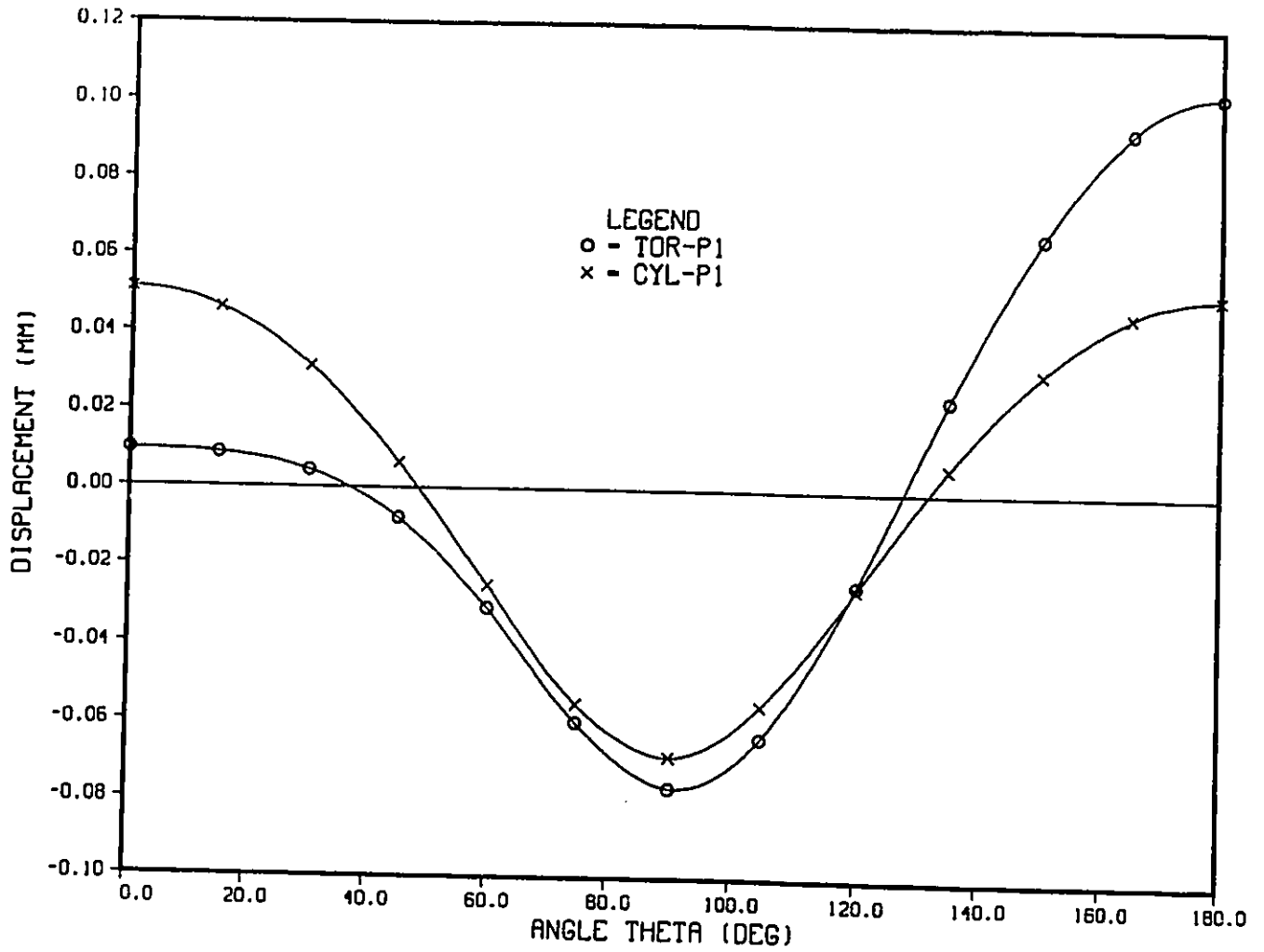


Figure 5.3: Radial displacement w at $\phi = 45^\circ$ for a 'pinched' shell ($p_1 = 1$).

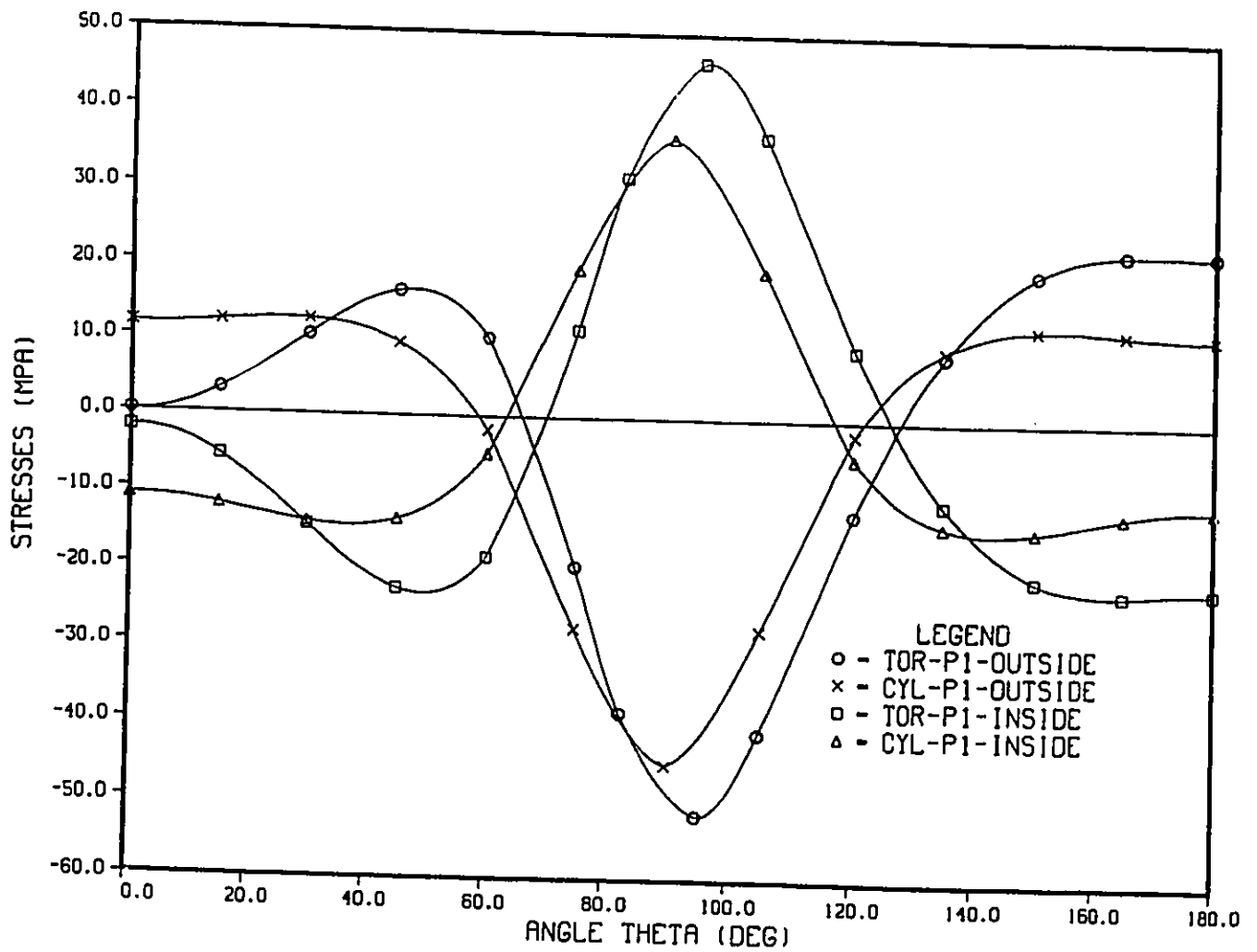


Figure 5.4: Stress σ_θ at $\phi = 45^\circ$ for a 'pinched' shell ($p_1 = 1$).

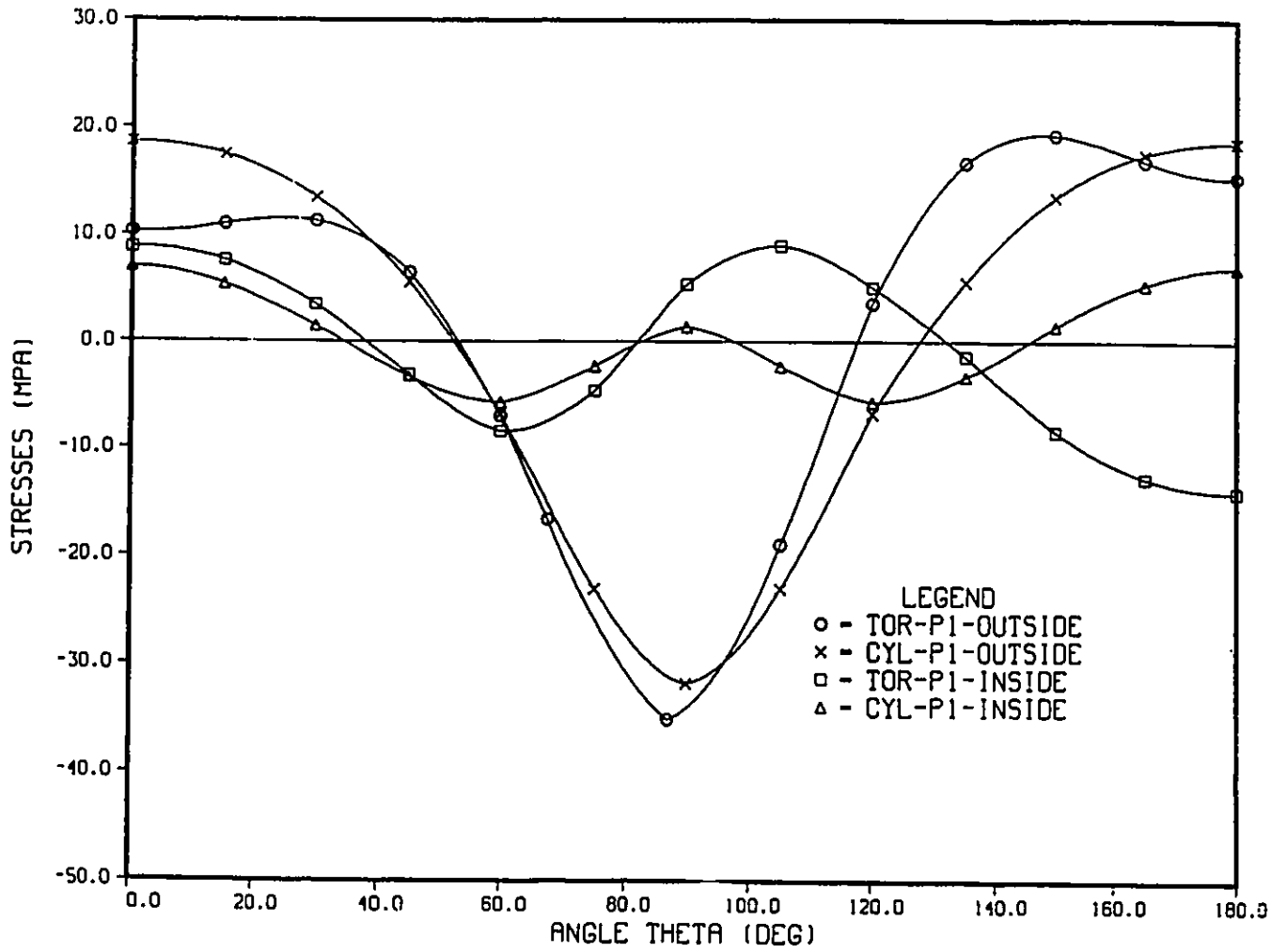


Figure 5.5: Stress σ_n at $\phi = 45^\circ$ for a 'pinched' shell ($p_1 = 1$).

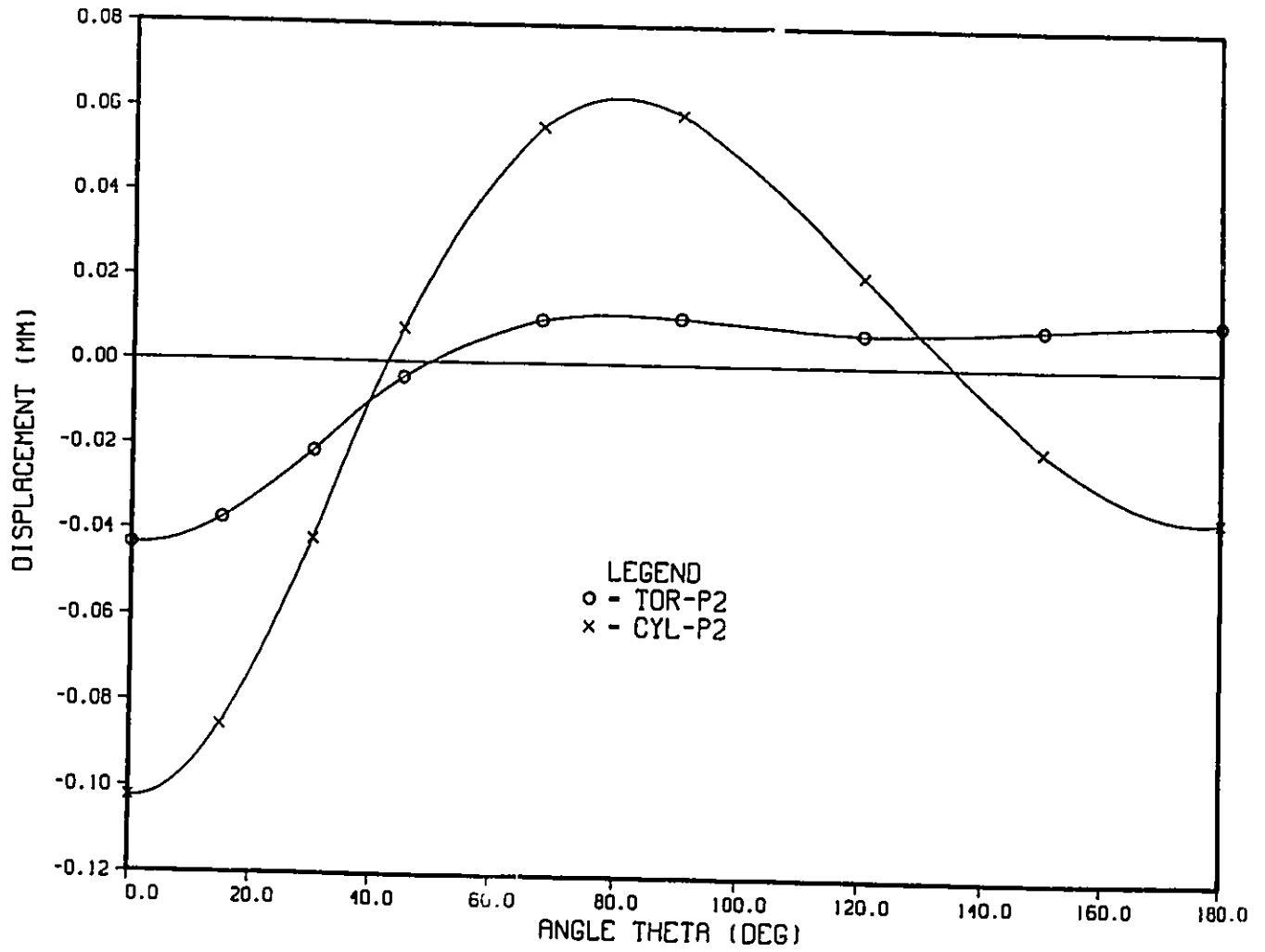


Figure 5.6: Radial displacement w at $\phi = 45^\circ$ for shell loaded at extrados ($p_2 = 1$).

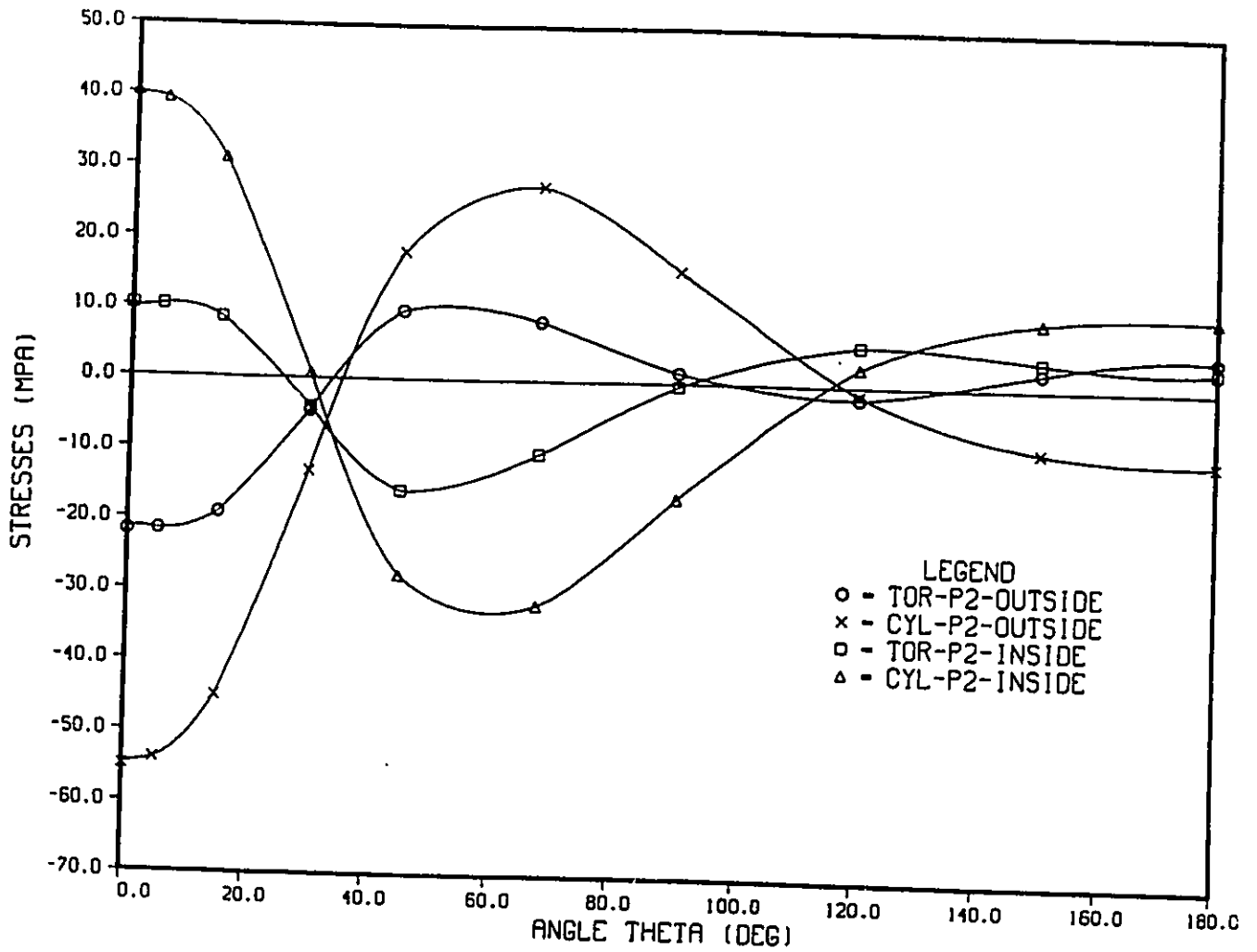


Figure 5.7: Stress σ_θ at $\phi = 45^\circ$ for shell loaded at extrados ($p_2 = 1$).

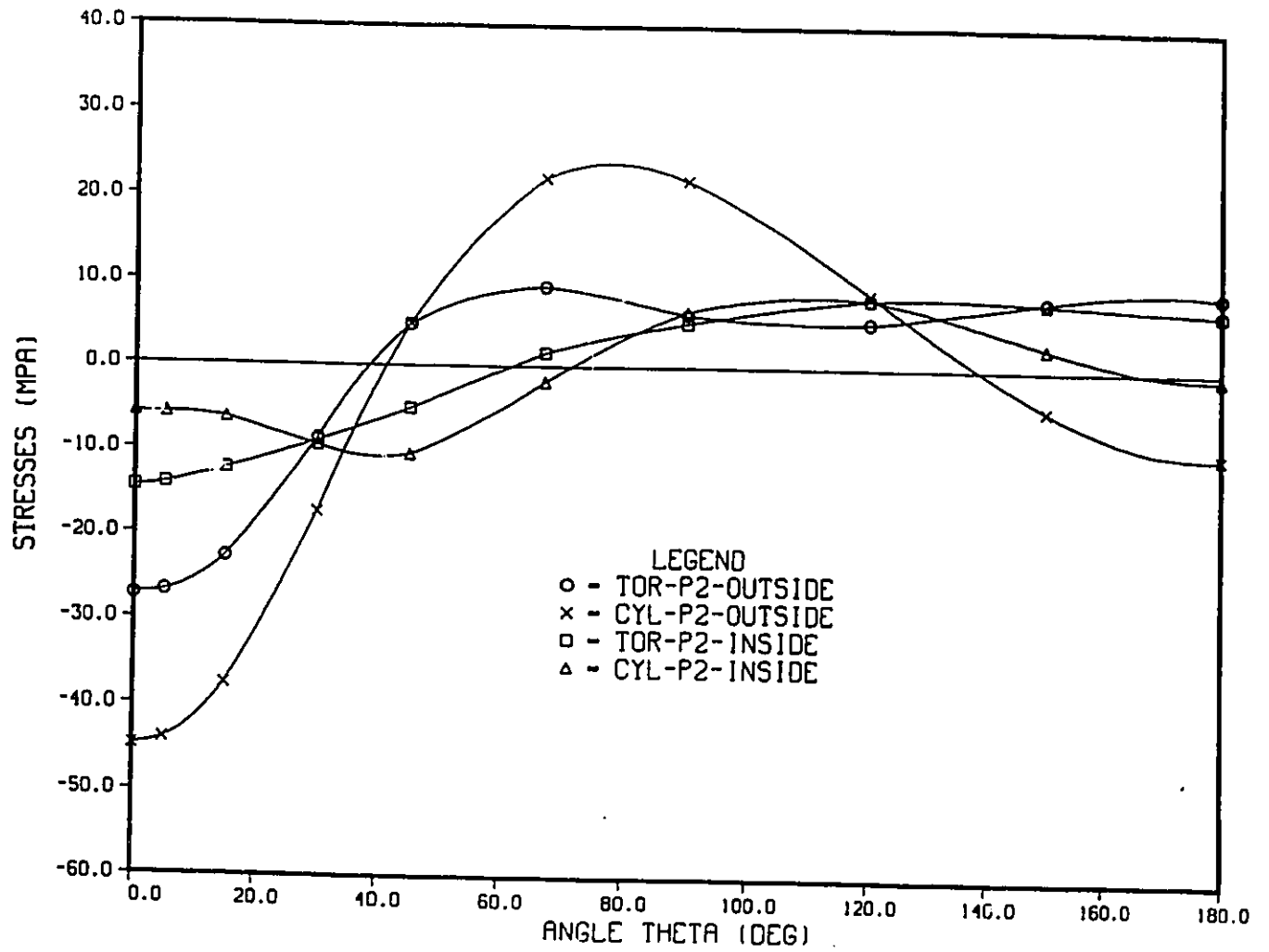


Figure 5.8: Stress σ_η at $\phi = 45^\circ$ for shell loaded at extrados ($p_2 = 1$).

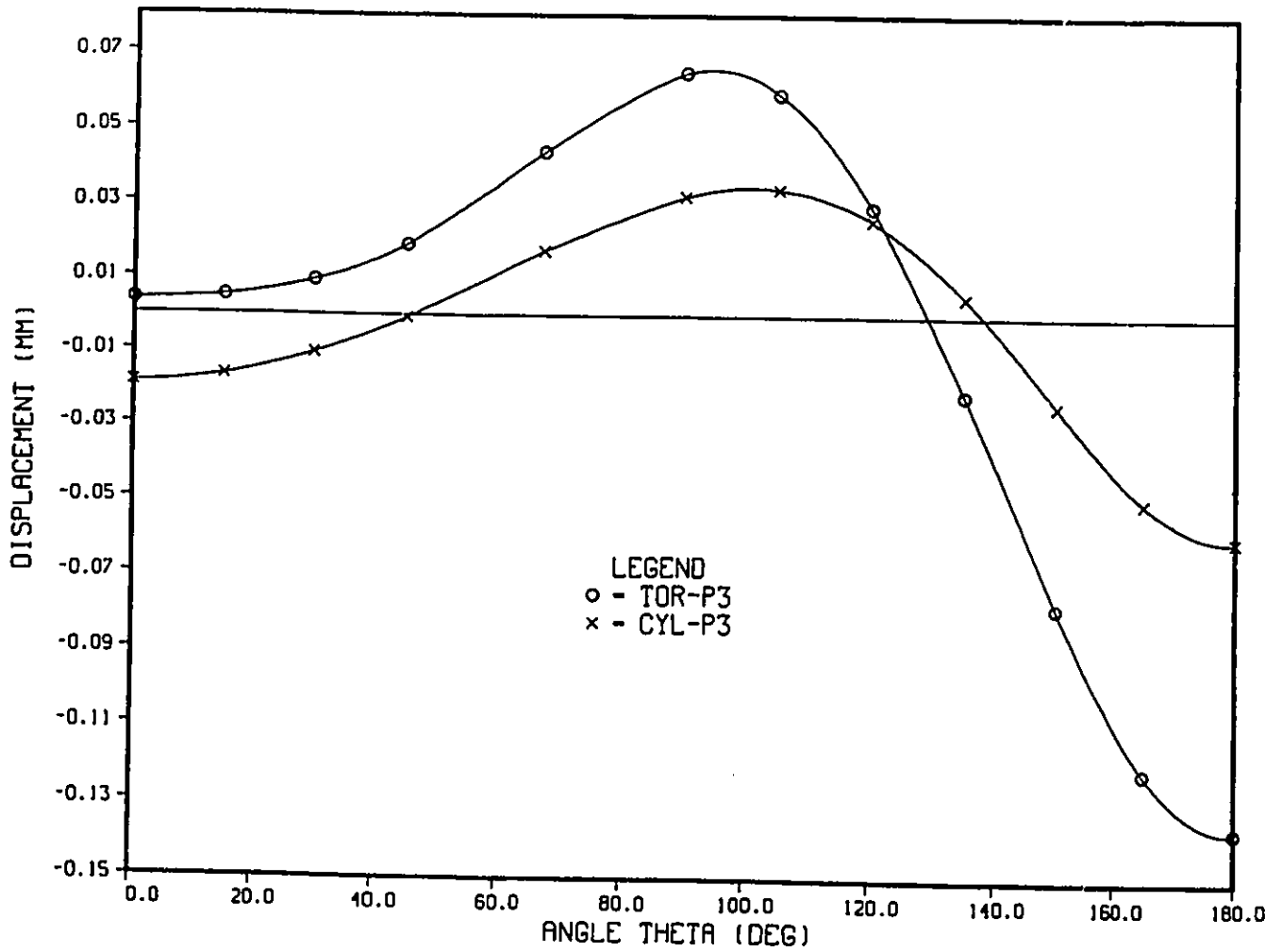


Figure 5.9: Radial displacement w at $\phi = 45^\circ$ for shell loaded at intrados ($p_3 = 1$).

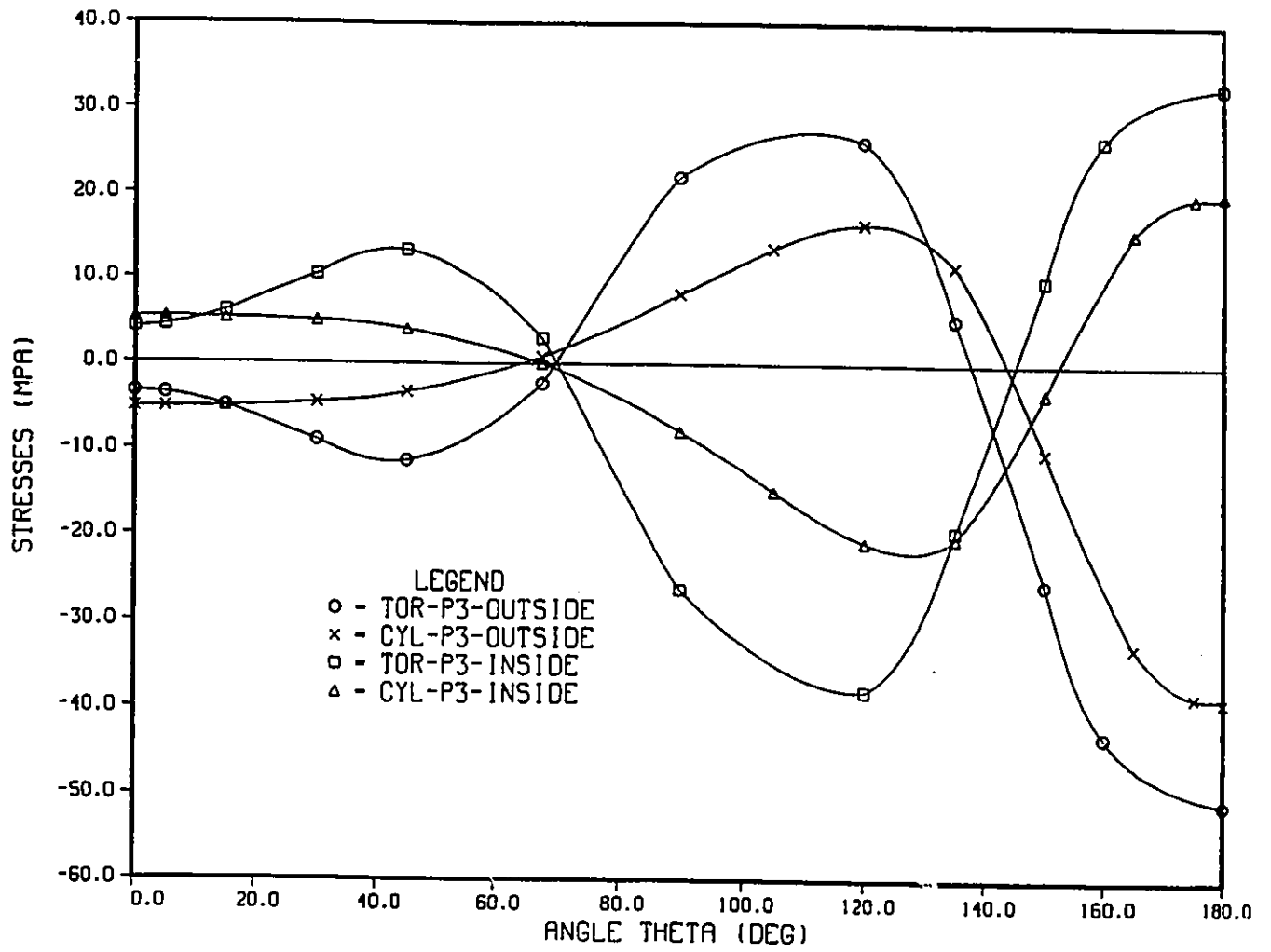


Figure 5.10: Stress σ_θ at $\phi = 45^\circ$ for shell loaded at intrados ($p_3 = 1$).

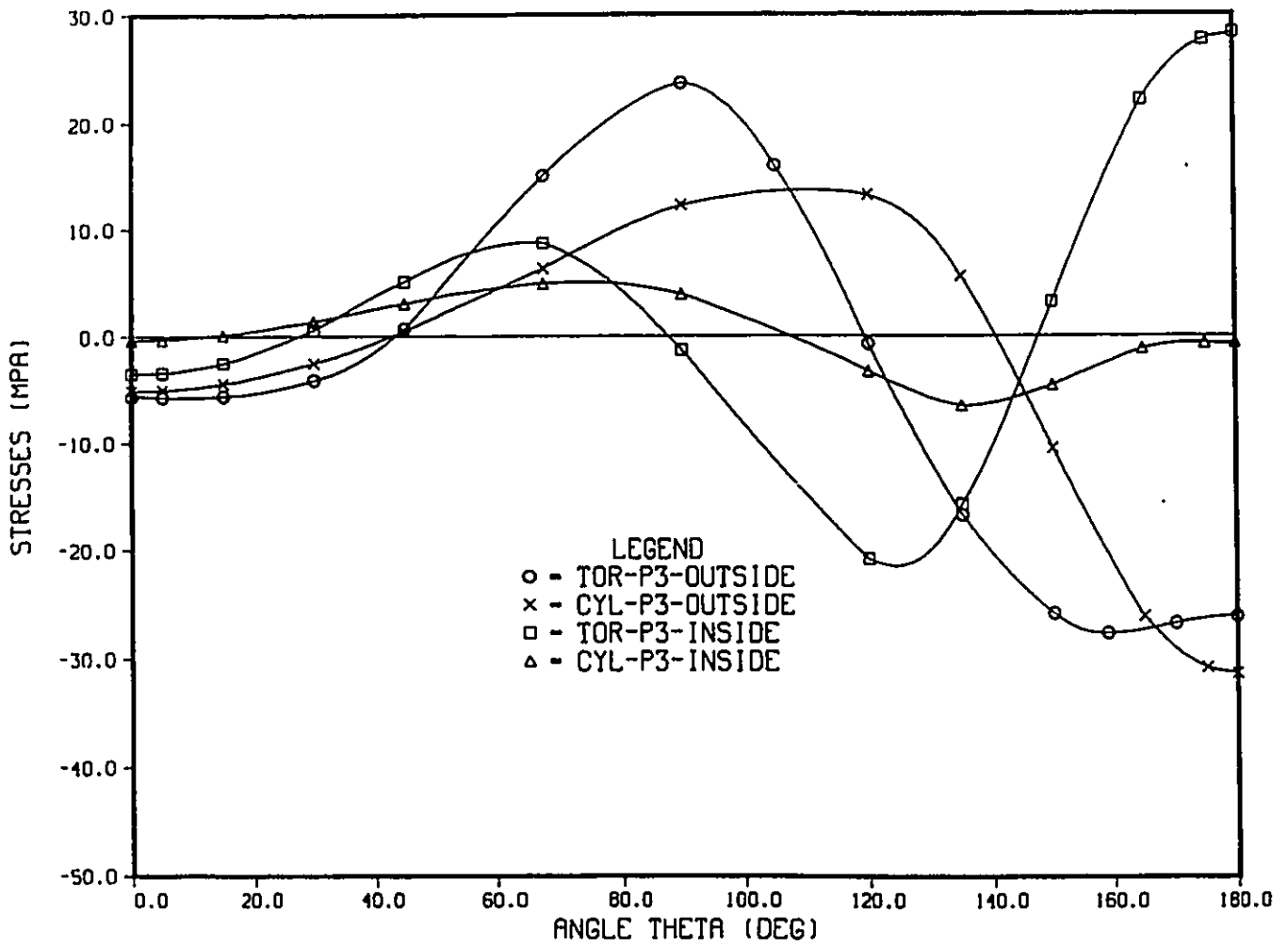


Figure 5.11: Stress σ_η at $\phi = 45^\circ$ for shell loaded at intrados ($p_3 = 1$).

Table 5.1: Geometry of Additional Load Cases

(distances are in mm)

<i>Case</i>	<i>r</i>	<i>R</i>	<i>t</i>	<i>l</i>	Γ	Ω	\bar{K}
1	100	300	.19	471	175.4	.203	.45
2	100	300	.33	471	101.0	.270	.52
3	100	300	.48	471	69.4	.325	.57
4	100	300	1.14	471	29.2	.504	.71
5	100	300	1.8	471	18.5	.640	.8
6	100	300	4.5	471	7.4	1.0	1.0
7	100	400	6.2	628	4.0	1.563	1.25
8	100	400	10.0	628	2.5	1.988	1.41
9	100	600	10.6	943	1.6	3.063	1.75
10	100	600	18.0	943	.9	4.0	2.0

Table 5.2: Characteristic Displacements and Stresses - Load at Extrados

(deflection is in *mm*, pressure and stresses are in *MPa*)

Case	Pres.	Cyl. Shell			Tor. Shell		
	<i>p</i>	<i>w</i>	σ_x	σ_θ	<i>w</i>	σ_η	σ_θ
1	.005	-.268	-27.67	-2.95	-.020	-4.89	-1.35
			2.52	8.85			
2	.005	-.167	-15.12	-1.67	-.010	-2.82	-.79
			-.86	-2.11			
3	.005	-.102	-9.47	-1.12	-.006	-1.95	-.54
			-1.36	-3.77			
4	.05	-.252	-28.62	-4.24	-.024	-9.19	-2.30
			-9.27	-25.94			
5	.05	-.112	-14.44	-2.48	-.015	-6.22	-1.46
			-6.06	-16.67			
6	.5	-.200	-34.75	-8.19	-.056	-23.68	-5.65
			-21.03	-54.24			
7	.6	-.222	-31.39	-5.77	-.084	-22.23	-4.50
			-22.32	-59.98			
8	.6	-.078	-13.58	-3.07	-.044	-11.53	-2.68
			-11.30	-29.26			
9	.71	-.156	-20.75	-2.91	-.170	-19.22	-2.81
			-14.10	-38.42			
10	.71	-.064	-10.30	-1.56	-.088	-10.92	-1.56
			-6.06	-14.58			

Table 5.3: Characteristic Displacements and Stresses - Load at Intrados

(deflection is in *mm*, pressure and stresses are in *MPa*)

Case	Pres.	Cylid. Shell			Tor. Shell		
	p	w	σ_x	σ_θ	w	σ_η	σ_θ
1	.01	-.27	-28.02	-5.48	-3.5	3.16	-3.97
			3.79	11.98		-12.43	-17.16
2	.01	-.17	-15.88	-3.22	-1.2	.64	-2.63
			-.005	-.04		-7.16	-9.81
3	.01	-.11	-10.29	-2.23	-.54	.19	-1.87
			-.84	-2.49		-4.87	-6.65
4	.1	-.27	-33.65	-9.13	-.95	.90	-7.63
			-8.98	-24.47		-19.34	-25.85
5	.1	-.12	-17.58	-5.54	-.38	1.29	-4.62
			-6.54	-17.13		-11.79	-15.67
6	1.0	-.23	-44.06	-20.12	-.61	10.53	-17.74
			-27.65	-63.27		-49.06	-71.98
7	1.0	-.24	-38.49	-11.67	-.67	6.49	-11.33
			-27.66	-69.58		-47.53	-95.37
8	1.0	-.09	-16.77	-6.56	-.23	-3.11	-6.87
			-14.94	-34.95		-26.84	-53.87
9	1.0	-.17	-24.41	-4.75	-.54	-15.11	-5.42
			-17.32	-45.00		-34.09	-82.71
10	1.0	-.07	-11.85	-2.55	-.17	-12.27	-2.96
			-7.73	-17.67		-14.73	-32.87

Chapter 6

Conclusions

From the present study, the following conclusions can be drawn.

- A solution scheme based on the Sanders shell theory has been developed for the linear elastostatic behavior of a toroidal shell subjected to surface loading.
- The results based on the Sanders shell theory for sample problems agree very closely with results from the FEM, MVD and Flügge shell theories. The accuracy of FEM (ADINA package) for toroidal shell was verified by experimentation in ref. [58]. Indications are that the results of the Sanders theory are superior to those of the MVD theory, and are thus suitable for making ovalization and other predictions for the ram bending process.

In the present study, Sanders shell theory solutions are limited to a sectorial toroidal shell of range $0^\circ \leq \phi \leq 90^\circ$ with simply supported conditions. In future studies it is suggested that additional end boundary conditions be considered.

- In general when shell action predominates toroidal shells are structurally superior in resisting local loads, compared to cylindrical shells, if the loads are at the extrados and inferior if the loads are at the intrados.
- As a guide to computer time requirements, the CPU time of the Sanders theory is compared with the CPU time of the FEM. The results show that the time for the Sanders theory (40.8 sec.) is significantly less than the time for the FEM (225.8 sec.).

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Appendix A

Coefficients for the Equilibrium Equations of The Sanders Theory

$$\nu_1 = \zeta^4(12r^2 + t^2)/12$$

$$\nu_2 = r^2\zeta^2(1 - \nu)/2 + t^2(4\zeta + 4\zeta^2 + 1)(1 - \nu)/96$$

$$\nu_3 = -r^2\zeta^3\gamma\sin\theta - t^2\zeta^3\gamma\sin\theta/12$$

$$\nu_4 = -\zeta^2\gamma^2\sin^2\theta(12r^2 + t^2)/12 - \nu\zeta^3\gamma\cos\theta(12r^2 + t^2)/12$$

$$\nu_5 = r^2\zeta^3(1 + \nu)/2 + t^2\zeta(2\zeta + 1)(\gamma\cos\theta - \frac{1}{2})(1 - \nu)/48 \\ + t^2\zeta^2\nu\gamma\cos\theta/12$$

$$\nu_6 = [r^2\zeta^2(3 - \nu)/2 - t^2\zeta^2\nu/12 + t^2(1 - \nu)(1 - 4\zeta^2)/96]\gamma\sin\theta \\ + t^2(4\zeta + 1 - \nu)\gamma^2\sin 2\theta/48$$

$$\nu_7 = -t^2\zeta^4/12$$

$$\nu_8 = t^2\zeta(\nu - 2\zeta - 1)/24$$

$$\nu_9 = t^2\zeta^3\gamma\sin\theta/12$$

$$\nu_{10} = (\nu - 1 - 4\zeta)t^2\gamma\sin\theta/24$$

$$\nu_{11} = (r^2\zeta^4 + t^2\zeta^2\gamma^2\sin^2\theta/12) + \zeta^3\nu\gamma\cos\theta(r^2 + t^2/12)$$

$$\nu_{12} = r^2\zeta^2\gamma^2\sin\theta\cos\theta - r^2\zeta^3\gamma\sin\theta$$

$$\begin{aligned} \nu_{13} = & (-\nu\zeta/4 + \zeta^2/2 + \nu\zeta^2/2 + \zeta/4)\gamma\cos\theta + 12\nu r^2\zeta^3/t^2 \\ & + 6r^2\zeta^3(1 - \nu)/t^2 + (\nu - 1)(2\zeta^2 + \zeta)/8 \end{aligned}$$

$$\nu_{14} = 6(\nu - 3)\gamma\sin\theta r^2\zeta^2/t^2 + \zeta(\nu - 3)\gamma^2\sin 2\theta/4 - (3 + 4\zeta)(1 - \nu)\gamma\sin\theta/8$$

$$\nu_{15} = 6(1 - \nu)r^2\zeta^4/t^2 + (\nu - 1)\zeta^2\gamma\cos\theta/2 + (1 - \nu)\zeta^2\gamma^2\cos^2\theta/2 + (1 - \nu)\zeta^2/8$$

$$\nu_{16} = 12r^2\zeta^2/t^2 + \gamma^2\cos^2\theta$$

$$\begin{aligned} \nu_{17} = & 6(\nu - 1)r^2\zeta^3\gamma\sin\theta/t^2 + (1 - \nu)\zeta\gamma^3\sin 2\theta\cos\theta/2 + 3(\nu - 1)\zeta\gamma^2\sin 2\theta/8 \\ & + (\nu - 1)\zeta(\gamma\cos\theta - \frac{1}{2})(2\gamma^2\sin 2\theta + \gamma\sin\theta)/4 + (1 - \nu)\zeta\gamma^3\sin\theta\cos^2\theta/2 \\ & + (1 - \nu)\zeta^2\gamma\sin\theta/2 + (\nu - 1)\zeta^2\gamma^2\sin 2\theta/2 \end{aligned}$$

$$\begin{aligned} \nu_{18} = & 6(1 - \nu)r^2\zeta^3\gamma\cos\theta/t^2 - 6(1 - \nu)r^2\zeta^2\gamma^2\sin^2\theta/t^2 + (1 - \nu)\zeta\gamma^3\cos 2\theta\cos\theta \\ & + (1 - \nu)\zeta\gamma^2\cos^2\theta/4 + (\nu - 1)\gamma^3\sin 2\theta\sin\theta/2 + (\nu - 1)\zeta^2\gamma^2\cos^2\theta/2 \\ & + (\nu - 1)\zeta\gamma^2\cos 2\theta/2 + (1 - \nu)\zeta(2\zeta - 1)\gamma\cos\theta/8 + (1 - \nu)\gamma^4\sin^2 2\theta/2 \\ & + (\nu - 1)\gamma^2\sin 2\theta(2\gamma^2\sin 2\theta + \gamma\sin\theta)/4 \\ & + (\nu - 1)(2\gamma^2\sin 2\theta + \gamma\sin\theta)(1 - 2\zeta)\gamma\sin\theta/8 \\ & + \gamma^3\sin^2\theta\cos\theta(1 - \nu)(1 - \zeta)/2 + (1 - \nu)(\zeta - 1)\gamma^2\sin^2\theta/4 \end{aligned}$$

$$\nu_{19} = -\gamma \cos \theta$$

$$\nu_{20} = -\zeta^2 \gamma \cos \theta + (1 - \nu) \zeta^2 / 2$$

$$\nu_{21} = \zeta \gamma^2 \sin 2\theta / 2 + 3(1 - \nu) \zeta \gamma \sin \theta / 2$$

$$\begin{aligned} \nu_{22} = & 12r^2 \zeta^2 \gamma \cos \theta / t^2 + 12\nu \zeta^3 r^2 / t^2 + 2(\nu - 1) \gamma^3 \sin^2 \theta \cos \theta \\ & + (\nu - 1) \zeta \gamma^2 \cos^2 \theta + (1 - \nu) \gamma \sin \theta (2\gamma^2 \sin 2\theta + \gamma \sin \theta) / 2 \\ & + (1 - \nu) \gamma^2 \sin^2 \theta + (1 - \nu) \zeta \gamma \cos \theta / 2 \end{aligned}$$

$$\nu_{23} = -\zeta^4$$

$$\nu_{24} = -\zeta(1 - \nu) / 2 - \zeta^2$$

$$\nu_{25} = [\zeta - (1 - \nu) / 2] \gamma \sin \theta$$

$$\nu_{26} = 2\gamma \sin \theta \zeta^3$$

$$\nu_{27} = 12r^2 \zeta^4 / t^2 + (12\nu r^2 / t^2 + 1 + \nu) \gamma \cos \theta \zeta^3 + \gamma^2 \sin^2 \theta \zeta^2$$

$$\nu_{28} = (1 - 6r^2 / t^2) \gamma^2 \sin 2\theta \zeta^2 - (12r^2 / t^2 + 1) \nu \gamma \sin \theta \zeta^3 + \gamma^3 \sin^3 \theta \zeta$$

$$\nu_{29} = -\gamma \cos \theta$$

$$\nu_{30} = [-\gamma \cos \theta + (1 - \nu) / 2] \zeta^2$$

$$\nu_{31} = -3\zeta \gamma^2 \sin 2\theta / 2 + [2\zeta^2 - \zeta(1 - \nu) / 2] \gamma \sin \theta$$

$$\begin{aligned} \nu_{32} = & [(12r^2 / t^2 + 1) \zeta^2 - \zeta(1 - \nu) / 2] \gamma \cos \theta - 2\zeta(1 - \nu) \gamma^2 \cos 2\theta \\ & - \zeta(1 + \nu) \gamma^2 \cos^2 \theta + [\zeta(1 + 2\nu) - (1 - \nu) / 2] \gamma^2 \sin^2 \theta \\ & - 2(1 - 2\nu) \gamma^3 \sin^2 \theta \cos \theta - (1 + 2\nu) \gamma^3 \sin \theta \sin 2\theta + 12\nu r^2 \zeta^3 / t^2 \end{aligned}$$

$$\nu_{33} = \zeta^4$$

$$\nu_{34} = 1$$

$$\nu_{35} = 2\zeta^2$$

$$\nu_{36} = -2\gamma\sin\theta\zeta^3$$

$$\nu_{37} = 2\gamma\sin\theta\zeta$$

$$\nu_{38} = 4\gamma^2\sin^2\theta + (3 - \nu)\gamma\cos\theta\zeta$$

$$\nu_{39} = -\gamma^2\sin^2\theta\zeta^2 - (1 + \nu)\gamma\cos\theta\zeta^3$$

$$\nu_{40} = -\gamma^3\sin^3\theta\zeta - \gamma^2\sin 2\theta\zeta^2 + \nu\gamma\sin\theta\zeta^3$$

$$\nu_{41} = 12r^2(\zeta^4 + \gamma^2\cos^2\theta\zeta^2 + 2\nu\gamma\cos\theta\zeta^3)/t^2$$

Appendix B

Computer Program TORSAN

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C   LAST MODIFICATION - JUNE, 1991                                TOR00010
C   NEED FILES 'TORSAN EXEC' 'TORSAN DATA' 'TORSAN FORTRAN'    TOR00020
C   RESULTS IN 'TORSAN RESULT'                                    TOR00030
C   PAD LOADING ON TOROIDAL SHELL - ELASTIC SHELL THEORY SOLUTION TOR00040
C   SANDERS THEORY IN DISPLACEMENT FORM IS USED                TOR00050
C   BI-SYMMETRIC RADIAL UNIFORM PAD LOADING IS ASSUMED         TOR00060
C   READIN AND WRITEOUT ALL ANGLES IN DEGREES                  TOR00070
C   D. REDEKOP U. OF OTTAWA MARCH 1989                         TOR00080
C   F. ZHANG U. OF OTTAWA JUNE 1991                            TOR00090
C   NOTE - COORD 'FI' VALUES ARE IO WHILE 'ETA' VALUES ARE CALCULATED TOR00100
C   CALL START                                                  TOR00110
C   CALL SOLVE                                                  TOR00120
C   CALL RESULT                                                TOR00130
C   STOP                                                        TOR00140
C   END                                                         TOR00150
C-----
C   SUBROUTINE START                                           TOR00160
C   INPUT DATA FIX GLOBAL CONSTANTS AND INITIALIZE             TOR00170
C   IMPLICIT DOUBLE PRECISION (A-H,O-Z)                         TOR00180
C   DOUBLE PRECISION M,M2,M3,M4                                 TOR00190
C   CHARACTER*20 NAME*80                                        TOR00200
C   COMMON/AAA/R,RR,PSI,T,E,POI,MHI,NHI,NFI,NT,NLOAD           TOR00210
C   COMMON/BBB/RES(40,20,10),GFI(40),GETA(40),GT(20),DATLOD(10,7) TOR00220
C   COMMON/CCC/PI5,PI2,PI,SOP2,RAD,OMP,OMP2,TP,AK,STIF,ROS,R2,R3,CZ, TOR00230
C   1 ZD,PSIR                                                  TOR00240
C   COMMON/FFF/P,ZK,ZC,ZX,ZY,G,G2,G3,G4,G5,M,M2,M3,M4,F,Q,A,S, TOR00250
C   1 UMP,DMP,TMP,UPP,DPP,TPP,BB(9),STC(20),STS(20),STZ(20), TOR00260
C   1 B2,B1,VMP,SMP,EKD,EKB,EKC,EKE,EL,EL1,H,EKA              TOR00270
C   PI=3.141592653589793D0                                     TOR00280
C   RAD=PI/180.D0                                             TOR00290
C   READ(5,16) NAME                                           TOR00300
C   16 FORMAT(A80)                                             TOR00310
C   MHI,NHI INDICATES MAX TERMS IN SERIES                     TOR00320
C   NFI,NT INDICATES SIZE OF GRID NLOAD=NUMBER OF LOADS      TOR00330
C   READ(5,*) MHI,NHI,NFI,NT,NLOAD                            TOR00340
C   ENTER DISTANCES IN MM ANGLES IN DEGREES                  TOR00350
C   R=X-SECT RADIUS RR=TOROIDAL RADIUS PSI=TOTAL ANGULAR LENGTH TOR00360
C   T=SHELL THICK E=YOUNG'S MODULUS POI=POISSON RATIO        TOR00370
C   READ(5,*) R,RR,PSI,T,E,POI                                TOR00380
C   ENTER COORDS OF RESULT GRID POINTS (DEGREES)             TOR00390
C   READ(5,*) (GFI(I),I=1,NFI)                                TOR00400
C   READ(5,*) (GT(I),I=1,NT)                                  TOR00410
C   DO 10 I=1,NLOAD                                           TOR00420
C   10 READ(5,*) (DATLOD(I,J),J=1,5)                            TOR00430
C   LOAD CARD=MAGN-ALPHA-DELTA-BETA-ZETA+PI*ALPHA/PSIR+PI*DELTA/PSIR TOR00440
C   WRITE(8,116) MHI,NHI                                       TOR00450
C   116 FORMAT(/1X,'MHI=',I4/1X,'NHI=',I4)                   TOR00460
C   WRITE(8,117)                                               TOR00470
C   117 FORMAT(/' SHELL THEORY IS THAT OF SANDERS ')          TOR00480
C   WRITE(8,126) R,RR,PSI,T,E,POI                              TOR00490
C   126 FORMAT(/' TOROIDAL SHELL'/                              TOR00500
C   1 1X,' X-SECT. RAD. =',E12.5,2X,'MM'/                     TOR00510
C   2 1X,' TOROIDAL RAD. =',E12.5,2X,'MM'/                     TOR00520
C   3 1X,' ANGULAR LENGTH =',E12.5,2X,'DEG'/                  TOR00530
C   4 1X,' THICKNESS =',E12.5,2X,'MM'/                        TOR00540
C   TOR00550

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4 1X,' YOUNG MODULUS  =',E12.5,2X,'MPA'/
5 1X,' POISSON RATIO  =',E12.5 /)
WRITE(8,136)
136 FORMAT(' LOAD DATA'/
1 ' LOAD PRESS.(MPA)',3X,'ALPHA(DEG)',3X,'DELTA(DEG)',
2 4X,'BETA(DEG)', 4X,'ZETA(DEG)')
PSIR=PSI*RAD
DO 110 I=1,NLOAD
WRITE(8,146)I,(DATLOD(I,J),J=1,5)
146 FORMAT(1X,I4,5E13.5)
DO 111 J=2,5
111 DATLOD(I,J)=DATLOD(I,J)*RAD
DATLOD(I,6)=PI*DATLOD(I,2)/PSIR
110 DATLOD(I,7)=PI*DATLOD(I,3)/PSIR
C GLOBAL CONSTANTS
PI2=PI*PI
PI5=.5D0*PI
SOP2=16.D0/PI2
OMP=1.D0-POI
OMP2=1.D0-POI*POI
TP=2.D0+POI
STIF=E*T/OMP2
ROS=R*R/STIF
R2=R*R
R3=R2*R
R4=R3*R
B2=1-R
B1=1+R
CZ=6.D0/(T*T)
P=POI
ZK=(T/R)*(T/R)/12.D0
ZC=E*T/(1.D0-POI*POI)
AK=ZC
ZX=E*(T*T*T)/(12.D0*OMP2)
ZD=ZX
F=.5D0
ZY=-F*R4/ZD
G=R/RR
G2=G*G
G3=G2*G
G4=G2*G2
G5=G4*G
H=T
H2=H*H
EL=H2/12.D0
EL1=12.D0/H2
EKA=R2/H2
Q=.25D0
A=.125D0
S=.0625D0
UMP=1.D0-P
DMP=2.D0-P
TMP=3.D0-P
VMP=4.D0-P
SMP=5.D0-P
TOR00560
TOR00570
TOR00580
TOR00590
TOR00600
TOR00610
TOR00620
TOR00630
TOR00640
TOR00650
TOR00660
TOR00670
TOR00680
TOR00690
TOR00700
TOR00710
TOR00720
TOR00730
TOR00740
TOR00750
TOR00760
TOR00770
TOR00780
TOR00790
TOR00800
TOR00810
TOR00820
TOR00830
TOR00840
TOR00850
TOR00860
TOR00870
TOR00880
TOR00890
TOR00900
TOR00910
TOR00920
TOR00930
TOR00940
TOR00950
TOR00960
TOR00970
TOR00980
TOR00990
TOR01000
TOR01010
TOR01020
TOR01030
TOR01040
TOR01050
TOR01060
TOR01070
TOR01080
TOR01090
TOR01100

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UPP=1.D0+P
DPP=2.D0+P
TPP=3.D0+P
EKD=1.D0+3.D0*P
EKB=1.D0+2.D0*P
EKC=1.D0-3.D0*P
EKE=1.D0-2.D0*P
BB(1)=ZY*A*G4
BB(2)=ZY*G3
BB(3)=ZY*(3.D0*G2+F*G4)
BB(4)=ZY*(4.D0*G+3.D0*G3)
BB(5)=ZY*(1.D0+3.D0*G2+.375*G4)*2.D0
BB(6)=BB(4)
BB(7)=BB(3)
BB(8)=BB(2)
BB(9)=BB(1)
C INITIALIZE
DO 200 I=1,NFI
DO 210 J=1,NT
RES(I,J,1)=GFI(I)
RES(I,J,2)=GT(J)
C INITIALLY RES CONTAINS FI(DEG) THETA(DEG) U V W E1 E2 K1 K2 P
C RESULTS TO BE GIVEN IN SETS OF CIRCULAR ARCS AROUND X-SECTION
DO 220 K=3,10
220 RES(I,J,K)=.000
210 CONTINUE
200 CONTINUE
DO 225 J=1,NFI
225 GETA(J)=GFI(J)*RAD/G
DO 230 J=1,NT
230 GT(J)=GT(J)*RAD
C STORE TRIG FUNCTIONS OF THETA
DO 300 J=1,NT
STC(J)=COS(GT(J))
STS(J)=SIN(GT(J))
300 STZ(J)=1.+G*STC(J)
RETURN
END
C-----
SUBROUTINE SOLVE
C MAIN CONTROL ROUTINE TO FIND DISPLACEMENT COEFFTS AND ADD TO
C BASIC UNKNOWNS (U,V,W,E1,E2,K1,K2,P) AT EACH OF GRID POINTS
IMPLICIT DOUBLE PRECISION (A-H,O-Z)
DOUBLE PRECISION M,M2,M3,M4
COMMON/AAA/R,RR,PSI,T,E,POI,MHI,NHI,NFI,NT,NLOAD
COMMON/BBB/ RES(40,20,10),GFI(40),GETA(40),GT(20),DATLOD(10,7)
COMMON/CCC/PI5,PI2,PI,SOP2,RAD,OMP,OMP2,TP,AK,STIF,ROS,R2,R3,CZ,
1 ZD,PSIR
COMMON/EEE/AA(200,200),B(200),X(200),PX(200),
1 AN(200,200),BN(200,1),XN(200),WK(200)
COMMON/FFF/P,ZK,ZC,ZX,ZY,G,G2,G3,G4,G5,M,M2,M3,M4,F,Q,A,S,
1 UMP,DMP,TMP,UPP,DPP,TPP,BB(9),STC(20),STS(20),STZ(20),
1 B2,B1,VMP,SMP,EKD,EKB,EKC,EKE,EL,EL1,H,EKA
NHIDUP=NH1+3
WRITE(6,*) ' CALCULATIONS IN PROGRESS FOR M = '
TOR01110
TOR01120
TOR01130
TOR01140
TOR01150
TOR01160
TOR01170
TOR01180
TOR01190
TOR01200
TOR01210
TOR01220
TOR01230
TOR01240
TOR01250
TOR01260
TOR01270
TOR01280
TOR01290
TOR01300
TOR01310
TOR01320
TOR01330
TOR01340
TOR01350
TOR01360
TOR01370
TOR01380
TOR01390
TOR01400
TOR01410
TOR01420
TOR01430
TOR01440
TOR01450
TOR01460
TOR01470
TOR01480
TOR01490
TOR01500
TOR01510
TOR01520
TOR01530
TOR01540
TOR01550
TOR01560
TOR01570
TOR01580
TOR01590
TOR01600
TOR01610
TOR01620
TOR01630
TOR01640
TOR01650

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C   CALCULATE CONTRIBUTIONS TERM BY TERM
DO 1000 MS=1,MHI,2
EM=REAL(MS)
M=EM*PI*G/PSIR
M2=M*M
M3=M2*M
M4=M2*M2
WRITE(6,*) MS
C   SET UP EQUIL EQNS FOR THIS MS FOR N=0,1,...NHI
C   CALCULATE RHS COEFFTS PX(200),B(200)
CALL LOAD(MS,NHIDUP)
C   CALCULATE LHS COEFFTS AA(200,200)
CALL COEFFD(MS,NHIDUP)
C   SOLVE THE EQNS SET USING CUSTOMIZED BAND SOLVER X(200)
CALL BANSOL(NHIDUP)
C   ADD TO BASIC UNKNOWNNS CONTRIBUTIONS FROM N=0
N=0
U=.0
V=X(1)
W=X(2)
PR=PX(1)
CALL ADVAL(MS,N,U,V,W,PR)
C   ADD TO BASIC UNKNOWNNS CONTRIBUTIONS FROM N=1,2,...,NHI
DO 1100 N=1,NHI
IX=(N-1)*3+2
U=X(IX+1)
V=X(IX+2)
W=X(IX+3)
PR=PX(N+1)
CALL ADVAL(MS,N,U,V,W,PR)
1100 CONTINUE
1000 CONTINUE
RETURN
END

C-----
SUBROUTINE LOAD(MS,NHI)
C   DEFINE PX AND B VECTORS FOR THIS MS
C   PX CONTAINS COEFTS OF FOURIER SERIES FOR LOAD
C   B IS RIGHT HAND SIDE OF EQUILIBRIUM EQN SET
IMPLICIT DOUBLE PRECISION (A-H,O-Z)
DOUBLE PRECISION M,M2,M3,M4
COMMON/AAA/R,RR,PSI,T,E,POI,MHI,NHIDUM,NFI,NT,NLOAD
COMMON/BBB/RES(40,20,10),GFI(40),GETA(40),GT(20),DATLOD(10,7)
COMMON/CCC/PI5,PI2,PI,SOP2,RAD,OMP,OMP2,TP,AK,STIF,ROS,R2,R3,CZ,
1 ZD,PSIR
COMMON/EEE/AA(200,200),B(200),X(200),PX(200),
1 AN(200,200),BN(200,1),XN(200),WK(200)
COMMON/FFF/P,ZK,ZC,ZX,ZY,G,G2,G3,G4,G5,M,M2,M3,M4,F,Q,A,S,
1 UMP,DMP,TMP,UPP,DPP,TPP,BB(9),STC(20),STS(20),STZ(20),
1 B2,B1,VMP,SMP,EKD,EKB,EKC,EKE,EL,EL1,H,EKA
DIMENSION PROD(10)
EM=REAL(MS)
C   FIND LOAD COEFFICIENT FOR N=0 CASE
1000 PR=.0D0
DO 10 I=1,NLOAD
TOR01860
TOR01870
TOR01880
TOR01890
TOR01900
TOR01910
TOR01920
TOR01930
TOR01940
TOR01950
TOR01960
TOR01970
TOR01980
TOR01990
TOR02000
TOR02010
TOR02020
TOR02030
TOR02040
TOR02050
TOR02060
TOR02070
TOR02080
TOR02090
TOR02100
TOR02110
TOR02120
TOR02130
TOR02140
TOR02150
TOR02160
TOR02170
TOR02180
TOR02190
TOR02200

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      BLM=DATLOD(I,6)*EM
      CLM=DATLOD(I,7)*EM
      PROD(I)=SIN(EM*PI5)*COS(BLM)*SIN(CLM)
10    PR=PR+(SOP2*DATLOD(I,1)*DATLOD(I,5)/EM)*FROD(I)
      PX(1)=PR
C     FIND LOAD COEFFICIENT FOR N GT 0 CASE
2000  DO 2100 N=1,NHI
      EN=REAL(N)
      N1=N+1
      PR=.000
      DO 2010 I=1,NLOAD
      BET=EN*DATLOD(I,4)
      ZET=EN*DATLOD(I,5)
2010  PR=PR+(2.*SOP2*DATLOD(I,1)/(EM*EN))*COS(BET)*SIN(ZET)*PROD(I)
2100  PX(N1)=PR
C     NOW FORM 'B' VECTOR
3000  NEQ=3*NHI+2
      DO 3010 I=1,NEQ
3010  B(I)=.0
      NHIP1=NHI+1
      DO 3100 I=1,NHIP1
      NN=I-1
      NNBLO=NN-5
      ICBLO=2+NNBLO*3
      PR=PX(I)
C     EACH PR MAKES SEVEN CONTRIBUTIONS TO 'B' VECTOR
      DO 3200 J=1,9
      ICB=ICBLO+J*3
      IF(ICB.GT.1) GO TO 3210
      IF(ICB.EQ.-10) ICB=ICB+24
      IF(ICB.EQ. -7) ICB=ICB+18
      IF(ICB.EQ. -4) ICB=ICB+12
      IF(ICB.EQ. -1) ICB=ICB+ 6
3210  IF(ICB.GT.NEQ) GO TO 3200
      B(ICB)=B(ICB)+BB(J)*PR
3200  CONTINUE
3100  CONTINUE
      RETURN
      END
C-----
SUBROUTINE COEFFD(MS,NHI)
C     DEFINE LHS OF EQUIL EQNS FOR THIS MS
C     G=GAMMA P=POISSON ZK=(T/R)*(T/R)/12
C     IHI=NUMBER OF SETS OF 3(OR 2) EQNS TO BE WRITTEN
C     NEQ=TOTAL NUMBER OF EQNS NEQ=NHI*3+2
C     IQ=NUMBER OF EQUATION BEING WRITTEN IN MATRIX AA
C     NN=ORDER OF EQN SET BEING WRITTEN
C     N=ORDER OF TERM WHOSE COEFT IS TO BE INSERTED IN THE EQN
C     IC=COLUMN OF FIRST NON-ZERO ENTRY
C     IMPLICIT DOUBLE PRECISION (A-H,O-Z)
      DOUBLE PRECISION M,M2,M3,M4
      COMMON/AAA/R,RR,PSI,T,E,POI,MHI,NHIDUP,NFI,NT,NLOAD
      COMMON/BBB/RES(40,20,10),GFI(40),GETA(40),GT(20),DATLOD(10,7)
      COMMON/CCC/PI5,PI2,PI,SOP2,RAD,OMP,OMP2,TP,AK,STIF,ROS,R2,R3,CZ,
1      ZD,PSIR
      TOR02210
      TOR02220
      TOR02230
      TOR02240
      TOR02250
      TOR02260
      TOR02270
      TOR02280
      TOR02290
      TOR02300
      TOR02310
      TOR02320
      TOR02330
      TOR02340
      TOR02350
      TOR02360
      TOR02370
      TOR02380
      TOR02390
      TOR02400
      TOR02410
      TOR02420
      TOR02430
      TOR02440
      TOR02450
      TOR02460
      TOR02470
      TOR02480
      TOR02490
      TOR02500
      TOR02510
      TOR02520
      TOR02530
      TOR02540
      TOR02550
      TOR02560
      TOR02570
      TOR02580
      TOR02590
      TOR02600
      TOR02610
      TOR02620
      TOR02630
      TOR02640
      TOR02650
      TOR02660
      TOR02670
      TOR02680
      TOR02690
      TOR02700
      TOR02710
      TOR02720
      TOR02730
      TOR02740
      TOR02750

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COMMON/FFF/P,ZK,ZC,ZX,ZY,G,G2,G3,G4,G5,M,M2,M3,M4,F,Q,A,S, TOR02760
1 UMP,DMP,TMP,UPP,DPP,TPP,BB(9),STC(20),STS(20),STZ(20), TOR02770
1 B2,B1,VMP,SMP,EKD,EKB,EKC,EKE,EL,EL1,H,EKA TOR02780
COMMON/EEE/AA(200,200),B(200),X(200),PX(200), TOR02790
1 AN(200,200),BN(200,1),XN(200),WK(200) TOR02800
EM=REAL(MS) TOR02810
NEQ=3*NHI+2 TOR02820
DO 1 I=1,NEQ TOR02830
DO 2 J=1,27 TOR02840
2 AA(I,J)=.0 TOR02850
1 CONTINUE TOR02860
1000 IQ=0 TOR02870
IHI=NHI+1 TOR02880
C SET UP EQUATION SET FOR THIS MS DEFINE AA MATRIX AND BB VECTOR TOR02890
R4=R3*R TOR02900
H2=H*H TOR02910
EKA=R2/H2 TOR02920
DO 900 I=1,IHI TOR02930
NN=I-1 TOR02940
IF(NN.EQ.0) GO TO 210 TOR02950
C FIRST OF THREE EQUATIONS 'U' EGN MISSING IF NN=0 TOR02960
IQ=IQ+1 TOR02970
IC=1 TOR02980
110 N=NN-4 TOR02990
N2=N*N TOR03000
N3=N2*N TOR03010
AA(IQ,IC)= TOR03020
1 (-.0625*N2*(R2+.08333333333*H2) TOR03030
1 -.0625*N*(R2+.08333333333*H2) TOR03040
1 -.0625*P*(R2+.08333333333*H2)+.0625*R2+.005208333333*H2)*G4 TOR03050
AA(IQ,IC+1)=.0 TOR03060
AA(IQ,IC+2)= TOR03070
1 (-.005208333335*N3*H2-.005208333335*N2*H2 TOR03080
1 -.5*N*(.125*R2+.125*P*(R2+.08333333333*H2)-.01041666667*H2))*G4 TOR03090
120 N=NN-3 TOR03100
N2=N*N TOR03110
N3=N2*N TOR03120
AA(IQ,IC+3)= TOR03130
1 (-.5*N2*(R2+.08333333333*H2) TOR03140
1 -.375*N*(R2+.08333333333*H2) TOR03150
1 -.375*P*(R2+.08333333333*H2)+.25*R2+.02083333333*H2)*G3 TOR03160
AA(IQ,IC+4)= TOR03170
1 (.5*N*M*(.125*R2*(1.+P)+.02083333334*H2*P+.01041666667*H2 TOR03180
1 *(1.-P))- .5*M TOR03190
1 *(.125*(3.-P)*R2-.02083333334*H2*P TOR03200
1 -.01041666667*H2*UMP+.04166666667*H2))*G3 TOR03210
AA(IQ,IC+5)= TOR03220
1 (-.04166666667*N3*H2-.03125*N2*H2 TOR03230
1 -.5*N*(.75*P*(R2+.08333333333*H2)+R2-.04166666668*H2) TOR03240
1 -.125*R2)*G3 TOR03250
130 N=NN-2 TOR03260
N2=N*N TOR03270
N3=N2*N TOR03280
AA(IQ,IC+6)= TOR03290
1 (-1.5*N2*(R2+.08333333333*H2) TOR03300

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1 -.75*N*(R2+.0833333333*H2) TOR03310
1 -.5*M2*(.25*R2*(1.-P)+.02083333334*H2*(1.-P))+.25*R2 TOR03320
1 +.02083333333*H2-.75*P*(R2+.0833333333*H2))*G2 TOR03330
1 +(-.25*N2*(R2+.0833333333*H2) TOR03340
1 -.125*N*(R2+.0833333333*H2) TOR03350
1 -.25*P*(R2+.0833333333*H2))*G4 TOR03360
AA(IQ,IC+7)= TOR03370
1 (.5*N*M*(.75*R2*(1.+P)+.08333333335*H2*P+.04166666667*H2 TOR03380
1 *(1.-P))-5*M TOR03390
1 *(.5*(3.-P)*R2-.08333333335*H2*P TOR03400
1 -.04166666668*H2*UMP+.02083333333*H2*(5.-P))*G2 TOR03410
AA(IQ,IC+8)= TOR03420
1 (-.125*N3*H2-.06250000002*N2*H2 TOR03430
1 -.5*N*(3.*R2-.04166666667*H2+1.5*P*(R2+.08333333333*H2)) TOR03440
1 +.04166666667*M2*H2-.5*R2-.02083333334*N*M2*H2)*G2 TOR03450
1 +(-.02083333334*N3*H2-.01041666667*N2*H2 TOR03460
1 -.5*N*(.5*R2+.5*P*(R2+.08333333333*H2))*G4 TOR03470
140 N=NN-1 TOR03480
N2=N*N TOR03490
N3=N2*N TOR03500
AA(IQ,IC+9)= TOR03510
1 (-.5*N*(R2+.08333333333*H2) TOR03520
1 -.5*M2*(R2*(1.-P)+.125*H2*(1.-P)) TOR03530
1 -2.*N2*(R2+.08333333333*H2)-.5*P*(R2+.08333333333*H2))*G TOR03540
1 +(-1.5*N2*(R2+.08333333333*H2)-.25*R2-.02083333333*H2 TOR03550
1 -.375*N*(R2+.08333333333*H2) TOR03560
1 -1.125*P*(R2+.08333333333*H2))*G3 TOR03570
AA(IQ,IC+10)= TOR03580
1 (.5*N*M*(1.5*R2*(1.+P)+.08333333333*H2*P+.01041666667*H2 TOR03590
1 *(1.-P))-5*M TOR03600
1 *(.5*(3.-P)*R2-.08333333334*H2*P TOR03610
1 -.03125000001*H2*UMP))*G TOR03620
1 +(.5*N*M*(.375*R2*(1.+P)+.0625*H2*P+.03125000001*H2 TOR03630
1 *(1.-P))-5*M TOR03640
1 *(.125*(3.-P)*R2-.02083333334*H2*P TOR03650
1 -.01041666667*H2*UMP+.04166666667*H2))*G3 TOR03660
AA(IQ,IC+11)= TOR03670
1 (-.04166666667*N2*H2-.16666666667*N3*H2-.02083333334*N*M2*H2*SMP TOR03680
1 -.02083333334*M2*H2*(P-5.)-.5*N*(P*(R2+.08333333333*H2)+4.*R2) TOR03690
1 -.5*R2)*G TOR03700
1 +(-.125*N3*H2-.125*R2-.03125*N2*H2 TOR03710
1 -.5*N*(3.*R2+.25*P*(R2+.08333333333*H2)+.04166666668*H2))*G3 TOR03720
150 N=NN TOR03730
N2=N*N TOR03740
N3=N2*N TOR03750
AA(IQ,IC+12)= TOR03760
1 -N2*(R2+.08333333333*H2) TOR03770
1 -M2*(.5*R2*(1.-P)+.09375000003*H2*(1.-P)) TOR03780
1 +(-.5*R2-.04166666667*H2-1.5*P*(R2+.08333333333*H2) TOR03790
1 -3.*N2*(R2+.08333333333*H2) TOR03800
1 -M2*(.25*R2*(1.-P)+.02083333334*H2*(1.-P))*G2 TOR03810
1 +(-.375*N2*(R2+.08333333333*H2)-.125*R2-.01041666667*H2 TOR03820
1 -.375*P*(R2+.08333333333*H2))*G4 TOR03830
AA(IQ,IC+13)= TOR03840
1 N*M*(.5*R2*(1.+P)-.03125000001*H2*(1.-P)) TOR03850

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1 +(N*M*(.75*R2*(1.+P)+.0833333335*H2*P+.0416666668*H2
1 *(1.-P))) *G2 TOR03860
AA(IQ,IC+14)= TOR03870
1 -N*R2 -.0833333333*N3*H2+.0416666667*N*M2*H2*(P-3.) TOR03880
1 +(-.25*N3*H2-.0416666667*N*M2*H2 TOR03890
1 -N*(1.5*P*(R2+.0833333333*H2)+.0416666667*H2+3.*R2)) *G2 TOR03900
1 +(-.03125*N3*H2 TOR03910
1 -N*(.375*R2+.375*P*(R2+.0833333333*H2)+.0104166667*H2)) *G4 TOR03920
160 N=NN+1 TOR03930
N2=N*N TOR03940
N3=N2*N TOR03950
AA(IQ,IC+15)= TOR03960
1 (.5*N*(R2+.0833333333*H2) TOR03970
1 -.5*M2*(R2*(1.-P)+.125*H2*(1.-P)) TOR03980
1 -2.*N2*(R2+.0833333333*H2)-.5*P*(R2+.0833333333*H2)) *G TOR03990
1 +(-1.5*N2*(R2+.0833333333*H2)-.25*R2-.0208333333*H2 TOR04000
1 +.375*N*(R2+.0833333333*H2) TOR04010
1 -1.125*P*(R2+.0833333333*H2)) *G3 TOR04020
AA(IQ,IC+16)= TOR04030
1 (.5*N*M*(1.5*R2*(1.+P)+.0833333333*H2*P+.0104166667*H2 TOR04040
1 *(1.-P))+.5*M TOR04050
1 *(.5*(3.-P)*R2-.0833333334*H2*P TOR04060
1 -.0312500001*H2*UMP)) *G TOR04070
1 +(.5*N*M*(.375*R2*(1.+P)+.0625*H2*P+.0312500001*H2 TOR04080
1 *(1.-P))+.5*M TOR04090
1 *(.125*(3.-P)*R2-.0208333334*H2*P TOR04100
1 -.0104166667*H2*UMP+.0416666667*H2)) *G3 TOR04110
AA(IQ,IC+17)= TOR04120
1 (.0416666667*N2*H2-.1666666667*N3*H2-.02083333334*N*M2*H2*SMP TOR04130
1 +.02083333334*M2*H2*(P-5)-.5*N*(P*(R2+.0833333333*H2)+4.*R2) TOR04140
1 +.5*R2) *G TOR04150
1 +(-.125*N3*H2+.125*R2+.03125*N2*H2 TOR04160
1 -.5*N*(3.*R2+2.25*P*(R2+.0833333333*H2)+.0416666668*H2)) *G3 TOR04170
170 N=NN+2 TOR04180
N2=N*N TOR04190
N3=N2*N TOR04200
AA(IQ,IC+18)= TOR04210
1 (-1.5*N2*(R2+.0833333333*H2) TOR04220
1 +.75*N*(R2+.0833333333*H2) TOR04230
1 -.5*M2*(.25*R2*(1.-P)+.0208333334*H2*(1.-P))+.25*R2 TOR04240
1 +.0208333333*H2-.75*P*(R2+.0833333333*H2)) *G2 TOR04250
1 +(-.25*N2*(R2+.0833333333*H2) TOR04260
1 +.125*N*(R2+.0833333333*H2) TOR04270
1 -.25*P*(R2+.0833333333*H2)) *G4 TOR04280
AA(IQ,IC+19)= TOR04290
1 (.5*N*M*(.75*R2*(1.+P)+.0833333335*H2*P+.0416666667*H2 TOR04300
1 *(1.-P))+.5*M TOR04310
1 *(.5*(3.-P)*R2-.0833333335*H2*P TOR04320
1 -.0416666668*H2*UMP+.0208333333*H2*(5.-P)) *G2 TOR04330
AA(IQ,IC+20)= TOR04340
1 (-.125*N3*H2+.0625000002*N2*H2 TOR04350
1 -.5*N*(3.*R2-.0416666667*H2+1.5*P*(R2+.0833333333*H2)) TOR04360
1 -.0416666667*M2*H2+.50*R2-.0208333334*N*M2*H2) *G2 TOR04370
1 +(-.0208333334*N3*H2+.0104166667*N2*H2 TOR04380
1 -.5*N*(.5*R2+.5*P*(R2+.0833333333*H2)) *G4 TOR04390
TOR04400

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180	N=NN+3	TOR04410
	N2=N*N	TOR04420
	N3=N2*N	TOR04430
	AA(IQ, IC+21)=	TOR04440
	1 (-.5*N2*(R2+.0833333333*H2)	TOR04450
	1 +.375*N*(R2+.0833333333*H2)	TOR04460
	1 -.375*P*(R2+.0833333333*H2)+.25*R2+.0208333333*H2)*G3	TOR04470
	AA(IQ, IC+22)=	TOR04480
	1 (.5*N*M*(.125*R2*(1.+P)+.02083333334*H2*P+.01041666667*H2	TOR04490
	1 *(1.-P))+.5*M	TOR04500
	1 *(.125*(3.-P)*R2-.02083333334*H2*P	TOR04510
	1 -.01041666667*H2*UMP+.04166666667*H2))*G3	TOR04520
	AA(IQ, IC+23)=	TOR04530
	1 (-.04166666667*N3*H2+.03125000000*N2*H2	TOR04540
	1 -.5*N*(.75*P*(R2+.0833333333*H2)+R2-.04166666668*H2)	TOR04550
	1 +.125*R2)*G3	TOR04560
190	N=NN+4	TOR04570
	N2=N*N	TOR04580
	N3=N2*N	TOR04590
	AA(IQ, IC+24)=	TOR04600
	1 (-.0625*N2*(R2+.0833333333*H2)	TOR04610
	1 +.0625*N*(R2+.0833333333*H2)	TOR04620
	1 -.0625*P*(R2+.0833333333*H2)+.0625*R2+.005208333333*H2)*G4	TOR04630
	AA(IQ, IC+25)=.0	TOR04640
	AA(IQ, IC+26)=	TOR04650
	1 (-.0052083333335*N3*H2+.0052083333335*N2*H2	TOR04660
	1 -.5*N*(.125*R2+.125*P*(R2+.0833333333*H2)-.01041666667*H2))*G4	TOR04670
C	SECOND EQUATION 'V' EQN	TOR04680
210	IQ=IQ+1	TOR04690
	IC=1	TOR04700
310	N=NN-4	TOR04710
	N2=N*N	TOR04720
	AA(IQ, IC)=.0	TOR04730
	AA(IQ, IC+1)=	TOR04740
	1 (-.5*N2*(.75*UMP*EKA+.0625-.0625*P)	TOR04750
	1 +.5*N*(-.75*EKA*UMP-.0625+.0625*P)+.0625-.0625*P+.75*UMP*EKA)*G4	TOR04760
	AA(IQ, IC+2)=.0	TOR04770
320	N=NN-3	TOR04780
	N2=N*N	TOR04790
	AA(IQ, IC+3)=	TOR04800
	1 (.5*N*M*(.125+.125*P+3.*P*EKA+1.5*UMP*EKA)	TOR04810
	1 -.5*M*(.25*(6.*P-18.)*EKA-.375+.125*P))*G3	TOR04820
	AA(IQ, IC+4)=	TOR04830
	1 (-.5*N2*(6.*UMP*EKA+.125-.125*P)+.5*N*(-4.5*UMP*EKA-.375+.375*P)	TOR04840
	1 +3.75*UMP*EKA+.09375-.09375*P)*G3	TOR04850
	AA(IQ, IC+5)=	TOR04860
	1 (.125*N2*M+.125*N*M+.5*M*(6.*EKA*F*UPP-.25+.25*P))*G3	TOR04870
330	N=NN-2	TOR04880
	N2=N*N	TOR04890
	AA(IQ, IC+6)=	TOR04900
	1 (.5*N*M*(.5+.5*P+18.*P*EKA+9.*UMP*EKA)	TOR04910
	1 -.5*M*(6.*P-18.)*EKA-1.+5*P))*G2	TOR04920
	AA(IQ, IC+7)=	TOR04930
	1 (-.5*N2*(18.*(1.-P)*EKA-.1875+.1875*P)+6.*(1.-P)*EKA	TOR04940
	1+.5*N*(-.1875+.1875*P-9.*UMP*EKA)-.25+.25*P-.5*M2*(6.*EKA+.5))*G2	TOR04950

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1 +(-.5*N2*(3.*UMP*EKA+.25-.25*P)+.125-.125*P
1 +.5*N*(-.125+.125*P-1.5*UMP*EKA)+1.5*UMP*EKA)*G4
AA(IQ,IC+8)=
1 (-.5*N2*M*(-.75-.25*P)+.5*N*M*(1.25-.75*P)
1 +.5*M*(6.*EKA*(2.+3.*P)-1.*P))*G2
340 N=NN-1
N2=N*N
AA(IQ,IC+9)=
1 (.5*N*M*(.125+.875*P+36.*P*EKA+18.*UMP*EKA)
1 -.5*M*((6.*P-18.)*EKA-.875+.875*P))*G
1 +(.5*N*M*(9.*P*EKA+.625*P+.375+.5*UMP*EKA)
1 -.5*M*(.25*(6.*P-18.)*EKA-.375+.125*P))*G3
AA(IQ,IC+10)=
1 (-.5*N2*(24.*(1.-P)*EKA-.25+.25*P)+3.*UMP*EKA
1 +.5*N*(.625-.625*P-6.*UMP*EKA)+.0625-.0625*P
1 -12.*M2*EKA)*G
1 +(-.5*N2*(18.*(1.-P)*EKA+.375-.375*P)-.25+.25*P
1 +.5*N*(-4.5*UMP*EKA-.375+.375*P)+5.25*UMP*EKA)*G3
AA(IQ,IC+11)=
1 (.5*M3+.50*N2*M*P+.5*N*M*(1.5-1.5*P)
1 +.5*M*(6.*EKA*(2.+6.*P)+.5-.5*P))*G
1 +(.375*N2*M+.125*N*M+.5*M*(6.*EKA*1.5*UPP-.75+.75*P))*G3
350 N=NN
N2=N*N
AA(IQ,IC+12)=
1 N*M*(12.*P*EKA-.375+.375*P+6.*UMP*EKA)
1 +N*M*(.5+.5*P+18.*P*EKA+9.*UMP*EKA)*G2
AA(IQ,IC+13)=
1 -N2*(6.*(1.-P)*EKA+.125-.125*P)-12.*M2*EKA
1 +(6.*(1.-P)*EKA-N2*(18.*(1.-P)*EKA-.1875+.1875*P)
1 +.125-.125*P-M2*(6.*R2/H2+.5))*G2
1 +(-N2*(2.25*(1.-P)*EKA+.1875-.1875*P)
1 +.125-.125*P+1.5*(1.-P)*EKA)*G4
AA(IQ,IC+14)=
1 -N2*M*(.5-.5*P)+12.*EKA*M*P
1 +(-N2*M*(-.75-.25*P)+M*(6.*EKA*(2.+3.*P)+.5-.5*P))*G2
360 N=NN+1
N2=N*N
AA(IQ,IC+15)=
1 (.5*N*M*(.875*P+.125+36.*P*EKA+18.*UMP*EKA)
1 +.5*M*((6.*P-18.)*EKA-.875+.875*P))*G
1 +(.5*N*M*(9.*P*R2/H2+.625*P+.375+.5*UMP*EKA)
1 +.5*M*(.25*(6.*P-18.)*R2/H2-.375+.125*P))*G3
AA(IQ,IC+16)=
1 (-.5*N2*(24.*(1.-P)*R2/H2-.25+.25*P)
1 -.5*N*(.625-.625*P-6.*UMP*EKA)+3.*(1.-P)*R2/H2
1 +.0625-.0625*P-12.*M2*R2/H2)*G
1 +(-.5*N2*(18.*(1.-P)*R2/H2+.375-.375*P)-.25+.25*P
1 -.5*N*(-4.5*UMP*EKA-.375+.375*P)+5.25*UMP*EKA)*G3
AA(IQ,IC+17)=
1 (.5*M3+.5*N2*M*P-.5*N*M*(1.5-1.5*P)
1 +.5*M*(6.*EKA*(2.+6.*P)+.5-.5*P))*G
1 +(.375*N2*M-.125*N*M+.5*M*(6.*EKA*1.5*UPP-.75+.75*P))*G3
370 N=NN+2
N2=N*N

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TOR05500

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      AA(IQ,IC+18)=
1 (.5*N*M*(.5+.5*P+18.*P*R2/H2+9.*UMP*EKA)
1 +.5*M*((6.*P-18.)*R2/H2+.5*P-1.))*G2
      AA(IQ,IC+19)=
1 (-.5*N2*(18.*(1.-P)*R2/H2-.1875+.1875*P)+6.*(1.-P)*R2/H2
1 -.5*N*(-.1875+.1875*P-9.*UMP*EKA)-.5*M2*(6.*EKA+.5)-.25+.25*P)*G2
1 +(-.5*N2*(3.*(1.-P)*R2/H2+.25-.25*P)+.125-.125*P
1 -.5*N*(-.125+.125*P-1.5*UMP*EKA)+1.5*(1.-P)*R2/H2)*G4
      AA(IQ,IC+20)=
1 (-.5*N2*M*(-.75-.25*P)-.5*N*M*(1.25-.75*P)
1 +.5*M*(6.*EKA*(2.+3.*P)-1.*P))*G2
380 N=NN+3
      N2=N*N
      AA(IQ,IC+21)=
1 (.5*N*M*(.125*P+.125+3.*P*R2/H2+1.5*UMP*EKA)
1 +.5*M*(.25*(6.*P-18.)*R2/H2-.375+.125*P))*G3
      AA(IQ,IC+22)=
1 (-.5*N2*(3.*(1.-P)*R2/H2+.125-.125*P)-.5*N*(-1.5*UMP*EKA
1 -.375+.375*P)+3.75*UMP*EKA+.09375-.09375*P)*G3
      AA(IQ,IC+23)=
1 (.125*N2*M-.125*N*M+.5*M*(6.*EKA*F*UPP-.25+.25*P))*G3
390 N=NN+4
      N2=N*N
      AA(IQ,IC+24)=.0
      AA(IQ,IC+25)=
1 (-.5*N2*(.75*(1.-P)*R2/H2+.0625-.0625*P)-.5*N*
1 (-.75*UMP*EKA-.0625+.0625*P)+.0625-.0625*P+.75*UMP*EKA)*G4
      AA(IQ,IC+26)=.0
C EQUATIONS 'W' EQUATION
410 IQ=IQ+1
      IC=1
510 N=NN-4
      N2=N*N
      N3=N2*N
      N4=N3*N
      AA(IQ,IC )=
1 (.0625*N3+.5*N*(1.5*EKA+1.5*P*EKA+.125*P)+.125*N2-.0625
1 +.75*EKA+.0625*(12.*EKA+1.)*P)*G4
      AA(IQ,IC+1)=.0
      AA(IQ,IC+2)=
1 (.0625*N4+.0625*N3*P+6.*EKA*.25*UPP+.125*N3
1 +.5*N*(-.125+.125*P))*G4
520 N=NN-3
      N2=N*N
      N3=N2*N
      N4=N3*N
      AA(IQ,IC+3)=
1 (.5*N3+.5*N*(12.*EKA+9.*P*EKA+.25+.75*P)+.75*N2-.375
1 +3.*EKA+.375*(12.*EKA+1.)*P)*G3
      AA(IQ,IC+4)=
1 (-.5*M*(3.*EKA -.25+.25*P+3.*P*EKA)-.125*N2*M+.125*N*M)*G3
      AA(IQ,IC+5)=
1 (.5*N4-.5*N2*(-.25-.75*P)+6.*EKA*1.5*UPP+.75*N3
1 +.5*N*(-.75+.75*P))*G3
530 N=NN-2

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TOR05990
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TOR06010
TOR06020
TOR06030
TOR06040
TOR06050

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N2=N*N	TOR06060
N3=N2*N	TOR06070
N4=N3*N	TOR06080
AA(IQ, IC+6)=	TOR06090
1 (.5*N*(36.*EKA+18.*P+EKA+1.+1.5*P)+1.5*N3+.25*M2-.5	TOR06100
1 +3.*EKA+1.5*N2+.75*(12.*EKA+1.)*P+.25*N*M2)*G2	TOR06110
1 +(.25*N3+.25*N2+.5*N*(6.*EKA+6.*P+EKA+.5+.5*P)-.375	TOR06120
1 +1.5*EKA+.125*(12.*EKA+1.)*P)*G4	TOR06130
AA(IQ, IC+7)=	TOR06140
1 (-.5*M*(12.*EKA-2.+5*P+18.*P+EKA)+.5*N2*M*(-.75-.25*P)	TOR06150
1 -.5*N*M*(.25+.25*P))*G2	TOR06160
AA(IQ, IC+8)=	TOR06170
1 (1.5*N4-.5*N2*(-1.-1.5*P)-.5*M2*(-.5-.5*P)	TOR06180
1 +6.*EKA*(3.5+3.*P)+.5*M2*N2+1.5*N3+.5*N*(-1.+1.5*P)	TOR06190
1 -.5*N*M2)*G2	TOR06200
1 +(.25*N4-.5*N2*(-.5-.5*P)+.25*N3+6.*EKA*UPP	TOR06210
1 +.5*N*(-.75+.25*P))*G4	TOR06220
540 N=NN-1	TOR06230
N2=N*N	TOR06240
N3=N2*N	TOR06250
N4=N3*N	TOR06260
AA(IQ, IC+9)=	TOR06270
1 (N2+.5*N*(48.*EKA+12.*P+EKA+1.+P)	TOR06280
1 -.5*N*M2*(-2.5+.5*P)+2.*N3+.5*(12.*EKA+1.)*P	TOR06290
1 +.25*M2*(1.+P))*G	TOR06300
1 +(1.5*N3+.75*N2+.5*N*(36.*EKA+27.*P+EKA+2.75+2.25*P)-.875	TOR06310
1 +3.*EKA+.375*(12.*EKA+1.)*P)*G3	TOR06320
AA(IQ, IC+10)=	TOR06330
1 (-.5*M3-.5*M*(12.*EKA+.5+.5*P+36.*P+EKA)-.5*N2*M*P	TOR06340
1 -.5*N*M*(1.5+.5*P))*G	TOR06350
1 +(-.5*M*(9.*EKA-1.75+.75*P+9.*P+EKA)-.375*N2*M+.125*N*M)*G3	TOR06360
AA(IQ, IC+11)=	TOR06370
1 (2.*N4-.5*N2*(-1.-P)-.5*M2*(3.-P)+6.*EKA*2.*DPP	TOR06380
1 +2.*M2*N2+N3+.5*N*P-N*M2)*G	TOR06390
1 +(1.5*N4-.5*N2*(-2.75-2.25*P)+.75*N3+6.*EKA*4.5*UPP	TOR06400
1 +.5*N*(-1.75+.75*P))*G3	TOR06410
550 N=NN	TOR06420
N2=N*N	TOR06430
N3=N2*N	TOR06440
N4=N3*N	TOR06450
AA(IQ, IC+12)=	TOR06460
1 12.*N*EKA+N3-N*M2*(-1.5+.5*P)	TOR06470
1 +(3.*N3+.5*N*M2+N*(2.+18.*P+EKA+1.5*P+36.*EKA))*G2	TOR06480
1 +(.375*N3+N*(4.5*EKA+.5+4.5*P+EKA+.375*P))*G4	TOR06490
AA(IQ, IC+13)=	TOR06500
1 -12.*M*P*EKA+N2*M*(.5-.5*P)	TOR06510
1 +(-M*(12.*EKA+.5+P+18.*P+EKA)+N2*M*(-.75-.25*P))*G2	TOR06520
AA(IQ, IC+14)=	TOR06530
1 2.*M2*N2+M4+N4+12.*EKA	TOR06540
1 +(M2*N2-N2*(-2.-1.5*P)+3.*N4+12.*EKA*(3.5+3.*P)	TOR06550
1 -M2*(3.5-.5*P))*G2	TOR06560
1 +(.375*N4+12.*EKA*.75*UPP-N2*(-.5-.375*P))*G4	TOR06570
560 N=NN+1	TOR06580
N2=N*N	TOR06590
N3=N2*N	TOR06600

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N4=N3*N
AA(IQ, IC+15)=
1 (-N2+.5*N*(48.*EKA+12.*P+EKA+1.+P)
1 -.5*N*M2*(-2.5+.5*P)+2.*N3-.5*(12.*EKA+1.)*P
1 -.25*M2*(1.+P))*G
1 +(1.5*N3-.75*N2+.5*N*(36.*EKA+27.*P+EKA+2.75+2.25*P)+.875
1 - 3.*EKA-.375*(12.*EKA+1.)*P)*G3
AA(IQ, IC+16)=
1 (-.5*M3-.5*M*(12.*EKA+.5+.5*P+36.*P+EKA)-.5*N2*M*P
1 +.5*N*M*(1.5+.5*P))*G
1 +(-.5*M*(9.*EKA-1.75+.75*P+9.*P+EKA)-.375*N2*M-.125*N*M)*G3
AA(IQ, IC+17)=
1 (2.*N4-.5*N2*(-1.-P)-.5*M2*(3.-P)+6.*EKA*2.*DPP
1 +2.*M2*N2-N3-.5*N*P+N*M2)*G
1 +(1.5*N4-.5*N2*(-2.75-2.25*P)-.75*N3+6.*EKA*4.5*UPP
1 -.5*N*(-1.75+.75*P))*G3
570 N=NN+2
N2=N*N
N3=N2*N
N4=N3*N
AA(IQ, IC+18)=
1 (.5*N*(36.*EKA+18.*P+EKA+1.+1.5*P)+1.5*N3-.25*M2+.5
1 - 3.*EKA-1.5*N2-.75*(12.*EKA+1.)*P+.25*N*M2)*G2
1 +(.25*N3-.25*N2+.5*N*(6.*EKA+6.*P+EKA+.5+.5*P)+.375
1 - 1.5*EKA-.125*(12.*EKA+1.)*P)*G4
AA(IQ, IC+19)=
1 (-.5*M*(12.*EKA-2+.5*P+18.*P+EKA)+.5*N2*M*(-.75-.25*P)
1 +.5*N*M*(.25+.25*P))*G2
AA(IQ, IC+20)=
1 (1.5*N4-.5*N2*(-1.-1.5*P)-.5*M2*(-.5-.5*P)
1 +6.*EKA*(3.5+3.*P)+.5*M2*N2-1.5*N3-.5*N*(-1.+1.5*P)
1 +.5*N*M2)*G2
1 +(.25*N4-.5*N2*(-.5-.5*P)-.25*N3+6.*EKA*UPP
1 -.5*N*(-.75+.25*P))*G4
580 N=NN+3
N2=N*N
N3=N2*N
N4=N3*N
AA(IQ, IC+21)=
1 (.5*N3+.5*N*(12.*EKA+9.*P+EKA+.25+.75*P)-.75*N2+.375
1 - 3.*EKA-.375*(12.*EKA+1.)*P)*G3
AA(IQ, IC+22)=
1 (-.5*M*(3.*EKA-.25+.25*P+3.*P+EKA)-.125*N2*M-.125*N*M)*G3
AA(IQ, IC+23)=
1 (.5*N4-.5*N2*(-.25-.75*P)+6.*EKA*1.5*UPP-.75*N3
1 -.5*N*(-.75+.75*P))*G3
590 N=NN+4
N2=N*N
N3=N2*N
N4=N3*N
AA(IQ, IC+24)=
1 (.0625*N3+.5*N*(1.5*EKA+1.5*P+EKA+.125*P)-.125*N2+.0625
1 -.75*EKA-.0625*(12.*EKA+1.)*P)*G4
AA(IQ, IC+25)=.0
AA(IQ, IC+26)=

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TOR07150

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1 (.0625*N4+.0625*N2*P+6.*EKA*.25*UPP-.125*N3
1 -.5*N*(-.125+.125*P))*G4 TOR07160
900 CONTINUE TOR07170
C FIX UP 'U' EQUATIONS TOR07180
2100 JHI=10 TOR07190
      JEQ=3 TOR07200
      DO 2110 I=1,4 TOR07210
      DO 2120 J=1,JHI TOR07220
2120 AA(JEQ,J)=.0 TOR07230
      JHI=JHI-3 TOR07240
2110 JEQ=JEQ+3 TOR07250
      AA(3,11)=AA(3,11) TOR07260
1 -((.5*.0*M*(1.5*R2*(1.+P)+.08333333333*H2*P+.01041666667*H2
1 *(1.-P))+.5*M TOR07270
1 *.5*(3.-P)*R2-.08333333333*H2*P TOR07280
1 -.03125000001*H2*UMP))*G TOR07290
1 +(.5*.0*M*(.375*R2*(1.+P)+.0625*H2*P+.03125000001*H2
1 *(1.-P))+.5*M TOR07300
1 *.125*(3.-P)*R2-.02083333334*H2*P TOR07310
1 -.01041666667*H2*UMP+.04166666667*H2))*G3) TOR07320
      AA(3,12)=AA(3,12) TOR07330
1 -((.04166666667*.0*H2-.1666666667*.0*H2-.02083333334*.0*M2*H2*SMP
1 +.02083333334*M2*H2*(P-.5)-.5*.0*(P*(R2+.08333333333*H2)+4.*R2)
1 +.5*R2)*G TOR07340
1 +(-.125*.0*H2+.125*R2+.03125*.0*H2 TOR07350
1 -.5*.0*(3.*R2+2.25*P*(R2+.08333333333*H2)+.04166666668*H2))*G3) TOR07360
      AA(3,13)=AA(3,13) TOR07370
1 -((-1.5*(R2+.08333333333*H2) TOR07380
1 +.75*(R2+.08333333333*H2) TOR07390
1 -.5*M2*(.25*R2*(1.-P)+.02083333334*H2*(1.-P))+.25*R2 TOR07400
1 +.02083333333*H2-.75*P*(R2+.08333333333*H2))*G2 TOR07410
1 +(-.25*(R2+.08333333333*H2) TOR07420
1 +.125*(R2+.08333333333*H2) TOR07430
1 -.25*P*(R2+.08333333333*H2))*G4) TOR07440
      AA(3,14)=AA(3,14) TOR07450
1 -((.5*M*(.75*R2*(1.+P)+.08333333335*H2*P+.04166666667*H2
1 *(1.-P))+.5*M TOR07460
1 *.5*(3.-P)*R2-.08333333335*H2*P TOR07470
1 -.04166666668*H2*UMP+.02083333333*H2*(5.-P))*G2) TOR07480
      AA(3,15)=AA(3,15) TOR07490
1 -((-125*H2+.06250000002*H2 TOR07500
1 -.5*(3.*R2-.04166666667*H2+1.5*P*(R2+.08333333333*H2)) TOR07510
1 -.04166666667*M2*H2+.50*R2-.02083333334*M2*H2))*G2 TOR07520
1 +(-.02083333334*H2+.01041666667*H2 TOR07530
1 -.5*(.5*R2+.5*P*(R2+.08333333333*H2))*G4) TOR07540
      AA(3,16)=AA(3,16) TOR07550
1 -((-5*4*(R2+.08333333333*H2) TOR07560
1 +.375*2*(R2+.08333333333*H2) TOR07570
1 -.375*P*(R2+.08333333333*H2)+.25*R2+.02083333333*H2))*G3) TOR07580
      AA(3,17)=AA(3,17) TOR07590
1 -((.5*2.*M*(.125*R2*(1.+P)+.02083333334*H2*P+.01041666667*H2
1 *(1.-P))+.5*M TOR07600
1 *.125*(3.-P)*R2-.02083333334*H2*P TOR07610
1 -.01041666667*H2*UMP+.04166666667*H2))*G3) TOR07620
      AA(3,18)=AA(3,18) TOR07630
1 -.01041666667*H2*UMP+.04166666667*H2))*G3) TOR07640
      AA(3,18)=AA(3,18) TOR07650
1 -.01041666667*H2*UMP+.04166666667*H2))*G3) TOR07660
      AA(3,18)=AA(3,18) TOR07670
1 -.01041666667*H2*UMP+.04166666667*H2))*G3) TOR07680
      AA(3,18)=AA(3,18) TOR07690
1 -.01041666667*H2*UMP+.04166666667*H2))*G3) TOR07700

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1 -((-04166666667*8.*H2+.03125000000*4.*H2
1 -.5*2.*(75*P*(R2+.08333333333*H2)+R2-.04166666668*H2)
1 +.125*R2)*G3) TOR07710
AA(3,19)=AA(3,19) TOR07720
1 -((-0625*9.*(R2+.08333333333*H2) TOR07730
1 +.0625*3.*(R2+.08333333333*H2) TOR07740
1 -.0625*P*(R2+.08333333333*H2)+.0625*R2+.005208333333*H2)*G4) TOR07750
AA(3,21)=AA(3,21) TOR07760
1 -((-005208333335*27.*H2+.005208333335*9.*H2 TOR07770
1 -.5*3*(.125*R2+.125*P*(R2+.08333333333*H2)-.01041666667*H2))*G4) TOR07780
AA(6,8)=AA(6,8) TOR07790
1 -(.5*0.*M*(.75*R2*(1.+P)+.08333333335*H2*P+.04166666667*H2 TOR07800
1 *(1.-P))+.5*M TOR07810
1 *(.5*(3.-P)*R2-.08333333335*H2*P TOR07820
1 -.04166666668*H2*UMP+.02083333333*H2*(5.-P))*G2) TOR07830
AA(6,9)=AA(6,9) TOR07840
1 -((-125*0.*H2+.06250000002*0.*H2 TOR07850
1 -.5*0*(3.*R2-.04166666667*H2+1.5*P*(R2+.08333333333*H2)) TOR07860
1 -.04166666667*H2*H2+.50*R2-.02083333334*0.*M2*H2)*G2 TOR07870
1 +(-.02083333334*0.*H2+.01041666667*0.*H2 TOR07880
1 -.5*0*(.5*R2+.5*P*(R2+.08333333333*H2))*G4) TOR07890
AA(6,10)=AA(6,10) TOR07900
1 -((-5*(R2+.08333333333*H2) TOR07910
1 +.375*(R2+.08333333333*H2) TOR07920
1 -.375*P*(R2+.08333333333*H2)+.25*R2+.02083333333*H2)*G3) TOR07930
AA(6,11)=AA(6,11) TOR07940
1 -(.5*M*(.125*R2*(1.+P)+.02083333334*H2*P+.01041666667*H2 TOR07950
1 *(1.-P))+.5*M TOR07960
1 *(.125*(3.-P)*R2-.02083333334*H2*P TOR07970
1 -.01041666667*H2*UMP+.04166666667*H2))*G3) TOR07980
AA(6,12)=AA(6,12) TOR07990
1 -((-04166666667*H2+.03125000000*H2 TOR08000
1 -.5*(.75*P*(R2+.08333333333*H2)+R2-.04166666668*H2) TOR08010
1 +.125*R2)*G3) TOR08020
AA(6,13)=AA(6,13) TOR08030
1 -((-0625*4.*(R2+.08333333333*H2) TOR08040
1 +.0625*2.*(R2+.08333333333*H2) TOR08050
1 -.0625*P*(R2+.08333333333*H2)+.0625*R2+.005208333333*H2)*G4) TOR08060
AA(6,15)=AA(6,15) TOR08070
1 -((-005208333335*8.*H2+.005208333335*4.*H2 TOR08080
1 -.5*2*(.125*R2+.125*P*(R2+.08333333333*H2)-.01041666667*H2))*G4) TOR08090
AA(9,5)=AA(9,5) TOR08100
1 -(.5*0.*M*(.125*R2*(1.+P)+.02083333334*H2*P+.01041666667*H2 TOR08110
1 *(1.-P))+.5*M TOR08120
1 *(.125*(3.-P)*R2-.02083333334*H2*P TOR08130
1 -.01041666667*H2*UMP+.04166666667*H2))*G3) TOR08140
AA(9,6)=AA(9,6) TOR08150
1 -((-04166666667*0.*H2+.03125*0.*H2 TOR08160
1 -.5*0*(.75*P*(R2+.08333333333*H2)+R2-.04166666668*H2) TOR08170
1 +.125*R2)*G3) TOR08180
AA(9,7)=AA(9,7) TOR08190
1 -((-0625*(R2+.08333333333*H2) TOR08200
1 +.0625*(R2+.08333333333*H2) TOR08210
1 -.0625*P*(R2+.08333333333*H2)+.0625*R2+.005208333333*H2)*G4) TOR08220
AA(9,9)=AA(9,9) TOR08230
TOR08240
TOR08250

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1 -((-0.005208333335*H2+.005208333335*H2 TOR08260
1 -.5*(.125*R2+.125*P*(R2+.08333333333*H2)-.01041666667*H2))*G4) TOR08270
AA(12,3)=AA(12,3) TOR08280
1 -((-0.005208333335*0.*H2+.005208333335*0.*H2 TOR08290
1 -.5*0.*(125*R2+.125*P*(R2+.08333333333*H2)-.01041666667*H2))*G4) TOR08300
C FIX UP 'V' EQUATIONS TOR08310
2200 JHI=13 TOR08320
JEQ=1 TOR08330
DO 2210 I=1,5 TOR08340
DO 2220 J=1,JHI TOR08350
2220 AA(JEQ,J)=.0 TOR08360
JHI=JHI-3 TOR08370
2210 JEQ=JEQ+3 TOR08380
AA(4,11)=AA(4,11)+ TOR08390
1 (-.5*0.*(24.*(1.-P)*R2/H2-.25+.25*P) TOR08400
1 -.5*0.*(625-.625*P-6.*UMP*EKA)+3.*(1.-P)*R2/H2 TOR08410
1 +.0625-.0625*P-12.*M2*R2/H2)*G TOR08420
1 +(-.5*0.*(18.*(1.-P)*R2/H2+.375-.375*P)-.25+.25*P TOR08430
1 -.5*0.*(-4.5*UMP*EKA-.375+.375*P)+5.25*UMP*EKA)*G3 TOR08440
AA(4,12)=AA(4,12)+ TOR08450
1 (.5*M3+.5*0.*M*P-.5*0.*M*(1.5-1.5*P) TOR08460
1 +.5*M*(6.*EKA*(2.+8.*P)+.5-.5*P))*G TOR08470
1 +(.375*0.*M-.125*0.*M+.5*M*(6.*EKA*1.5*UPP-.75+.75*P))*G3 TOR08480
AA(4,13)=AA(4,13)+ TOR08490
1 (.5*M*(.5+.5*P+18.*P*R2/H2+9.*UMP*EKA) TOR08500
1 +.5*M*(6.*P-18.)*R2/H2+.5*P-1.))*G2 TOR08510
AA(4,14)=AA(4,14)+ TOR08520
1 (-.5*(18.*(1.-P)*R2/H2-.1875+.1875*P)+8.*(1.-P)*R2/H2 TOR08530
1 -.5*(-.1875+.1875*P-9.*UMP*EKA)-.5*M2*(6.*EKA+.5)-.25+.25*P)*G2 TOR08540
1 +(-.5*(3.*(1.-P)*R2/H2+.25-.25*P)+.125-.125*P TOR08550
1 -.5*(-.125+.125*P-1.5*UMP*EKA)+1.5*(1.-P)*R2/H2)*G4 TOR08560
AA(4,15)=AA(4,15)+ TOR08570
1 (-.5*M*(-.75-.25*P)-.5*M*(1.25-.75*P) TOR08580
1 +.5*M*(6.*EKA*(2.+3.*P)-1.+P))*G2 TOR08590
AA(4,16)=AA(4,16)+ TOR08600
1 (.5*2.*M*(.125*P+.125+3.*P*R2/H2+1.5*UMP*EKA) TOR08610
1 +.5*M*(.25*(6.*P-18.)*R2/H2-.375+.125*P))*G3 TOR08620
AA(4,17)=AA(4,17)+ TOR08630
1 (-.5*4.*(6.*(1.-P)*R2/H2+.125-.125*P)-.5*2.*(-4.5*EKA*UMP TOR08640
1 -.375+.375*P)+3.75*UMP*EKA+.09375-.09375*P)*G3 TOR08650
AA(4,18)=AA(4,18)+ TOR08660
1 (.125*4.*M-.125*2.*M+.5*M*(6.*EKA*F*UPP-.25+.25*P))*G3 TOR08670
AA(4,20)=AA(4,20)+ TOR08680
1 (-.5*9.*(75*(1.-P)*R2/H2+.0625-.0625*P)-.5*3.* TOR08690
1 (-.75*UMP*EKA-.0625+.0625*P)+.75*UMP*EKA+.0625-.0625*P)*G4 TOR08700
AA(7,8)=AA(7,8)+ TOR08710
1 (-.5*0.*(18.*(1.-P)*R2/H2-.1875+.1875*P)+6.*(1.-P)*R2/H2 TOR08720
1 -.5*0.*(-.1875+.1875*P-9.*UMP*EKA)-.25+.25*P-.5*M2*(6.*EKA+.5)) TOR08730
1 *G2+(-.5*0.*(3.*(1.-P)*R2/H2+.25-.25*P)+.125-.125*P TOR08740
1 -.5*0.*(-.125+.125*P-1.5*UMP*EKA)+1.5*(1.-P)*R2/H2)*G4 TOR08750
AA(7,9)=AA(7,9)+ TOR08760
1 (-.5*0.*M*(-.75-.25*P)-.5*0.*M*(1.25-.75*P) TOR08770
1 +.5*M*(6.*EKA*(2.+3.*P)-1.+P))*G2 TOR08780
AA(7,10)=AA(7,10)+ TOR08790
1 (.5*M*(.125*P+.125+3.*P*R2/H2+1.5*UMP*EKA) TOR08800

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1 +.5*M*(.25*(6.*P-18.)*R2/H2-.375+.125*P))*G3	TOR08810
AA(7,11)=AA(7,11)+	TOR08820
1 (-.5*(6.*(1.-P)*R2/H2+.125-.125*P)-.5*(-4.5*(1.-P)*R2/H2	TOR08830
1 -.375+.375*P)+3.75*UMP*EKA+.09375-.09375*P)*G3	TOR08840
AA(7,12)=AA(7,12)+	TOR08850
1 (.125*M-.125*M+.5*M*(6.*EKA*F*UPP-.25+.25*P))*G3	TOR08860
AA(7,14)=AA(7,14)+	TOR08870
1 (-.5*.4*(.75*(1.-P)*R2/H2+.0625-.0625*P)-.5*2.*	TOR08880
1 (-.75*UMP*EKA-.0625+.0625*P)+.75*UMP*EKA+.0625-.0625*P)*G4	TOR08890
AA(10,5)=AA(10,5)+	TOR08900
1 (-.5*0.*(6.*(1.-P)*R2/H2+.125-.125*P)-.5*0*(-4.5*(1.-P)*R2/H2	TOR08910
1 -.375+.375*P)+3.75*UMP*EKA+.09375-.09375*P)*G3	TOR08920
AA(10,6)=AA(10,6)+	TOR08930
1 (.125*0.*M-.125*0.*M+.5*M*(6.*EKA*F*UPP-.25+.25*P))*G3	TOR08940
AA(10,8)=AA(10,8)+	TOR08950
1 (-.5*(.75*(1.-P)*R2/H2+.0625-.0625*P)-.5*	TOR08960
1 (-.75*UMP*EKA-.0625+.0625*P)+.75*UMP*EKA+.0625-.0625*P)*G4	TOR08970
AA(13,2)=AA(13,2)+	TOR08980
1 (-.5*0*(.75*(1.-P)*R2/H2+.0625-.0625*P)-.5*0.*	TOR08990
1 (-.75*UMP*EKA-.0625+.0625*P)+.75*UMP*EKA+.0625-.0625*P)*G4	TOR09000
AA(13,3)=0.	TOR09010
C FIX UP 'W' EQUATIONS	TOR09020
2300 JHI=13	TOR09030
JEQ=2	TOR09040
DO 2310 I=1,5	TOR09050
DO 2320 J=1,JHI	TOR09060
2320 AA(JEQ,J)=.0	TOR09070
JHI=JHI-3	TOR09080
2310 JEQ=JEQ+3	TOR09090
AA(5,11)=AA(5,11)+	TOR09100
1 (-.5*M3-.5*M*(12.*EKA+.5+.5*P+36.*P*EKA)-.5*0.*M*P	TOR09110
1 +.5*0.*M*(1.5+.5*P))*G	TOR09120
1 +(-.5*M*(9.*EKA-1.75+.75*P+9.*P*EKA)-.375*0.*M-.125*0.*M)*G3	TOR09130
AA(5,12)=AA(5,12)+	TOR09140
1 (2.*0-.5*0*(-1.-P)-.5*M2*(3.-P)+6.*EKA*2.*DPP	TOR09150
1 +2.*M2*0.-0.-.5*0.*P+0.*M2)*G	TOR09160
1 +(1.5*0-.5*0*(-2.75-2.25*P)-.75*0+.6.*EKA*4.5*UPP	TOR09170
1 -.5*0*(-1.75+.75*P))*G3	TOR09180
AA(5,13)=AA(5,13)+	TOR09190
1 (.5*(36.*EKA+18.*P*EKA+1.+1.5*P)+1.5-.25*M2+.5	TOR09200
1 -3.*EKA-1.5-.75*(12.*EKA+1.)*P+.25*M2)*G2	TOR09210
1 +(.25-.25+.5*(6.*EKA+6.*P*EKA+.5+.5*P)+.375	TOR09220
1 -1.5*EKA-.125*(12.*EKA+1.)*P)*G4	TOR09230
AA(5,14)=AA(5,14)+	TOR09240
1 (-.5*M*(12.*EKA-2.+5*P+18.*P*EKA)+.5*M*(-.75-.25*P)	TOR09250
1 +.5*M*(.25+.25*P))*G2	TOR09260
AA(5,15)=AA(5,15)+	TOR09270
1 (1.5-.5*(-1.-1.5*P)-.5*M2*(-.5-.5*P)	TOR09280
1 +6.*EKA*(3.5+3.*P)+.5*M2-1.5-.5*(-1.+1.5*P)	TOR09290
1 +.5*M2)*G2	TOR09300
1 +(.25-.5*(-.5-.5*P)-.25+6.*EKA*UPP	TOR09310
1 -.5*(-.75+.25*P))*G4	TOR09320
AA(5,16)=AA(5,16)+	TOR09330
1 (.5*8+.5*2*(12.*EKA+9.*P*EKA+.25+.75*P)-.75*4.+375	TOR09340
1 -3.*EKA-.375*(12.*EKA+1.)*P)*G3	TOR09350

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AA(5,17)=AA(5,17)+
1 (-.5*M*(3.*EKA-.25+.25*P+3.*P*EKA)-.125*4.*M-.125*2.*M)*G3 TOR09360
AA(5,18)=AA(5,18)+ TOR09370
1 (.5*16.-.5*4.*(-.25-.75*P)+6.*EKA*1.5*UPP-.75*8. TOR09380
1 -.5*2.*(-.75+.75*P))*G3 TOR09390
AA(5,19)=AA(5,19)+ TOR09400
1 (.0625*27+.5*3.*(1.5*EKA+1.5*P*EKA+.125*P)-.125*9+.0625 TOR09410
1 -.75*EKA-.0625*(12.*EKA+1.)*P)*G4 TOR09420
AA(5,21)=AA(5,21)+ TOR09430
1 (.0625*81+.0625*9.*P+6.*EKA*.25*UPP-.125*27. TOR09440
1 -.5*3.*(-.125+.125*P))*G4 TOR09450
AA(8,8)=AA(8,8)+ TOR09460
1 (-.5*M*(12.*EKA-2+.5*P+18.*P*EKA)+.5*0.*M*(-.75-.25*P) TOR09470
1 +.5*0.*M*(.25+.25*P))*G2 TOR09480
AA(8,9)=AA(8,9)+ TOR09490
1 (1.5*0.-.5*0.*(-1.-1.5*P)-.5*M2*(-.5-.5*P) TOR09500
1 +6.*EKA*(3.5+3.*P)+.5*M2*0.-1.5*0.-.5*0.*(-1.+1.5*P) TOR09510
1 +.5*0.*M2)*G2 TOR09520
1 +(.25*0.-.5*0.*(-.5-.5*P)-.25*0.+6.*EKA*UPP TOR09530
1 -.5*0.*(-.75+.25*P))*G4 TOR09540
AA(8,10)=AA(8,10)+ TOR09550
1 (.5+.5*(12.*EKA+9.*P*EKA+.25+.75*P)-.75+.375 TOR09560
1 - 3.*EKA-.375*(12.*EKA+1.)*P)*G3 TOR09570
AA(8,11)=AA(8,11)+ TOR09580
1 (-.5*M*(3.*EKA-.25+.25*P+3.*P*EKA)-.125*M-.125*M)*G3 TOR09590
AA(8,12)=AA(8,12)+ TOR09600
1 (.5-.5*(-.25-.75*P)+6.*EKA*1.5*UPP-.75 TOR09610
1 -.5*(-.75+.75*P))*G3 TOR09620
AA(8,13)=AA(8,13)+ TOR09630
1 (.0625*8+.5*2.*(1.5*EKA+1.5*P*EKA+.125*P)-.125*4+.0625 TOR09640
1 -.75*EKA-.0625*(12.*EKA+1.)*P)*G4 TOR09650
AA(8,15)=AA(8,15)+ TOR09660
1 (.0625*16+.0625*4.*P+6.*EKA*.25*UPP-.125*8. TOR09670
1 -.5*2.*(-.125+.125*P))*G4 TOR09680
AA(11,5)=AA(11,5)+ TOR09690
1 (-.5*M*(3.*EKA-.25+.25*P+3.*P*EKA)-.125*0.*M-.125*0.*M)*G3 TOR09700
AA(11,6)=AA(11,6)+ TOR09710
1 (.5*0.-.5*0.*(-.25-.75*P)+6.*EKA*1.5*UPP-.75*0. TOR09720
1 -.5*0.*(-.75+.75*P))*G3 TOR09730
AA(11,7)=AA(11,7)+ TOR09740
1 (.0625+.5*(1.5*EKA+1.5*P*EKA+.125*P)-.125+.0625 TOR09750
1 -.75*EKA-.0625*(12.*EKA+1.)*P)*G4 TOR09760
AA(11,9)=AA(11,9)+ TOR09770
1 (.0625+.0625*P+6.*EKA*.25*UPP-.125 TOR09780
1 -.5*(-.125+.125*P))*G4 TOR09790
AA(14,3)=AA(14,3)+ TOR09800
1 (.0625*0+.0625*0.*P+6.*EKA*.25*UPP-.125*0. TOR09810
1 -.5*0.*(-.125+.125*P))*G4 TOR09820
RETURN TOR09830
END TOR09840

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C SUBROUTINE BANSOL(NHI)
  IMPLICIT DOUBLE PRECISION (A-H,O-Z)
C CUSTOMIZED BAND SOLVER FOR STRIP BANDED MATRIX - TOROIDAL SHELL
  COMMON/EEE/A(200,200),B(200),X(200),PX(200),

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TOR09850
TOR09860
TOR09870
TOR09880
TOR09890
TOR09900

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1          AN(200,200),BN(200,1),XN(200),WK(200)
C THE U'ZERO' EQN IS THE ZERO EQN OF THE SYSTEM TOR09910
C IROW= ROW NO OF EQN BEFORE SET TO BE REDUCED TOR09920
C IO = ROW NO OF EQN TO BE REDUCED (ORIGIN ROW) TOR09930
C JOS = COL NO OF PIVOT (16,17 OR 18) ON DESTINATION ROW TOR09940
C ID = ROW NO OF EQN TO BE MODIFIED (DESTINATION ROW) TOR09950
C KLO = COL NO OF 1ST COF OF ORIGIN ROW TO BE ADDED TO DEST ROW TOR09960
C KHI = COL NO OF LAST COF OF ORIGIN ROW TO BE ADDED TO DEST ROW TOR09970
C JDS = COL NO OF FACTOR OF DESTINATION ROW TOR09980
C JD = COL NO OF DESTINATION ROW TO BE MODIFIED TOR09990
NEQ=NHI*3+2 TOR10000
IHI=NHI+1 TOR10010
IO=0 TOR10020
C REDUCE EQNS FOR EACH ORDER OF N TOR10030
DO 1000 I=1,IHI TOR10040
C THREE EQNS REDUCED FOR EACH N (EXCEPT FOR N=0 THEN ONLY TWO) TOR10050
DO 1100 IG=1,3 TOR10060
C IG=1 'U' EQN IG=2 'V' EQN IG=3 'W' EQN TOR10070
IF(IO.EQ.0) GO TO 1100 TOR10080
IF(IO.EQ.NEQ) GO TO 1100 TOR10090
JOS=12+IG TOR10100
D=1./A(IO,JOS) TOR10110
ID=IO TOR10120
C A GIVEN PIVOT AFFECTS VERTICALLY 8 GROUPS OF EQNS TOR10130
DO 1200 J=1,5 TOR10140
JHI=15-JOS TOR10150
IF(J.GT.1) JHI=3 TOR10160
IF(JHI.EQ.0) GO TO 1200 TOR10170
C MUST CONSIDER 1,2 OR 3 EQNS VERTICALLY IN EACH GROUP TOR10180
DO 1300 JG=1,JHI TOR10190
ID=ID+1 TOR10200
IF(ID.GT.NEQ) GO TO 1300 TOR10210
JDS=JOS-(J-1)*5 TOR10220
IF(JDS.LT.1) GO TO 1300 TOR10230
FAC=A(ID,JDS)*D TOR10240
IF(ABS(FAC).LT.(1.E-15)) GO TO 1300 TOR10250
KLO=JOS+1 TOR10260
KHI=27 TOR10270
JD=JDS TOR10280
DO 1400 K=KLO,KHI TOR10290
JD=JD+1 TOR10300
IF(JD.GT.27) GO TO 1400 TOR10310
A(ID,JD)=A(ID,JD)-FAC*A(IO,K) TOR10320
1400 CONTINUE TOR10330
B(ID)=B(ID)-FAC*B(IO) TOR10340
1300 CONTINUE TOR10350
1200 CONTINUE TOR10360
1100 IO=IO+1 TOR10370
1000 CONTINUE TOR10380
C BACKSUBSTITUTE DONT HAVE '1' ON DIAGONALS TOR10390
C BOTTOM SET OF THREE EQUATIONS TOR10400
X(NEQ)=B(NEQ)/A(NEQ,15) TOR10410
NEQ1=NEQ-1 TOR10420
X(NEQ1)=(B(NEQ1)-A(NEQ1,15)*X(NEQ))/A(NEQ1,14) TOR10430
NEQ2=NEQ-2 TOR10440
TOR10450

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X(NEQ2)=(B(NEQ2)-A(NEQ2,14)*X(NEQ1)-A(NEQ2,15)*X(NEQ))/A(NEQ2,13) TOR10460
C BACKSUBSTITUTE NHI-1 GROUPS OF THREE EQUATIONS TOR10470
NHIM1=NHI-1 TOR10480
DO 2000 I=1,NHIM1 TOR10490
C START WITH 'W' EQN THEN 'V' EQN LAST 'U' EQN TOR10500
DO 2100 IG=1,3 TOR10510
JHI=15+I*3 TOR10520
IF(JHI.GT.27) JHI=27 TOR10530
IF(I.GT.2.AND.IG.GT.1) JHI=24 TOR10540
JLO=17-IG TOR10550
IRA=NEQ+1-(I*3+IG) TOR10560
IK=0 TOR10570
SUM=.0 TOR10580
DO 2200 J=JLO,JHI TOR10590
IK=IK+1 TOR10600
IRB=IRA+IK TOR10610
2200 SUM=SUM+A(IRA,J)*X(IRB) TOR10620
X(IRA)=(B(IRA)-SUM)/A(IRA,JLO-1) TOR10630
2100 CONTINUE TOR10640
2000 CONTINUE TOR10650
C TOP SET OF TWO EQUATIONS TOR10660
IK=0 TOR10670
SUM=.0 TOR10680
DO 2310 J=16,27 TOR10690
IK=IK+1 TOR10700
2310 SUM=SUM+A(2,J)*X(2+IK) TOR10710
X(2)=(B(2)-SUM)/A(2,15) TOR10720
IK=0 TOR10730
SUM=.0 TOR10740
DO 2330 J=15,27 TOR10750
IK=IK+1 TOR10760
2330 SUM=SUM+A(1,J)*X(1+IK) TOR10770
X(1)=(B(1)-SUM)/A(1,14) TOR10780
RETURN TOR10790
END TOR10800
C----- TOR10810
SUBROUTINE ADVAL(MS,N,U,V,W,PR) TOR10820
IMPLICIT DOUBLE PRECISION (A-H,O-Z) TOR10830
DOUBLE PRECISION MDUM,M2,M3,M4 TOR10840
C ADD CONTRIBUTION FROM ONE TERM TO DISP-STRAIN-CURV TOR10850
C AT EACH OF OUTPUT POINTS TOR10860
COMMON/AAA/R,RR,PSI,T,E,POI,MHI,NHI,NFI,NT,NLOAD TOR10870
COMMON/BBB/ RES(40,20,10),GFI(40),GETA(40),GT(20),DATLOD(10,7) TOR10880
COMMON/CCC/PI5,PI2,PI,SOP2,RAD,OMP,OMP2,TP,AK,STIF,ROS,R2,R3,CZ, TOR10890
1 ZD,PSIR TOR10900
COMMON/FFF/P,ZK,ZC,ZX,ZY,G,G2,G3,G4,G5,MDUM,M2,M3,M4,F,Q,A,S, TOR10910
1 UMP,DMP,TMP,UPP,DPP,TPP,BB(9),STC(20),STS(20),STZ(20), TOR10920
1 B2,B1,VMP,SMP,EKD,EKB,EKC,EKE,EL,EL1,H,EKA TOR10930
DIMENSION SETA(40),CETA(40),ST(20),CT(20) TOR10940
EM=REAL(MS) TOR10950
EN=REAL(N) TOR10960
IF(N.GT.0) GO TO 2000 TOR10970
C CASE FOR N=0 REMEMBER 'FI' SIGNIFIES 'ETA' TOR10980
B=EM*PI*G/PSIR TOR10990
DO 10 I=1,NFI TOR11000

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ARG=B*GETA(I)
SETA(I)=SIN(ARG)
10 CETA(I)=COS(ARG)
C COMPUTING V W E1 E2 K1 K2 PR FOR EACH OF GRID POINTS
DO 11 I=1,NFI
DO 11 J=1,NT
C=STC(J)
Z=STZ(J)
ET=W/R
EFI=(-B*V+G*C*W)/(R*Z)
AKFI=(-B*V*G*C+B*B*W)/(R2*Z*Z)
RES(I,J,4)=RES(I,J,4)+V*CETA(I)
RES(I,J,5)=RES(I,J,5)+W*SETA(I)
RES(I,J,6)=RES(I,J,6)+ET*SETA(I)
RES(I,J,7)=RES(I,J,7)+EFI*SETA(I)
RES(I,J,9)=RES(I,J,9)+AKFI*SETA(I)
11 RES(I,J,10)=RES(I,J,10)+PR*SETA(I)
RETURN
C CASE FOR N.GT.0
2000 DO 20 J=1,NT
ARG=EN*GT(J)
ST(J)=SIN(ARG)
20 CT(J)=COS(ARG)
C COMPUTING U V W EX EFI KX KFI FOR EACH OF GRID POINTS
DO 21 I=1,NFI
DO 21 J=1,NT
C=STC(J)
S=STS(J)
Z=STZ(J)
CC=CT(J)*CETA(I)
CS=CT(J)*SETA(I)
SS=ST(J)*SETA(I)
SC=ST(J)*CETA(I)
ET=(N*U+W)/R
EFIS=(-G*S*U)/(Z*R)
EFIC=(-B*V+G*C*W)/(R*Z)
AKT=(N*U+N*N*W)/R2
AKFIC=(-C*B*V*G+B*B*W)/(R2*Z*Z)
AKFIS=G*S*(-U-N*W)/(R2*Z)
RES(I,J,3)=RES(I,J,3)+U*SS
RES(I,J,4)=RES(I,J,4)+V*CC
RES(I,J,5)=RES(I,J,5)+W*CS
RES(I,J,6)=RES(I,J,6)+ET*CS
RES(I,J,7)=RES(I,J,7)+(EFIC*CS+EFIS*SS)
RES(I,J,8)=RES(I,J,8)+AKT*CS
RES(I,J,9)=RES(I,J,9)+(AKFIC*CS+AKFIS*SS)
21 RES(I,J,10)=RES(I,J,10)+PR*CS
RETURN
END
C-----
SUBROUTINE RESULT
IMPLICIT DOUBLE PRECISION (A-H,O-Z)
C FINALIZE SOLUTION VALUES AND WRITEOUT
COMMON/AAA/R,RR,PSI,T,E,POI,MHI,NHI,NFI,NT,NLOAD
COMMON/BBB/ RES(40,20,10),GFI(40),GETA(40),GT(20),DATLOD(10,7)

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TOR11010
TOR11020
TOR11030
TOR11040
TOR11050
TOR11060
TOR11070
TOR11080
TOR11090
TOR11100
TOR11110
TOR11120
TOR11130
TOR11140
TOR11150
TOR11160
TOR11170
TOR11180
TOR11190
TOR11200
TOR11210
TOR11220
TOR11230
TOR11240
TOR11250
TOR11260
TOR11270
TOR11280
TOR11290
TOR11300
TOR11310
TOR11320
TOR11330
TOR11340
TOR11350
TOR11360
TOR11370
TOR11380
TOR11390
TOR11400
TOR11410
TOR11420
TOR11430
TOR11440
TOR11450
TOR11460
TOR11470
TOR11480
TOR11490
TOR11500
TOR11510
TOR11520
TOR11530
TOR11540
TOR11550

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COMMON/CCC/PI5,PI2,PI,SOP2,RAD,OMP,OMP2,TP,AK,STIF,ROS,R2,R3,CZ. TOR11560
1 ZD,PSIR TOR11570
C START WITH FI T(DEG) U V W ET EFI KT KFI PR TOR11580
C FINALISE TO FI T(DEG) U V W STT STB SFIT SFIB PR TOR11590
WRITE(8,16) TOR11600
16 FORMAT(/' RESULTS FOR DISPLACEMENTS(MM) AND STRESSES(MPA)'/ TOR11610
1 7X,'FI',3X,'THETA',9X,'U',14X,'V',14X,'W',11X, TOR11620
2 'STTOP',10X,'STBOT',10X,'SFITOP',9X,'SFIBOT',7X,'PR') TOR11630
DO 100 I=1,NFI TOR11640
DO 110 J=1,NT TOR11650
ENT=AK*(RES(I,J,6)+POI*RES(I,J,7)) TOR11660
ENFI=AK*(RES(I,J,7)+POI*RES(I,J,6)) TOR11670
EMT=ZD*(RES(I,J,8)+POI*RES(I,J,9)) TOR11680
EMFI=ZD*(RES(I,J,9)+POI*RES(I,J,8)) TOR11690
116 FORMAT(1X,'EMT = ',D15.6,2X,'EMFI = ',D15.6) TOR11700
RES(I,J,6)=ENT/T+EMT*CZ TOR11710
RES(I,J,7)=ENT/T-EMT*CZ TOR11720
RES(I,J,8)=ENFI/T+EMFI*CZ TOR11730
RES(I,J,9)=ENFI/T-EMFI*CZ TOR11740
110 WRITE(8,26) (RES(I,J,K),K=1,10) TOR11750
26 FORMAT(1X,2F8.2,7D15.6,F8.4) TOR11760
100 WRITE(8,36) TOR11770
36 FORMAT(1H) TOR11780
RETURN TOR11790
END TOR11800
C***** END ***** TOR11810
C LISTING FOR TORSAN EXEC TOR11820
C----- TOR11830
C/* TOR11840
C F EXEC WILL RUN A VS FORTRAN PROGRAM. TOR11850
C IT IS INVOKED BY TYPING: TOR11860
C F <PROGRAM NAME> O( ONOCOMPILE" OSINGLE" ODATA=<DATA FILE NAME>" TOR11870
C ORESULT=<RESULT FILE NAME>" TOR11880
C OLIBRARY=<TEXT LIBRARY NAME>" " TOR11890
C WHERE OPTIONAL ARGUMENTS ARE: TOR11900
C NOCOMPILE: USED IF A COMPILED TEXT VERSION EXISTS. TOR11910
C SINGLE: SPECIFIES THAT THE SINGLE PRECISION IMSL LIBRARY SHOULD TOR11920
C BE USED. THE DEFAULT IS DOUBLE PRECISION. TOR11930
C DATA: GIVES THE NAME OF THE INPUT DISK FILE, OF FILETYPE DATA. TOR11940
C THE DEFAULT IS THE PROGRAM NAME. TOR11950
C RESULT: GIVES THE NAME OF THE RESULT FILE, OF FILETYPE RESULT. TOR11960
C THE DEFAULT IS THE PROGRAM NAME. TOR11970
C LIBRARY: GIVES THE NAME OF A USER SUPPLIED TEXT LIBRARY. TOR11980
C IN THE FORTRAN PROGRAM, THE FILE NUMBERS USED MUST BE: 5 FOR INPUT FROTOR11990
C DISK DATA FILE; 6 FOR OUTPUT TO DISK DATA FILE; 7 FOR INPUT/OUTPUT WITTOR2000
C THE TERMINAL. TOR12010
C /* TOR12020
C PARSE UPPER ARG PROGRAMNAME '(' OP.1 OP.2 OP.3 OP.4 OP.5 TOR12030
C DATANAME=PROGRAMNAME TOR12040
C RESULTNAME=PROGRAMNAME TOR12050
C IMSLLIBRARY='IMSLDLIB' TOR12060
C USERLIBRARY='' TOR12070
C DO I=1 TO 5 TOR12080
C PARSE VAR OP.I OPTION '=' ARGUMENT TOR12090
C IF ABBREV('NOCOMPILE',OPTION,1) THEN COMPILE='NO' TOR12100

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C IF ABBREV('DATA',OPTION,1) THEN DATANAME=ARGUMENT TOR12110
C IF ABBREV('RESULT',OPTION,1) THEN RESULTNAME=ARGUMENT TOR12120
C IF ABBREV('SINGLE',OPTION,1) THEN IMSLLIBRARY='IMSLSLIB' TOR12130
C IF ABBREV('LIBRARY',OPTION,1) THEN USERLIBRARY=ARGUMENT TOR12140
C END TOR12150
C IF COMPILE /= 'NO' THEN 'FORTVS' PROGRAMNAME '(SDUMP' TOR12160
C 'GLOBAL TXTLIB VLNKMLIB VFORTLIB CMSLIB' IMSLLIBRARY TOR12170
C 'FILEDEF 5 DISK' DATANAME 'DATA A' TOR12180
C 'FILEDEF 8 DISK' RESULTNAME 'RESULT A (LRECL 130' TOR12190
C 'FILEDEF 8 TERMINAL' TOR12200
C /* 'ERASE' PROGRAMNAME 'LISTING' */ TOR12210
C 'LOAD' PROGRAMNAME '(START' TOR12220
C TOR12230
C LISTING FOR TORSAN DATA FILE TOR12240
C TOR12250
C$DATA TOR12260
C PAD LOADING USING TORSAN PROGRAM JUNE30,1991 TOR12270
C 51 ,25, 15,10, 1 TOR12280
C 84.15, 252.45, 90., 7.1, .207006, .3 TOR12290
C .0,11.25,22.5,27.5,30.,32.5,37.5,45.,52.5,57.5,60.,62.5,67.5, TOR12300
C 78.75,90. TOR12310
C 0.,5.,15.,30.,45.,67.5,90.,120.,150.,180. TOR12320
C 1., 7.5, 7.5, 15., 15. TOR12330
C*****END ALL***** TOR12340

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